

**Appendix E:
Preliminary SUSMP Report**

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**PRELIMINARY
SUSMP
REPORT**

EMERALD ISLE

**GULLANE DRIVE
APN's 173-670-016 &
173-670-004
SANTA ROSA, CA 95403**

**OAKMONT SENIOR LIVING
LLC**

B&R JOB # 4081.02

APRIL 10, 2017



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- 1. Proposed Condition Exhibit
- 2. Preliminary Grading, Drainage, and Utility Drawings

Project Description

The proposed project is located on Gullane Drive, east of Thomas Lake Harris in the City of Santa Rosa. The project site totals 12.57± acres.

The site is an undeveloped isolated knoll, surrounded by the Fountaingrove Golf & Country Club. The site's slopes range primarily from 10% to 25%. The site currently drains overland in all directions from the knoll. The runoff travels across the surrounding existing golf course before entering the golf course surface and underground storm drain systems, or into Fountaingrove Lake to the east.

The Soil Map on the following page was generated using the USDA Natural Resource Conservation Service Web Soil Survey. The soil classifications for the project area are majority Goulding cobbly clay loam (GIE), some Raynor clay (RaD), with insignificant amounts of Spreckles Loam (SkD) and Felta very gravelly loam (FaE).

The proposed Oakmont Senior Living project is a 49 unit residential care facility for the elderly, more commonly known as assisted living. A two-story structure of approximately 68,144 square feet is designed architecturally from the ground up with many features throughout the project that meet the special needs of the elderly.

Outdoor amenities will include fountains, arbors, dining patios, walking paths, raised garden beds, badminton sports courts, bocce court, pet park and 12 garages. The project will include construction of a new building with interior and surrounding courtyards, landscaping, garages and parking lots, and an access driveway with sidewalks extending Gullane Drive to the site. The project is classified as a residential development and triggers these storm water LID requirements by creating over 10,000 square feet of new impervious surface.

Pollution Prevention Measures

The project design has incorporated pollution source controls intended to prevent pollutants from entering downstream drainage systems. These source controls include:

- Drainage inlets and structural BMP's will be fitted with gross pollutant (trash) racks and interceptor trays. Racks and trays will be regularly inspected and any captured debris removed and properly disposed of.
- The site landscape and hardscape will be professionally maintained, swept clean and with landscape leaves and debris removed on a regular schedule over the course of the year.
- Trash will be stored in a covered exterior trash enclosure. Local drainage will be routed away from the trash enclosure location.
- Vehicles will not be allowed to be washed or mechanically maintained on the site. All such activities will be required to occur off-site with encouragement of the use of commercial car-washes and car maintenance and repair shops.
- Landscape irrigation heads will be sized and tuned to avoid overspray and overwatering along with the selected use of drip irrigation.
- Capture and retention of at least the runoff from the 85th percentile storm event to the maximum extent practicable (MEP) of the entire site

- Routing excess intercepted runoff to sheet drain downslope through adjacent existing vegetation.

Types of BMP's

The Low Impact Development Technical Design Manual (Manual) encourages the use of Low Impact Development (LID) techniques to both retain and treat runoff water from impervious surfaces. The Manual prioritizes both universal techniques that are independent of soil type, groundwater level, and groundwater contamination and small scale, landscape-based LID techniques located close to the source of pollution. However, the higher priority BMP's are not always feasible on all sites.

Drainage Management Areas (DMAs) 1, 4, 7 and 9, as indicated on the attached Proposed Condition Map, will drain into infiltration trenches, installed on contour on the existing hillside. All runoff from the 85th percentile storm will be captured. Runoff from larger storms will bypass the trenches once they are full and eventually spill over and sheet flow down the existing hillside.

Drainage Management Area (DMAS) 2, 6 and 8, as indicated on the attached Proposed Condition Map, will drain into a structural (i.e. KriStar Cudo™) infiltration system. The system will be sized such that all runoff from the 85th percentile storm routed through these BMP's will be captured and retained. Runoff from larger storms will bypass the trenches once they are full and eventually spill over and sheet flow down the existing hillside.

Drainage Management Area (DMA) 3, as indicated on the attached Proposed Condition Map, will drain into a structural (i.e. KriStar Cudo™) infiltration system. The system will be sized such that all runoff from the 85th percentile storm routed through these BMP's will be captured and retained. Runoff from larger storms will bypass and be conveyed to an underground storm drain system via a storm drain pipe connection. Retained stormwater will be allowed to infiltrate into the soil.

Drainage Management Areas (DMAS) 5 and 10, as indicated on the attached Proposed Condition Map, will drain through a pervious concrete curb & gutter and into a bioretention area beneath the adjacent sidewalk for most of the length of the driveway. All runoff from the 85th percentile storm will be captured and retained. Excess runoff from larger events will be bypassed via perforated pipe to the private site storm drain system. Retained stormwater will be allowed to infiltrate into the soil.

Level of Treatment and Volume Capture

The design goal of 100% capture for the overall site will be achieved by routing 100% of event runoff (or as near to 100% as possible) through the various BMP's associated with each of the DMA's around the project. In addition, BMP's will be designed to retain at minimum the volume of runoff generated by the 85th percentile event directed into each BMP before bypassing any excess runoff. Excess runoff will be dispersed to multiple outlets around the project where it will continue on downhill through the existing trees and vegetation before exiting the property. Outlets will be designed to de-energize and distribute flow to mitigate the erosive effect of concentrated flow.

Maintenance Funding

BMPs shall be inspected and maintained as described in "Planter Strip Bioretention Inspection and Maintenance Checklist", "Porous Pavement Inspection and Maintenance Checklist" and "Interception Trench Inspection and Maintenance Checklist provided in the LID Manual Reference Documents Section. Structural BMP's such as the "Cudo" unit arrays shall be inspected and maintained in accordance with the operations and maintenance (O&M) manual published by the manufacturer. All associated costs for inspection or maintenance of the onsite best management practices (BMPs) shall be budgeted for this purpose and carried out by Oakmont Senior Living LLC., or its assigned successor(s).

FOR OFFICE USE ONLY:
Does this project require
permanent storm water BMP's?

☐ Y ☐ N



Print Form

File No:	Quadrant
Related Files:	
Set:	
Department Use Only	

Storm Water Determination Worksheet

PURPOSE AND APPLICABILITY: Use this form to determine whether or not this project will need to incorporate permanent Storm Water Best Management Practices (BMP's) and submit a Standard Urban Storm Water Mitigation Plan (SUSMP) as required by the City's National Pollutant Discharge Elimination System Municipal Separate Storm Sewer System (NPDES MS4) only. Your project may still need to incorporate permanent storm water BMP's as required by CALGREEN or North Coast Regional Water Quality Control Board (NCRWQCB) section 401 permit.

Part 1: Project Information

Emerald Isle

Project Name

Oakmont Senior Living, LLC

Applicant (owner or developer) Name

Gullane Drive

Site Address

9240 Old Redwood Highway, Suite 200

Mailing Address

Santa Rosa, CA, 95403

City/State/Zip

Windsor, CA, 95492

City/State/Zip

Permit Number(s) - if applicable

(707) 535-3200/bill.mabry@oakmontsl.com/(707) 535-3200

Phone/Email/Fax

Brelje & Race Consulting Engineers

Engineer Name

475 Aviation Blvd., Suite 120

Mailing Address

Santa Rosa, CA, 95403

City/State/Zip

(707) 576-1322 / bartholow@brce.com

Phone/Email

Type of Application/Project:

☐ Subdivision ☒ Grading Permit ☒ Building Permit ☒ Hillside Development
☐ Design Review ☒ Use Permit ☐ Encroachment ☐ Time Extensions ☐ Other

PART 2: Project Exemptions

1. Is this project a routine maintenance activity¹ that is being conducted to maintain original line and grade, hydraulic capacity, and original purpose of facility such as resurfacing existing roads and parking lots?

☐ Yes ☒ No

2. Is this project a stand alone pedestrian pathway, trail or off street bike lane?

☐ Yes ☒ No

¹ "Routine Maintenance Activity" This exemption includes activities such as overlays and/or resurfacing of existing roads or parking lots as well as trenching and patching activities and reroofing activities.

3. Is this a project that does not create or replace any impervious surface²?

☐ Yes ☒ No

4. Is this a project that proposes fewer than four dwelling units and a combined total of less than 1.0 acre of new or replaced impervious surface?

☐ Yes ☒ No

Did you answer "YES" to any of the above questions in Part 3?

☐ **YES:** This project will *not* need to incorporate permanent Storm Water BMP's. **Please complete the "Exemption Signature Section" on Page 4.**

☒ **NO:** Proceed to Part 3 below to see if this project will need to incorporate permanent Storm Water BMP's.

Part 3: Project Triggers

Projects that Trigger Requirements:

Please answer the following questions to determine whether this project requires permanent Storm Water BMP's and the submittal of a SUSMP.

1. Does this project create or replace a combined total of 1.0 acre or more of impervious surface?

☒ Yes ☐ No

2. Does this project create or replace a combined total of 10,000 ft² or more of impervious street, roads, highways, or freeway construction or reconstruction? ☒ Yes ☐ No

3. Does this project include four or more new dwelling units? ☒ Yes ☐ No

4. Is this project an industrial park³, commercial strip mall⁴, retail gasoline outlet, restaurant, or an automotive service facility creating or replacing a combined total of 10,000 sq ft or more of impervious surface? ☐ Yes ☒ No

5. Is this project a parking lot (not included as part of a project type listed above) creating or replacing a combined total of 10,000 sq ft or more of impervious surface or with 25 or more parking spaces? ☐ Yes ☒ No

Did you answer "YES" to any of the above questions in Part 3?

☒ **YES:** This project requires permanent Storm Water BMP's and the submittal of a SUSMP. **Please complete the remainder of this worksheet and sign the "Acknowledgment Signature Section" on Page 4.**

☐ **NO:** This project will not need to incorporate permanent Storm Water BMP's. **Please complete the "Exemption Signature Section" on Page 4.**

² "Impervious Surface" is defined as an area that has been modified to reduce storm water runoff capture and percolation into underlying soils. Such surfaces include rooftops, walkways, and parking areas. Permeable pavements shall be considered impervious for this section if they have subdrains to preclude infiltration into underlying soils.

³ "Industrial Park" is defined as industrial facility or building and associated impervious surface on a site zoned or planned to allow industrial or commercial development (planning for mixed use residential, industrial or commercial development and redevelopment is included).

⁴ "Commercial Strip Mall" is defined as commercial facility or impervious surface on a site zoned or planned to allow commercial or industrial use (planning for mixed use residential, industrial or commercial development and redevelopment is included) with street access and onsite parking.

Emerald Isle

Part 4: Project Description

1. Total Project area: 12.57



square feet



acres

2. Existing land use(s): (check all that apply)



Commercial



Industrial



Residential



Public



Other

Description of buildings, significant site features (creeks, wetlands, heritage trees), etc.:

Existing project site is a vacant open space area with trees and grass.

3. Existing impervious surface area: 0.00



square feet



acres

4. Proposed Land Use(s): (check all that apply)



Commercial



Industrial



Residential



Public



Other

Description of buildings, significant site features (creeks, wetlands, heritage trees), etc.:

The proposed project site will consist of a 49 unit assisted living facility in one building and associated parking driveway and pedestrian walkways.

5. Existing impervious surface area: 2.70



square feet



acres

Acknowledgment Signature Section:

As the property owner or developer, I understand that this project is required to implement permanent Storm Water Best Management Practices and the submittal of a SUSMP as required by the City's NPDES MS4. Any unknown responses must be resolved to determine if the project is subject to these requirements.*

Applicant Signature_____
Date**Exemption Signature Section:**

As the property owner or developer, I understand that this project as currently designed does not require permanent Storm Water BMP's nor the submittal of a SUSMP as required by the City's NPDES MS4. I understand that redesign may require submittal of a new Determination Worksheet and may require permanent Storm Water BMP's.*

Applicant Signature_____
Date

*Your project may still need to incorporate permanent storm water BMP's as required by CALGREEN or North Coast Regional Water Quality Control Board (NCRWQCB) section 401 permit.

Implementation Requirements: All calculations shall be completed using the "Storm Water Calculator" available at: www.srcity.org/stormwaterLID

Design Goal: Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event, as calculated using the "Urban Hydrology for Small Watersheds" TR-55 Manual. 100% volume capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 2 and 3 of this calculator do not need to be completed. This is a retention requirement.

Design Requirements: If the Design Goal of 100% volume capture is not achieved; then both Requirement 1-100% Treatment AND Requirement 2- Volume Capture must be achieved.

Requirement 1: Treatment of 100% of the flow generated by the 85th percentile 24 hour storm event, as calculated using the Rational Method and a known intensity of 0.20 inches per hour.

Requirement 2: Capture (infiltration and/or reuse) of the increase in volume of storm water due to development generated by the 85th percentile 24 hour storm event, as calculated using the "Urban Hydrology for Small Watersheds" TR-55 Manual. This is a retention requirement.

BMP Selection Table

Best Management Practice (BMP)	Detail Sheet	Detail Title	Can be used with...					Achieves...			Treatment		Volume Capture	Pollution Prevention Credit	BMP in priority selected?		Explanation of selection	Other notes:
			High Ground Water	Contamination	Slope Constraints	Achieves...	Treatment	Volume Capture	Pollution Prevention Credit	BMP in priority selected?	Yes	No						
Universal LID Features- to be considered on all projects.	Living Roof	N/A	x	x	x		x	x			x							
	Rainwater Harvesting	N/A	x	x	x						x							
	Interceptor Trees	N/A	x	x	x								x					
	Vegetated Buffer Strip	UN-01											x					
	Bovine Terrace	UN-02	x										x					
	Impervious Area Disconnection	N/A	x	x	x								x					

BMP Selection Table

Best Management Practice (BMP)	Detail Sheet	Detail Title	Can be used with...												Achieves...	Treatment	Volume Capture	Pollution Prevention Credit	BMP in this priority selected?	Explanation of selection		Other notes:
			High Ground Water	Contamination	Slope Constraints	Achieves...														Yes	No	
Priority 1 and 1A BMPs- to be installed with no underdrains or liners. Must drain all stading water within 72 hours.	Rain Garden	P1-01													x							
	Roadside Bioretention	P1-02												x								
	Vegetated Swale-with Bioretention	P1-06												x								
	Constructed Wetlands	N/A												x								
	Infiltration Trench	P1-07												x					x			

BMP Selection Table

Best Management Practice (BMP)	Detail Sheet	Detail Title	Can be used with...										Achieves...	Volume Capture		Pollution Prevention Credit		BMP in this priority selected?	Other notes:	
			High Ground Water	Contamination	Slope Constraints	Treatment	Volume Capture	Pollution Prevention Credit	Yes	No	Explanation of selection	Other notes:								
Priority 2 BMPs- with subsurface drains installed above the capture volume.	Rain Garden	P2-01																		
	Roadside Bioretention	P2-02																		
		P2-03																		
		P2-04																		
		P2-05																		
	Pervious Pavement	P2-06																		
Constructed Wetlands	N/A																			

BMP Selection Table

Management Practice (BMP)	Detail Sheet	Detail Title	Best										Other notes:			
			Can be used with...	High Ground Water	Contamination	Slope Constraints	Achieves...	Treatment	Volume Capture	Pollution Prevention Credit	BMP in this priority selected?	Yes		No		
Priority 3 BMPs- installed with subdrains and/or impermeable liner. Does not achieve volume capture and must be used as part of a treatment train.	Rain Garden	P3-01	Rain Garden	x	x	x	x	x	x						x	
	Bioretention	P3-02	Roadside Bioretention - Flush Design	x	x	x	x	x								
		P3-03	Roadside Bioretention - Contiguous SW	x	x	x	x	x								x
		P3-04	Roadside Bioretention - Curb Opening	x	x	x	x	x								x
		P3-05	Roadside Bioretention - No C & G	x	x	x	x	x	x							x
																x
	Flow Through Planters															x
	Pervious Pavement	P1-04	Vegetated Buffer Strip	x	x	x	x	x	x							x
	Vegetated Swale	P3-07	Vegetated Swale	x	x	x	x	x	x							x

BMP Selection Table

Best Management Practice (BMP)		Can be used with...				Slope Constraints		Achieves...		Treatment		Volume Capture		Credit		BMP in this priority selected?		No	Explanation of selection	Other notes:
		High Ground Water Contamination		Contamination		Slope Constraints		Achieves...		Treatment		Volume Capture		Credit		BMP in this priority selected?				
Priority 4 BMPs- does not achieve volume capture and must be used as part of a treatment train.	Tree Filter Unit	x	x	x	x			x										x		
	Modular Bioretention	x	x	x	x			x												
Priority 5 BMPs- does not achieve volume capture and must be used as part of a treatment train.	Chambered Separator	x	x	x	x			x										x		
	Units	x	x	x	x			x												
	Centrifugal Separator	x	x	x	x			x										x		
	Units	x	x	x	x			x										x		
as part of a treatment train.	Trash Excluders	x	x	x	x			x										x		
	Filter Inserts	x	x	x	x			x										x		
Priority 6 BMPs-	Offset Program							N/A	N/A	N/A										
Other	Detention	x																		

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project: 4081.02
Address/Location: Emerald Isle
Designer: Brejle & Race Consulting Engineers
Date: March 20, 2017
Inlet Number/Tributary Area/BMP: 1

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 32.615 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains^[1]**Input:**

Select disconnection condition:

Runoff is directed across landscape: Width of area: 5' to 9'

Condition Factor = 0.25

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²

Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

(32.615 x 0.25 x 0.00) = 0.00 ft²

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

Select Density:

3-4

Units per Acre

Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

(32.615 x 0.25 x 0.00 x 0.19) = 0.00 ft²

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area** 1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for *existing tree canopy*=

0 ft²

Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction = 0 ft² = Sum of areas managed by evergreen + deciduous + existing canopy

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 32,615 ft²

Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²

Reduced Tributary Area to be used for Calculations = 32,615 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$S = \frac{1000}{CN} - 10$

Where:

S = Potential maximum retention after runoff (in)⁽⁶⁾
CN = Curve Number⁽⁵⁾

$Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8+S)]} \times \frac{1ft}{12"} \quad \text{Where:}$

Q = Runoff depth (ft)⁽⁶⁾
P = Precipitation (in) = 1.00
K = Seasonal Precipitation Factor⁽⁷⁾

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

$V = (Q)(A_r)$

Where:

V = Volume of Storm Water to be Retained (ft³)
A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A_r = 32,615 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select post development ground cover description⁽⁵⁾ = **Impervious - Paved Parking, Rooftop, Driveways**

OR: CN_{POST} = 98
Composite post development CN⁽⁹⁾ = 90

NOTE:
Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Solution: Volume of storm water - Post Development

S_{POST} = 1.15 in

S_{POST} = 1000 / 90 - 10

Where:

S_{POST} = Post development potential maximum retention after runoff (in).

Q_{POST} = 0.03523 ft

Q_{POST} = $\frac{[(1.00 * 1.17) - (0.2 * 1.15)]^2}{[(1.00 * 1.17) + (0.8 * 1.15)]} \times \frac{1ft}{12in}$

Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

V_{GOAL} = 1149.03 ft³

V_{GOAL} = (0.03523)(32,615)

V_{GOAL} = Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$$\underline{Q_{\text{TREATMENT}}} = (0.2 \text{ in/hr})(A_r)(C_{\text{POST}})(K) \text{ cfs}$$

Where:

$Q_{\text{TREATMENT}}$ = Design flow rate required to be treated (cfs)

C_{POST} = Rational method runoff coefficient for the developed condition^[10]

 A_i = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres)

K = Seasonal Precipitation Factor^[7]

Input:

$A_r =$	32,615	$\text{ft}^2 =$ 0.74874 Acres
$C_{\text{POST}}^{[10]} =$	0.50	
$K^{[7]} =$	1.2	

Solution:

$$Q_{\text{TREATMENT}} = 0.2(0.7487)(0.50)(1.17)$$

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, **must** be achieved.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000 - 10}{CN}$$

Where:

S= Potential maximum retention after runoff (in)^[6]
CN= Curve Number^[5]

$$Q = \frac{[(P-K) - (0.2 \cdot S)]^2}{[(P-K) + (0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:

Q= Runoff depth (ft)^[6]
P= Precipitation (in) = 0.92 0.92 Inches in the Santa Rosa area, based on local historical data.
K= Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:

V= Volume of Storm Water to be Retained (ft³)
A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A _r =	32,615 ft ²
K ^[7] =	1.2

Drop down Lists

Select hydrologic soil type within tributary area^[8] = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select predevelopment ground cover description^[9] = **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**
Select post development ground cover description^[9] = **Impervious - Paved Parking, Rooftop, Driveways**

CN _{PRE} =	82
CN _{POST} =	98
Composite Predevelopment CN ^[9] =	80
Composite Post development CN ^[9] =	90

OR

Composite Predevelopment CN^[9] = 80
Composite Post development CN^[9] = 90

Solution:

Pre Development Storm Water Runoff Volume

$$S_{PRE} = \frac{1000}{2.50} - 10 = 390$$

$$S_{PRE} = \frac{1000}{80} - 10 = 2.5$$

Where:
S_{PRE}= Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = \frac{[(0.92 - 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 - 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}} = 0.00900 \text{ ft}$$

Q_{PRE}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = 293.54 \text{ ft}^3$$

V_{PRE}= Pre Development Volume of Storm Water Generated (ft³)

Post Development Storm Water Runoff Volume

$$S_{POST} = \frac{1000}{1.15} - 10 = 869.56$$

$$S_{POST} = \frac{1000}{90} - 10 = 1.1$$

Where:
S_{POST}= Post development potential maximum retention after runoff (in).

$$Q_{POST} = \frac{[(0.92 - 1.17) - (0.2 \cdot 1.15)]^2}{[(0.92 - 1.17) + (0.8 \cdot 1.15)]} \times \frac{1 \text{ ft}}{12 \text{ in}} = 0.02990 \text{ ft}$$

Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = 975.19 \text{ ft}^3$$

V_{POST}= Post Development Volume of Storm Water Generated (ft³)

Solution: Volume Capture Requirement

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (975.19) - (293.54)$$

$$V_{DELTA} = 681.65 \text{ ft}^3$$

Where:

Delta Volume Capture= The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, must be achieved.

NOTE:

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

LID BMP Sizing Tool: 100% Volume Capture Goal: V_{GOAL}

Formulas:

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{2582.08 \text{ ft}^3}{}$$

Where:

$V_{LID\ GOAL}$ = Required volume of soil in LID BMP.

$A_{LID\ GOAL}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$A_{LID\ GOAL} = (W)(L) = \frac{504.00 \text{ ft}^2}{}$$

$$V_{GOAL} = 1,149 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.4

D = 5.1 ft

W = 6.0 ft

L = 84.0 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

Solution:

$$\text{Percent of Goal Achieved} = 100.33\% = \left[\frac{(5.1 \times 504)}{2,582} \right] \times 100$$

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA}

Formulas:

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{\#DIV/0! \text{ ft}^3}{}$$

Where:

$V_{LID\ DELTA}$ = Required volume of soil in LID BMP

$A_{LID\ DELTA}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$A_{LID\ DELTA} = (W)(L) = \frac{0.00 \text{ ft}^2}{}$$

$$V_{DELTA} = 681.65 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.0

D = 0.0 ft

W = 0.0 ft

L = 0.0 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

Solution:

$$\text{Percent of Requirement Achieved} = \#DIV/0! \%$$

$$= \#DIV/0!$$

INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project: 4081.02
Address/Location: Emerald Isle
Designer: Brejle & Race Consulting Engineers
Date: March 20, 2017
Inlet Number/Tributary Area/BMP: 2

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 11,768 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains ^[1]**Input:**

Select disconnection condition:
Condition Factor = 0.25

Runoff is directed across landscape. Width of area: 5' to 9'

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²
Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

$$(11,768 \times 0.25 \times 0.00) = 0.00 \text{ ft}^2$$

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

Units per Acre

Select Density: 3-4
Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

$$(11,768 \times 0.25 \times 0.00 \times 0.19) = 0.00 \text{ ft}^2$$

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).



APPENDIX C STORM WATER CALCULATOR

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area** 1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for *existing tree canopy*=

0 ft²

Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction = 0 ft² = Sum of areas managed by evergreen + deciduous + existing canopy

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 11,768 ft²

Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²

Reduced Tributary Area to be used for Calculations = 11,768 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:

S = Potential maximum retention after runoff (in)⁽⁶⁾
CN = Curve Number⁽⁵⁾

$$Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8+S)]} \times \frac{1ft}{12"} \quad \text{Where:}$$

Q = Runoff depth (ft)⁽⁶⁾
P = Precipitation (in) = 1.00
K = Seasonal Precipitation Factor⁽⁷⁾

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

$$V = (Q)(A_r)$$

Where:

V = Volume of Storm Water to be Retained (ft³)
A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A_r = 11,768 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select post development ground cover description⁽⁹⁾ = **Impervious - Paved Parking, Rooftop, Driveways**

OR: CN_{POST} = 98
Composite post development CN⁽⁹⁾ = 89

NOTE:
Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Solution:

Volume of storm water - Post Development

S_{POST} = 1.19 in

S_{POST} = 1000 / 89 - 10

Where:

S_{POST} = Post development potential maximum retention after runoff (in).

Q_{POST} = 0.03411 ft

$$Q_{POST} = \frac{[(1.00 * 1.17) + (0.2 * 1.19)]^2}{[(1.00 * 1.17) + (0.8 * 1.19)]} \times \frac{1ft}{12in}$$

Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

V_{GOAL} = 401.41 ft³

V_{GOAL} = (0.03411)(11,768)

V_{GOAL} = Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$$Q_{\text{TREATMENT}} = (0.2 \text{ in/hr})(A_r)(C_{\text{POST}})(K) \text{ cfs}$$

Where:

$Q_{\text{TREATMENT}}$ = Design flow rate required to be treated (cfs)

C_{POST} = Rational method runoff coefficient for the developed condition^[10]

A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres)

K = Seasonal Precipitation Factor^[7]

Input:

$$A_r = 11,768 \text{ ft}^2 = 0.27016 \text{ Acres}$$

$$C_{\text{POST}}^{[10]} = 0.44$$

$$K^{[7]} = 1.2$$

Solution:

$$Q_{\text{TREATMENT}} = 0.02782 \text{ cfs}$$

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, **must** be achieved.

C value note:

The C value used for this calculation is smaller than the value used for hydraulic Flood Control design.

The table of values can be found here

This smaller value should not be used to size the overflow bypass.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:

S= Potential maximum retention after runoff (in)^[6]
CN= Curve Number^[5]

$$Q = \frac{[(P-K) - (0.2 \cdot S)]^2}{[(P-K) + (0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:

Q= Runoff depth (ft)^[6]
P= Precipitation (in) = 0.92 0.92 Inches in the Santa Rosa area, based on local historical data.
K= Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:

V= Volume of Storm Water to be Retained (ft³)
A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

$$A_r = \frac{11,768 \text{ ft}^2}{K^{[7]} = 1.2}$$

Drop down Lists

Select hydrologic soil type within tributary area^[8] = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select predevelopment ground cover description^[9] = **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**
Select post development ground cover description^[9] = **Impervious - Paved Parking, Rooftop, Driveways**

$$\begin{array}{l} CN_{PRE} = 82 \\ CN_{POST} = 98 \\ \text{OR} \\ \text{Composite Predevelopment } CN^{[9]} = 80 \\ \text{Composite Post development } CN^{[9]} = 89 \end{array}$$

Solution:

Pre Development Storm Water Runoff Volume

$$S_{PRE} = \frac{1000}{2.50} - 10$$

$$S_{PRE} = \frac{1000}{80} - 10$$

Where:

S_{PRE}= Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = 0.00900 \text{ ft}$$

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{PRE}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = 105.91 \text{ ft}^3$$

$$V_{PRE} = (0.00900)(11,768)$$

V_{PRE}= Pre Development Volume of Storm Water Generated (ft³)

Post Development Storm Water Runoff Volume

$$S_{POST} = \frac{1.19 \text{ in}}{1.19 \text{ in}}$$

$$S_{POST} = \frac{1000}{89} - 10$$

Where:

S_{POST}= Post development potential maximum retention after runoff (in).

$$Q_{POST} = 0.02888 \text{ ft}$$

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 1.19)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 1.19)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = 339.86 \text{ ft}^3$$

$$V_{POST} = (0.02888)(11,768)$$

V_{POST}= Post Development Volume of Storm Water Generated (ft³)

Solution: Volume Capture Requirement

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (339.86) - (105.91)$$

Where:

Delta Volume Capture= The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

$$V_{DELTA} = 233.95 \text{ ft}^3$$

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, must be achieved.

NOTE:

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

LID BMP Sizing Tool: 100% Volume Capture Goal; V_{GOAL} **Formulas:**

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{587.45 \text{ ft}^3}{}$$

$$A_{LID\ GOAL} = (W)(L) = \frac{144.00 \text{ ft}^2}{}$$

Where:
 $V_{LID\ GOAL}$ = Required volume of soil in LID BMP.
 $A_{LID\ GOAL}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{GOAL} = \frac{401 \text{ ft}^3}{}$$

Where:

P = Porosity (enter as a decimal)
D = Depth below perforated pipe if present (in decimal feet)
W = Width (in decimal feet)
L = Length (in decimal feet)

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

Input:

P = 0.7 as a decimal
D = 4.4 ft Below perforated pipe if present
W = 8.0 ft
L = 18.0 ft

Solution:

$$\text{Percent of Goal Achieved} = \frac{107.86 \%}{} = [(4.4 \times 144) / 587] \times 100$$

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

NOTE:

LID Sizing Tool only applicable for volume based BMPs. Not required if site requires treatment only.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA} **Formulas:**

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{\#DIV/0! \text{ ft}^3}{}$$

$$A_{LID\ DELTA} = (W)(L) = \frac{0.00 \text{ ft}^2}{}$$

Where:
 $V_{LID\ DELTA}$ = Required volume of soil in LID BMP
 $A_{LID\ DELTA}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{DELTA} = \frac{233.95 \text{ ft}^3}{}$$

Where:

P = Porosity (enter as a decimal)
D = Depth below perforated pipe if present (in decimal feet)
W = Width (in decimal feet)
L = Length (in decimal feet)

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

Input:

P = 0.0 as a decimal
D = 0.0 ft Below perforated pipe if present
W = 0.0 ft
L = 0.0 ft

Solution:

$$\text{Percent of Requirement Achieved} = \frac{\#DIV/0! \%}{} = \#DIV/0!$$

INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.

NOTE:

LID Sizing Tool only applicable for volume based BMPs. Not required if site requires treatment only.

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project: 4081.02
Address/Location: Emerald Isle
Designer: Brejle & Race Consulting Engineers
Date: March 20, 2017
Inlet Number/Tributary Area/BMP: 3

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 26,139 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains^[1]**Input:**

Select disconnection condition:

Runoff is directed across landscape: Width of area: 5' to 9'

Condition Factor = 0.25

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²

Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

$$(26,139 \times 0.25 \times 0.00) = 0.00 \text{ ft}^2$$

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

Select Density: 3-4

Units per Acre

Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

$$(26,139 \times 0.25 \times 0.00 \times 0.19) = 0.00 \text{ ft}^2$$

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area** 1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for existing tree canopy= 0 ft² Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction = 0 ft² = Sum of areas managed by evergreen + deciduous + existing canopy

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 26,139 ft²

Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²

Reduced Tributary Area to be used for Calculations = 26,139 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:

S = Potential maximum retention after runoff (in)⁽⁶⁾
CN = Curve Number⁽⁵⁾

$$Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8+S)]} \times \frac{1ft}{12"} \quad \text{Where:}$$

Q = Runoff depth (ft)⁽⁶⁾
P = Precipitation (in) = 1.00
K = Seasonal Precipitation Factor⁽⁷⁾

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

$$V = (Q)(A_r)$$

Where:

V = Volume of Storm Water to be Retained (ft³)
A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A_r = 26,139 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select post development ground cover description⁽⁵⁾ = **Impervious - Paved Parking, Rooftop, Driveways**

OR: CN_{POST} = 81
Composite post development CN⁽⁹⁾ = 96

NOTE:
Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Solution:

Volume of storm water - Post Development

S_{POST} = 0.46025 in

S_{POST} = 1000 - 10
96

Where:

S_{POST} = Post development potential maximum retention after runoff (in).

Q_{POST} = 0.06297 ft

Q_{POST} = $\frac{[(1.00 * 1.17) - (0.2 * 0.46)]^2}{[(1.00 * 1.17) + (0.8 * 0.46)]} \times \frac{1ft}{12in}$

Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

V_{GOAL} = 1645.97 ft³

V_{GOAL} = (0.06297)(26,139)

V_{GOAL} = Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$Q_{TREATMENT} = (0.2 \text{ in/hr})(A_r)(C_{POST})(K) \text{ cfs}$

Where:

$Q_{TREATMENT}$ = Design flow rate required to be treated (cfs)

C_{POST} = Rational method runoff coefficient for the developed condition^[10]

A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres)

K = Seasonal Precipitation Factor^[7]

Input:

$A_r = 26,139 \text{ ft}^2 = 0.60007 \text{ Acres}$
 $C_{POST}^{[10]} = 0.72$
 $K^{[7]} = 1.2$

Solution:

$Q_{TREATMENT} = 0.10110 \text{ cfs}$

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, **must** be achieved.

C value note:

The C value used for this calculation is smaller than the value used for hydraulic Flood Control design.

The table of values can be found here

This smaller value should not be used to size the overflow bypass.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:S= Potential maximum retention after runoff (in)^[6]
CN= Curve Number^[5]

$$Q = \frac{[(P-K) - (0.2 \cdot S)]^2}{[(P-K) + (0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:Q= Runoff depth (ft)^[6]

P= Precipitation (in) = 0.92 0.92 Inches in the Santa Rosa area, based on local historical data.

K= Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:V= Volume of Storm Water to be Retained (ft³)A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)**Input:** (Pick data from drop down lists or enter calculated values)

A _r =	26,139 ft ²
K ^[7] =	1.2

Drop down Lists

Select hydrologic soil type within tributary area^[8] = **D: 0 - 0.05 in/hr infiltration (transmission) rate**

Select predevelopment ground cover description^[9] = **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**

Select post development ground cover description^[9] = **Impervious - Paved Parking, Rooftop, Driveways**

CN _{PRE} =	82
CN _{POST} =	98
Composite Predevelopment CN ^[9] =	80
Composite Post development CN ^[9] =	96

ORComposite Predevelopment CN^[9] = 80
Composite Post development CN^[9] = 96**Solution:****Pre Development Storm Water Runoff Volume**

$$S_{PRE} = \frac{1000}{80} - 10 = 2.50 \text{ in}$$

$$S_{PRE} = \frac{1000}{80} - 10$$

Where:S_{PRE}= Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}} = 0.00900 \text{ ft}$$

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{PRE}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = (0.00900)(26,139) = 235.25 \text{ ft}^3$$

$$V_{PRE} = (0.00900)(26,139)$$

V_{PRE}= Pre Development Volume of Storm Water Generated (ft³)**Post Development Storm Water Runoff Volume**

$$S_{POST} = \frac{1000}{96} - 10 = 0.46025 \text{ in}$$

$$S_{POST} = \frac{1000}{96} - 10$$

Where:S_{POST}= Post development potential maximum retention after runoff (in).

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 0.46)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.46)]} \times \frac{1 \text{ ft}}{12 \text{ in}} = 0.05591 \text{ ft}$$

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 0.46)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.46)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = (0.05591)(26,139) = 1461.43 \text{ ft}^3$$

$$V_{POST} = (0.05591)(26,139)$$

V_{POST}= Post Development Volume of Storm Water Generated (ft³)**Solution: Volume Capture Requirement**

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (1,461.43) - (235.25)$$

$$V_{DELTA} = 1226.18 \text{ ft}^3$$

Where:

Delta Volume Capture= The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, must be achieved.

NOTE:

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

LID BMP Sizing Tool: 100% Volume Capture Goal: V_{GOAL}

Formulas:

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{2212.33 \text{ ft}^3}{0.7}$$

$$A_{LID\ GOAL} = (W)(L) = \frac{504.00 \text{ ft}^2}{14.0 \text{ ft}}$$

Where:
 $V_{LID\ GOAL}$ = Required volume of soil in LID BMP.
 $A_{LID\ GOAL}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{GOAL} = 1,646 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)
D = Depth below perforated pipe if present (in decimal feet)
W = Width (in decimal feet)
L = Length (in decimal feet)

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

Input:

P = 0.7
D = 4.4 ft
W = 14.0 ft
L = 36.0 ft

as a decimal
Below perforated pipe if present

Solution:

$$\text{Percent of Goal Achieved} = 100.24\%$$

$$= [(4.4 \times 504) / 2,212] \times 100$$

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA}

Formulas:

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{\#DIV/0! \text{ ft}^3}{0.00}$$

$$A_{LID\ DELTA} = (W)(L) = \frac{0.00 \text{ ft}^2}{0.00 \text{ ft}}$$

Where:
 $V_{LID\ DELTA}$ = Required volume of soil in LID BMP
 $A_{LID\ DELTA}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{DELTA} = 1226.18 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)
D = Depth below perforated pipe if present (in decimal feet)
W = Width (in decimal feet)
L = Length (in decimal feet)

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

Input:

P = 0.0
D = 0.0 ft
W = 0.0 ft
L = 0.0 ft

as a decimal
Below perforated pipe if present

Solution:

$$\text{Percent of Requirement Achieved} = \#DIV/0!\%$$

$$= \#DIV/0!$$

INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project: 4081.02
Address/Location: Emerald Isle
Designer: Brejle & Race Consulting Engineers
Date: March 20, 2017
Inlet Number/Tributary Area/BMP: 4

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 6.883 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains^[1]**Input:**

Select disconnection condition:

Runoff is directed across landscape; Width of area: 5' to 9'

Condition Factor = 0.25

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²

Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

(6.883 x 0.25 x 0.00) = 0.00 ft²

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

Units per Acre

Select Density: 3-4

Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

(6.883 x 0.25 x 0.00 x 0.19) = 0.00 ft²

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).



APPENDIX C
STORM WATER CALCULATOR

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area**
1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction = **0.00** ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for *existing tree canopy*=

0 ft²

Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction = **0** ft² = Sum of areas managed by evergreen + deciduous + existing canopy

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = **0.00** ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 6,883 ft²

Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²

Reduced Tributary Area to be used for Calculations = 6,883 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:

S = Potential maximum retention after runoff (in)⁽⁶⁾
CN = Curve Number⁽⁵⁾

$$Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8+S)]} \times \frac{1ft}{12"} \quad \text{Where:}$$

Q = Runoff depth (ft)⁽⁶⁾
P = Precipitation (in) = 1.00
K = Seasonal Precipitation Factor⁽⁷⁾

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

$$V = (Q)(A_r)$$

Where:

V = Volume of Storm Water to be Retained (ft³)
A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A_r = 6,883 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select post development ground cover description⁽⁵⁾ = **Impervious - Paved Parking, Rooftop, Driveways**

OR: CN_{POST} = 81
Composite post development CN⁽⁹⁾ = 81

NOTE:
Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Solution:

Volume of storm water - Post Development

S_{POST} = 2.29 in

S_{POST} = 1000 / 81 - 10

Where:

S_{POST} = Post development potential maximum retention after runoff (in).

Q_{POST} = 0.01407 ft

Q_{POST} = $\frac{[(1.00 * 1.17) - (0.2 * 2.29)]^2}{[(1.00 * 1.17) + (0.8 * 2.29)]} \times \frac{1ft}{12in}$

Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

V_{GOAL} = 96.84 ft³

V_{GOAL} = (0.01407)(6,883)

V_{GOAL} = Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$$Q_{\text{TREATMENT}} = (0.2 \text{ in/hr})(A_r)(C_{\text{POST}})(K) \text{ cfs}$$

Where:

 $Q_{\text{TREATMENT}}$ = Design flow rate required to be treated (cfs) C_{POST} = Rational method runoff coefficient for the developed condition^[10] A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres) K = Seasonal Precipitation Factor^[7]**Input:**

$$\begin{array}{l} A_r = 6.883 \text{ ft}^2 = 0.15801 \text{ Acres} \\ C_{\text{POST}}^{[10]} = 0.16 \\ K^{[7]} = 1.2 \end{array}$$

Solution:

$$Q_{\text{TREATMENT}} = 0.00577 \text{ cfs}$$

$$Q_{\text{TREATMENT}} = (0.2)(0.1580)(0.16)(1.17)$$

C value note:

The C value used for this calculation is smaller than the value used for hydraulic Flood Control design.

[The table of values can be found here](#)

This smaller value should not be used to size the overflow bypass.

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, **must** be achieved.

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:

S= Potential maximum retention after runoff (in)^[6]
CN= Curve Number^[5]

$$Q = \frac{[(P-K) \cdot (0.2 \cdot S)]^2}{[(P-K) + (0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:

Q= Runoff depth (ft)^[6]
P= Precipitation (in) = 0.92 0.92 Inches in the Santa Rosa area, based on local historical data.
K= Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:

V= Volume of Storm Water to be Retained (ft³)
A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A _r =	6.883 ft ²
K ^[7] =	1.2

Drop down Lists

Select hydrologic soil type within tributary area^[8] = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select predevelopment ground cover description^[9] = **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**
Select post development ground cover description^[9] = **Impervious - Paved Parking, Rooftop, Driveways**

CN _{PRE} =	82
CN _{POST} =	98
Composite Predevelopment CN ^[9] =	80
Composite Post development CN ^[9] =	81

OR

Composite Predevelopment CN^[9] = 80
Composite Post development CN^[9] = 81

Solution:

Pre Development Storm Water Runoff Volume

$$S_{PRE} = 2.50 \text{ in}$$

$$S_{PRE} = \frac{1000}{80} - 10$$

Where:

S_{PRE}= Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = 0.00900 \text{ ft}$$

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) \cdot (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{PRE}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = 61.95 \text{ ft}^3$$

$$V_{PRE} = (0.00900)(6.883)$$

V_{PRE}= Pre Development Volume of Storm Water Generated (ft³)

Post Development Storm Water Runoff Volume

$$S_{POST} = 2.29 \text{ in}$$

$$S_{POST} = \frac{1000}{81} - 10$$

Where:

S_{POST}= Post development potential maximum retention after runoff (in).

$$Q_{POST} = 0.01096 \text{ ft}$$

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) \cdot (0.2 \cdot 2.29)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.29)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = 75.44 \text{ ft}^3$$

$$V_{POST} = (0.01096)(6.883)$$

V_{POST}= Post Development Volume of Storm Water Generated (ft³)

Solution: Volume Capture Requirement

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (75.44) - (61.95)$$

Where:

Delta Volume Capture= The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

$$V_{DELTA} = 13.49 \text{ ft}^3$$

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, must be achieved.

NOTE:

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

LID BMP Sizing Tool: 100% Volume Capture Goal: V_{GOAL}

Formulas:

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{217.63 \text{ ft}^3}{}$$

Where:

$V_{LID\ GOAL}$ = Required volume of soil in LID BMP.

$A_{LID\ GOAL}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$A_{LID\ GOAL} = (W)(L) = \frac{73.00 \text{ ft}^2}{}$$

$$V_{GOAL} = \frac{97 \text{ ft}^3}{}$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

P = 0.4

D = 3.0 ft

W = 2.0 ft

L = 36.5 ft

as a decimal

Below perforated pipe if present

Input:

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

Solution:

$$\text{Percent of Goal Achieved} = \frac{100.63 \%}{} = \left[\frac{(3.0 \times 73)}{218} \right] \times 100$$

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA}

Formulas:

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{\#DIV/0! \text{ ft}^3}{}$$

Where:

$V_{LID\ DELTA}$ = Required volume of soil in LID BMP

$A_{LID\ DELTA}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$A_{LID\ DELTA} = (W)(L) = \frac{0.00 \text{ ft}^2}{}$$

$$V_{DELTA} = \frac{13.49 \text{ ft}^3}{}$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

P = 0.0

D = 0.0 ft

W = 0.0 ft

L = 0.0 ft

as a decimal

Below perforated pipe if present

Input:

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

Solution:

$$\text{Percent of Requirement Achieved} = \frac{\#DIV/0! \%}{} = \#DIV/0!$$

INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project: 4081.02
Address/Location: Emerald Isle
Designer: Brejle & Race Consulting Engineers
Date: March 20, 2017
Inlet Number/Tributary Area/BMP: 5

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 20,155 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains^[1]**Input:**

Select disconnection condition:

Runoff is directed across landscape; Width of area: 5' to 9'

Condition Factor = 0.25

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²

Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

(20,155 x 0.25 x 0.00) = 0.00 ft²

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

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Units per Acre

Select Density: 3-4

Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

(20,155 x 0.25 x 0.00 x 0.19) = 0.00 ft²

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area** 1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction =

0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for *existing tree canopy*=

0 ft²

Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction =

0 ft²

= Sum of areas managed by evergreen + deciduous + existing canopy

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 20,155 ft²
Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²
Reduced Tributary Area to be used for Calculations = 20,155 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$S = \frac{1000}{CN} - 10$ Where: S = Potential maximum retention after runoff (in)⁽⁶⁾
CN = Curve Number⁽⁵⁾
 $Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8 \cdot S)]} \times \frac{1ft}{12"} \times 1.00$ Where: Q = Runoff depth (ft)⁽⁶⁾
P = Precipitation (in) = 1.00
K = Seasonal Precipitation Factor⁽⁷⁾
 $V = (Q)(A_r)$ Where: V = Volume of Storm Water to be Retained (ft³)
A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

Input: (Pick data from drop down lists or enter calculated values)

A_r = 20,155 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select post development ground cover description⁽⁵⁾ = **Impervious - Paved Parking, Rooftop, Driveways**

OR: Composite post development CN⁽⁹⁾ = 96

NOTE:
Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Volume of storm water - Post Development

$S_{POST} = \frac{1000}{96} - 10$ Where: S_{POST} = Post development potential maximum retention after runoff (in).
 $Q_{POST} = \frac{[(1.00 \cdot 1.17) + (0.2 \cdot 0.40)]^2}{[(1.00 \cdot 1.17) + (0.8 \cdot 0.40)]} \times \frac{1ft}{12in}$ Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.
 $V_{GOAL} = (0.06645)(20,155)$ V_{GOAL} = Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$$\underline{Q_{\text{TREATMENT}}} = (0.2 \text{ in/hr})(A_r)(C_{\text{POST}})(K) \text{ cfs}$$

Where:

$Q_{\text{TREATMENT}}$ = Design flow rate required to be treated (cfs)

C_{POST} = Rational method runoff coefficient for the developed condition^[10]

 A_i = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres)

K = Seasonal Precipitation Factor^[7]

Input:

$A_r =$	20,155	$\text{ft}^2 =$	0.46270 Acres
$C_{\text{POST}}^{[10]} =$	0.74		
$K^{[7]} =$	1.2		

Solution:

$$Q_{\text{TREATMENT}} = 0.08012 \text{ cfs} \quad Q_{\text{TREATMENT}} = (0.2)(0.4627)(0.74)(1.17)$$

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved, then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, **must** be achieved.

C value note:

The C value used for this calculation is smaller than the value used for hydraulic Flood Control design.

The table of values can be found here

This smaller value should not be used to size the overflow bypass.

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:

S= Potential maximum retention after runoff (in)^[6]
CN= Curve Number^[5]

$$Q = \frac{[(P-K) \cdot (0.2 \cdot S)]^2}{[(P-K) + (0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:

Q= Runoff depth (ft)^[6]
P= Precipitation (in) = 0.92 0.92 Inches in the Santa Rosa area, based on local historical data.
K= Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:

V= Volume of Storm Water to be Retained (ft³)
A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A _r =	20,155 ft ²
K ^[7] =	1.2

Drop down Lists

Select hydrologic soil type within tributary area^[8] = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select predevelopment ground cover description^[9] = **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**
Select post development ground cover description^[9] = **Impervious - Paved Parking, Rooftop, Driveways**

CN _{PRE} =	82
CN _{POST} =	98
Composite Predevelopment CN ^[9] =	80
Composite Post development CN ^[9] =	96

OR

Composite Predevelopment CN^[9] = 80
Composite Post development CN^[9] = 96

Solution:

Pre Development Storm Water Runoff Volume

$$S_{PRE} = 2.50 \text{ in}$$

$$S_{PRE} = \frac{1000}{80} - 10$$

Where:

S_{PRE}= Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = 0.00900 \text{ ft}$$

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) \cdot (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{PRE}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = 181.40 \text{ ft}^3$$

$$V_{PRE} = (0.00900)(20,155)$$

V_{PRE}= Pre Development Volume of Storm Water Generated (ft³)

Post Development Storm Water Runoff Volume

$$S_{POST} = 0.39501 \text{ in}$$

$$S_{POST} = \frac{1000}{96} - 10$$

Where:

S_{POST}= Post development potential maximum retention after runoff (in).

$$Q_{POST} = 0.05925 \text{ ft}$$

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) \cdot (0.2 \cdot 0.40)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.40)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = 1194.18 \text{ ft}^3$$

$$V_{POST} = (0.05925)(20,155)$$

V_{POST}= Post Development Volume of Storm Water Generated (ft³)

Solution: Volume Capture Requirement

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (1,194.18) - (181.40)$$

Where:

Delta Volume Capture= The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

$$V_{DELTA} = 1012.79 \text{ ft}^3$$

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, must be achieved.

NOTE:

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

LID BMP Sizing Tool: 100% Volume Capture Goal: V_{GOAL}

Formulas:

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{3348.25}{100} \text{ ft}^3$$

$$A_{LID\ GOAL} = (W)(L) = 1384.50 \text{ ft}^2$$

Where:

$V_{LID\ GOAL}$ = Required volume of soil in LID BMP.

$A_{LID\ GOAL}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{GOAL} = 1,339 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.4

D = 2.4 ft

W = 6.5 ft

L = 213.0 ft

as a decimal

Below perforated pipe if present

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

Solution:

$$\text{Percent of Goal Achieved} = 100.07\%$$

$$= [(2.4 \times 1,385) / 3,348] \times 100$$

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA}

Formulas:

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{\#DIV/0!}{100} \text{ ft}^3$$

$$A_{LID\ DELTA} = (W)(L) = 0.00 \text{ ft}^2$$

Where:

$V_{LID\ DELTA}$ = Required volume of soil in LID BMP

$A_{LID\ DELTA}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{DELTA} = 1012.79 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.0

D = 0.0 ft

W = 0.0 ft

L = 0.0 ft

as a decimal

Below perforated pipe if present

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

Solution:

$$\text{Percent of Requirement Achieved} = \#DIV/0!\%$$

$$= \#DIV/0!$$

INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project: 4081.02
Address/Location: Emerald Isle
Designer: Brejle & Race Consulting Engineers
Date: March 20, 2017
Inlet Number/Tributary Area/BMP: 6

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 23,040 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains^[1]**Input:**

Select disconnection condition:

Runoff is directed across landscape; Width of area: 5' to 9'

Condition Factor = 0.25

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²

Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

(23,040 x 0.25 x 0.00) = 0.00 ft²

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

Select Density: 3-4

Units per Acre

Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

(23,040 x 0.25 x 0.00 x 0.19) = 0.00 ft²

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area** 1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for *existing tree canopy*=

0 ft²

Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction = 0 ft² = Sum of areas managed by evergreen + deciduous + existing canopy

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 23,040 ft²

Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²

Reduced Tributary Area to be used for Calculations = 23,040 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$S = \frac{1000}{CN} - 10$ Where: S = Potential maximum retention after runoff (in)⁽⁶⁾
CN = Curve Number⁽⁵⁾

$Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8 \cdot S)]} \times \frac{1ft}{12"} \times 1.00$ Where: Q = Runoff depth (ft)⁽⁶⁾
P = Precipitation (in) = 1.00
K = Seasonal Precipitation Factor⁽⁷⁾

$V = (Q/A_s)$ Where: V = Volume of Storm Water to be Retained (ft³)
A_s = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

Input: (Pick data from drop down lists or enter calculated values)

A_s = 23,040 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = D: 0 - 0.05 in/hr infiltration (transmission) rate
Select post development ground cover description⁽⁵⁾ = Impervious - Paved Parking, Rooftop, Driveways

OR: CN_{POST} = 98
Composite post development CN⁽⁹⁾ = 95

NOTE:
Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Volume of storm water - Post Development

$S_{POST} = \frac{1000}{95} - 10$ Where: S_{POST} = Post development potential maximum retention after runoff (in).

$Q_{POST} = \frac{[(1.00 \cdot 1.17) - (0.2 \cdot 0.55)]^2}{[(1.00 \cdot 1.17) + (0.8 \cdot 0.55)]} \times \frac{1ft}{12in}$ Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

V_{GOAL} = (0.05816)(23,040) V_{GOAL} = Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$$Q_{\text{TREATMENT}} = (0.2 \text{ in/hr})(A_r)(C_{\text{POST}})(K) \text{ cfs}$$

Where:

$Q_{TREATMENT}$ = Design flow rate required to be treated (cfs)

C_{POST} = Rational method runoff coefficient for the developed condition^[10]

A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres)

K = Seasonal Precipitation Factor^[7]

Input:

$A_t =$	23,040	$\text{ft}^2 =$	0.52893	Acres
$C_{\text{POST}}^{[10]} =$	0.69			
$K^{[7]} =$	1.2			

Solution:

$$Q_{\text{TREATMENT}} = 0.2(0.5289)(0.69)(1.17)$$

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, **must** be achieved.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:

S = Potential maximum retention after runoff (in)^[6]
CN = Curve Number^[5]

$$Q = \frac{[(P-K)-(0.2 \cdot S)]^2}{[(P-K)+(0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:

Q = Runoff depth (ft)^[6]
P = Precipitation (in) = 0.92 0.92 Inches in the Santa Rosa area, based on local historical data.
K = Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:

V = Volume of Storm Water to be Retained (ft³)
A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A _r =	23,040 ft ²
K ^[7] =	1.2

Drop down Lists

Select hydrologic soil type within tributary area^[8] = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select predevelopment ground cover description^[9] = **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**
Select post development ground cover description^[9] = **Impervious - Paved Parking, Rooftop, Driveways**

CN _{PRE} =	82
CN _{POST} =	98
Composite Predevelopment CN ^[9] =	80
Composite Post development CN ^[9] =	95

OR

Composite Predevelopment CN^[9] = 80
Composite Post development CN^[9] = 95

Solution:

Pre Development Storm Water Runoff Volume

$$S_{PRE} = \frac{1000}{2.50} - 10$$

$$S_{PRE} = \frac{1000}{80} - 10$$

Where:

S_{PRE} = Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{PRE} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = \frac{207.36 \text{ ft}^3}{(0.00900)/(23,040)}$$

$$V_{PRE} = (0.00900)/(23,040)$$

V_{PRE} = Pre Development Volume of Storm Water Generated (ft³)

Post Development Storm Water Runoff Volume

$$S_{POST} = \frac{1000}{0.54741} - 10$$

$$S_{POST} = \frac{1000}{95} - 10$$

Where:

S_{POST} = Post development potential maximum retention after runoff (in).

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 0.55)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.55)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 0.55)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.55)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = \frac{1182.41 \text{ ft}^3}{(0.05132)/(23,040)}$$

$$V_{POST} = (0.05132)/(23,040)$$

V_{POST} = Post Development Volume of Storm Water Generated (ft³)

Solution: Volume Capture Requirement

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (1,182.41) - (207.36)$$

$$V_{DELTA} = 975.05 \text{ ft}^3$$

Where:
Delta Volume Capture = The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, must be achieved.

NOTE:

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

APPENDIX C STORM WATER CALCULATOR

LID BMP Sizing Tool: 100% Volume Capture Goal: V_{GOAL}

Formulas:

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{1845.74 \text{ ft}^3}{0.7}$$

$$A_{LID\ GOAL} = (W)(L) = \frac{420.00 \text{ ft}^2}{10.0 \text{ ft}}$$

$$V_{GOAL} = 1,340 \text{ ft}^3$$

Where:

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

P= Porosity (enter as a decimal)

D= Depth below perforated pipe if present (in decimal feet)

W= Width (in decimal feet)

L= Length (in decimal feet)

Input:

P = 0.7

D = 4.4 ft

W = 10.0 ft

L = 42.0 ft

as a decimal

Below perforated pipe if present

Solution:

$$\text{Percent of Goal Achieved} = 100.12 \%$$

$$= [(4.4 \times 420) / 1,846] \times 100$$

NOTE:

LID Sizing Tool only applicable for volume based BMPs. Not required if site requires treatment only.

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA}

Formulas:

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{\#DIV/0! \text{ ft}^3}{0.0}$$

$$A_{LID\ DELTA} = (W)(L) = \frac{0.00 \text{ ft}^2}{0.00 \text{ ft}}$$

$$V_{DELTA} = 975.05 \text{ ft}^3$$

Where:

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

P= Porosity (enter as a decimal)

D= Depth below perforated pipe if present (in decimal feet)

W= Width (in decimal feet)

L= Length (in decimal feet)

Input:

P = 0.0

D = 0.0 ft

W = 0.0 ft

L = 0.0 ft

as a decimal

Below perforated pipe if present

Solution:

$$\text{Percent of Requirement Achieved} = \#DIV/0! \%$$

$$= \#DIV/0!$$

NOTE:

LID Sizing Tool only applicable for volume based BMPs. Not required if site requires treatment only.

INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project: 4081.02
Address/Location: Emerald Isle
Designer: Brejle & Race Consulting Engineers
Date: March 20, 2017
Inlet Number/Tributary Area/BMP: 7

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 5.828 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains^[1]**Input:**

Select disconnection condition:

Runoff is directed across landscape; Width of area: 5' to 9'

Condition Factor = 0.25

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²

Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

(5.828 x 0.25 x 0.00) = 0.00 ft²

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

Select Density: 3-4

Units per Acre

Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

(5.828 x 0.25 x 0.00 x 0.19) = 0.00 ft²

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area** 1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for *existing tree canopy*=

0 ft²

Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction = 0 ft² = Sum of areas managed by evergreen + deciduous + existing canopy

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 5,828 ft²

Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²

Reduced Tributary Area to be used for Calculations = 5,828 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$S = \frac{1000}{CN} - 10$

Where:

S= Potential maximum retention after runoff (in)⁽⁶⁾
CN= Curve Number⁽⁵⁾

$Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8+S)]} \times \frac{1ft}{12"} \quad \text{Where:}$

Q= Runoff depth (ft)⁽⁶⁾
P= Precipitation (in) = 1.00
K= Seasonal Precipitation Factor⁽⁷⁾

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

$V = (Q)(A_r)$

Where:

V= Volume of Storm Water to be Retained (ft³)
A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A_r = 5,828 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select post development ground cover description⁽⁵⁾ = **Impervious - Paved Parking, Rooftop, Driveways**

OR: Composite post development CN⁽⁹⁾ = 81

CN_{POST} = 90

NOTE:
Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Volume of storm water - Post Development

S_{POST} = 1.11 in

S_{POST} = 1000 / 90 - 10

Where:

S_{POST}= Post development potential maximum retention after runoff (in).

Q_{POST} = 0.03639 ft

Q_{POST} = $\frac{[(1.00 * 1.17) - (0.2 * 1.11)]^2}{[(1.00 * 1.17) + (0.8 * 1.11)]} \times \frac{1ft}{12in}$

Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

V_{GOAL} = 212.08 ft³

V_{GOAL} = (0.03639)(5,828)

V_{GOAL}= Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$$\underline{Q_{\text{TREATMENT}}} = (0.2 \text{ in/hr})(A_r)(C_{\text{POST}})(K) \text{ cfs}$$

Where:

$Q_{TREATMENT}$ = Design flow rate required to be treated (cfs)

C_{POST} = Rational method runoff coefficient for the developed condition^[10]

A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres)

K = Seasonal Precipitation Factor^[7]

Input:

A_r	5,828	ft ² = 0.13379 Acres
$C_{POST}^{[10]}$	0.51	
$K^{[7]}$	1.2	

Solution:

$Q_{\text{TREATMENT}} = 0.2(0.1338)(0.51)(1.17)$	$Q_{\text{TREATMENT}} = 0.01597 \text{ cfs}$
--	--

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, must be achieved.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:
S = Potential maximum retention after runoff (in)^[6]
CN = Curve Number^[5]

$$Q = \frac{[(P-K) - (0.2 \cdot S)]^2}{[(P-K) + (0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:

Q = Runoff depth (ft)^[6]

P = Precipitation (in) = 0.92 *0.92 inches in the Santa Rosa area, based on local historical data.*

K = Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:

V = Volume of Storm Water to be Retained (ft³)

A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A _r =	5,828 ft ²
K ^[7] =	1.2

Drop down Lists

Select hydrologic soil type within tributary area^[8] = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
 Select predevelopment ground cover description^[9] = **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**
 Select post development ground cover description^[9] = **Impervious - Paved Parking, Rooftop, Driveways**

CN _{PRE} =	82
CN _{POST} =	98
OR Composite Predevelopment CN ^[9] =	80
Composite Post development CN ^[9] =	90

Solution:

Pre Development Storm Water Runoff Volume

$$S_{PRE} = \frac{1000}{2.50} - 10$$

$$S_{PRE} = \frac{1000}{80} - 10$$

Where:
S_{PRE} = Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{PRE} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = \frac{52.45 \text{ ft}^3}{(0.00900) / (5,828)}$$

V_{PRE} = Pre Development Volume of Storm Water Generated (ft³)

Post Development Storm Water Runoff Volume

$$S_{POST} = \frac{1.11 \text{ in}}{1.11 \text{ in}}$$

$$S_{POST} = \frac{1000}{90} - 10$$

Where:
S_{POST} = Post development potential maximum retention after runoff (in).

$$Q_{POST} = \frac{0.03097 \text{ ft}}{[(0.92 \cdot 1.17) + (0.8 \cdot 1.11)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 1.11)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 1.11)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = \frac{180.49 \text{ ft}^3}{(0.03097) / (5,828)}$$

V_{POST} = Post Development Volume of Storm Water Generated (ft³)

Solution: Volume Capture Requirement

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (180.49) - (52.45)$$

Where:

Delta Volume Capture = The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

$$V_{DELTA} = 128.04 \text{ ft}^3$$

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, must be achieved.

NOTE:

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

LID BMP Sizing Tool: 100% Volume Capture Goal: V_{GOAL}

Formulas:

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{530.20}{100} \text{ ft}^3$$

$$A_{LID\ GOAL} = (W)(L) = 106.20 \text{ ft}^2$$

Where:

$V_{LID\ GOAL}$ = Required volume of soil in LID BMP.

$A_{LID\ GOAL}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{GOAL} = 212 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.4

D = 5.0 ft

W = 3.5 ft

L = 30.0 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

Solution:

$$\text{Percent of Goal Achieved} = 100.15\%$$

$$= [(5.0 \times 106) / 530] \times 100$$

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA}

Formulas:

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{\#DIV/0!}{100} \text{ ft}^3$$

$$A_{LID\ DELTA} = (W)(L) = 0.00 \text{ ft}^2$$

Where:

$V_{LID\ DELTA}$ = Required volume of soil in LID BMP

$A_{LID\ DELTA}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{DELTA} = 128.04 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.0

D = 0.0 ft

W = 0.0 ft

L = 0.0 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

Solution:

$$\text{Percent of Requirement Achieved} = \#DIV/0!\%$$

$$= \#DIV/0!$$

INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project:	4081.02
Address/Location:	Emerald Isle
Designer:	Brejle & Race Consulting Engineers
Date:	March 20, 2017
Inlet Number/Tributary Area/BMP:	8

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 14,107 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains^[1]**Input:**

Select disconnection condition:

Runoff is directed across landscape: Width of area: 5' to 9'

Condition Factor = 0.25

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²

Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

(14,107 x 0.25 x 0.00) = 0.00 ft²

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

Select Density:

3-4

Units per Acre

Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

(14,107 x 0.25 x 0.00 x 0.19) = 0.00 ft²

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area** 1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for *existing tree canopy*=

0 ft²

Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction = 0 ft² = Sum of areas managed by evergreen + deciduous + existing canopy

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 14,107 ft²

Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²

Reduced Tributary Area to be used for Calculations = 14,107 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:

S = Potential maximum retention after runoff (in)⁽⁶⁾
CN = Curve Number⁽⁵⁾

$$Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8+S)]} \times \frac{1ft}{12"} \quad \text{Where:}$$

Q = Runoff depth (ft)⁽⁶⁾
P = Precipitation (in) = 1.00
K = Seasonal Precipitation Factor⁽⁷⁾

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

$$V = (Q/A_s)$$

Where:

V = Volume of Storm Water to be Retained (ft³)
A_s = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A_s = 14,107 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select post development ground cover description⁽⁵⁾ = **Impervious - Paved Parking, Rooftop, Driveways**

OR: CN_{POST} = 81
Composite post development CN⁽⁹⁾ = 94

NOTE:
Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Solution:

Volume of storm water - Post Development

Page 8 of 9
S_{POST} = 0.59322 in

S_{POST} = 1000 - 10
94
Where: S_{POST} = Post development potential maximum retention after runoff (in).

Q_{POST} = 0.05617 ft

Q_{POST} = [(1.00 * 1.17) - (0.2 * 0.59)]² X 1ft
[(1.00 * 1.17) + (0.8 * 0.59)] 12in
Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

V_{GOAL} = 792.39 ft³

V_{GOAL} = (0.05617)(14,107)

V_{GOAL} = Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$$Q_{\text{TREATMENT}} = (0.2 \text{ in/hr})(A_r)(C_{\text{POST}})(K) \text{ cfs}$$

Where:

$Q_{\text{TREATMENT}}$ = Design flow rate required to be treated (cfs)

C_{POST} = Rational method runoff coefficient for the developed condition^[10]

A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres)

K = Seasonal Precipitation Factor^[7]

Input:

$$A_r = \frac{14,107 \text{ ft}^2}{43,560 \text{ ft}^2/\text{Acre}} = 0.32385 \text{ Acres}$$

$$C_{\text{POST}}^{[10]} = 0.59$$

$$K^{[7]} = 1.2$$

Solution:

$$Q_{\text{TREATMENT}} = (0.2)(0.3239)(0.59)(1.17) = 0.04494 \text{ cfs}$$

C value note:

The C value used for this calculation is smaller than the value used for hydraulic Flood Control design.

The table of values can be found here.

This smaller value should not be used to size the overflow bypass.

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, **must** be achieved.

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000 - 10}{CN}$$

Where:S= Potential maximum retention after runoff (in)^[6]
CN= Curve Number^[5]

$$Q = \frac{[(P-K) - (0.2 \cdot S)]^2}{[(P-K) + (0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:Q= Runoff depth (ft)^[6]
P= Precipitation (in) = 0.92 0.92 Inches in the Santa Rosa area, based on local historical data.
K= Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:V= Volume of Storm Water to be Retained (ft³)
A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)**Input:** (Pick data from drop down lists or enter calculated values)

A _r =	14,107 ft ²
K ^[7] =	1.2

Drop down ListsSelect hydrologic soil type within tributary area^[8] **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select predevelopment ground cover description^[9] **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**
Select post development ground cover description^[9] **Impervious - Paved Parking, Rooftop, Driveways**

CN _{PRE} =	82
CN _{POST} =	98
Composite Predevelopment CN ^[9] =	80
Composite Post development CN ^[9] =	94

ORComposite Predevelopment CN^[9] = 80
Composite Post development CN^[9] = 94**Solution:****Pre Development Storm Water Runoff Volume**

$$S_{PRE} = \frac{1000}{80} - 10$$

$$S_{PRE} = \frac{1000}{80} - 10$$

Where:S_{PRE}= Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{PRE}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = \frac{126.96 \text{ ft}^3}{14,107 \text{ ft}^2}$$

$$V_{PRE} = (0.00900)(14,107)$$

V_{PRE}= Pre Development Volume of Storm Water Generated (ft³)**Post Development Storm Water Runoff Volume**

$$S_{POST} = \frac{1000}{94} - 10$$

$$S_{POST} = \frac{1000}{94} - 10$$

Where:S_{POST}= Post development potential maximum retention after runoff (in).

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 0.59)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.59)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 0.59)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.59)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = \frac{697.31 \text{ ft}^3}{14,107 \text{ ft}^2}$$

$$V_{POST} = (0.04943)(14,107)$$

V_{POST}= Post Development Volume of Storm Water Generated (ft³)**Solution: Volume Capture Requirement**

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (697.31) - (126.96)$$

$$V_{DELTA} = 570.35 \text{ ft}^3$$

Where:

Delta Volume Capture= The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

INSTRUCTIONS:If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, **must** be achieved.**NOTE:**

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

APPENDIX C STORM WATER CALCULATOR

LID BMP Sizing Tool: 100% Volume Capture Goal: V_{GOAL}

Formulas:

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{1109.79}{0.7} \text{ ft}^3$$

$$A_{LID\ GOAL} = (W)(L) = \frac{260.00}{10.0} \text{ ft}^2$$

Where:

$V_{LID\ GOAL}$ = Required volume of soil in LID BMP.

$A_{LID\ GOAL}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{GOAL} = 792 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.7

D = 4.4 ft

W = 10.0 ft

L = 26.0 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

Solution:

$$\text{Percent of Goal Achieved} = \frac{103.08}{100} \times 100 = [(4.4 \times 260) / 1,110] \times 100$$

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

NOTE:

LID Sizing Tool only applicable for volume based BMPs. Not required if site requires treatment only.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA}

Formulas:

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{\#DIV/0!}{0.7} \text{ ft}^3$$

$$A_{LID\ DELTA} = (W)(L) = \frac{0.00}{10.0} \text{ ft}^2$$

Where:

$V_{LID\ DELTA}$ = Required volume of soil in LID BMP

$A_{LID\ DELTA}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{DELTA} = 570.35 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.0

D = 0.0 ft

W = 0.0 ft

L = 0.0 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

Solution:

$$\text{Percent of Requirement Achieved} = \frac{\#DIV/0!}{100} \times 100 = \#DIV/0!$$

INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.

NOTE:

LID Sizing Tool only applicable for volume based BMPs. Not required if site requires treatment only.

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project: 4081.02
Address/Location: Emerald Isle
Designer: Brejle & Race Consulting Engineers
Date: March 20, 2017
Inlet Number/Tributary Area/BMP: 9

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 6.043 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains^[1]**Input:**

Select disconnection condition:

Runoff is directed across landscape; Width of area: 5' to 9'

Condition Factor = 0.25

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²

Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

(6.043 x 0.25 x 0.00) = 0.00 ft²

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

Select Density: 3-4

Units per Acre

Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

(6.043 x 0.25 x 0.00 x 0.19) = 0.00 ft²

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area**
1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction = **0.00** ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for existing tree canopy=

0 ft²

Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction = **0** ft² = Sum of areas managed by evergreen + deciduous + existing canopy

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = **0.00** ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 6,043 ft²
Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²
Reduced Tributary Area to be used for Calculations = 6,043 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$S = \frac{1000}{CN} - 10$ Where: S= Potential maximum retention after runoff (in)⁽⁶⁾
CN= Curve Number⁽⁵⁾
 $Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8 \cdot S)]} \times \frac{1ft}{12"} \times 1.00$ Where: Q= Runoff depth (ft)⁽⁶⁾
P= Precipitation (in) = 1.00
K= Seasonal Precipitation Factor⁽⁷⁾
 $V = (Q)(A_r)$ Where: V= Volume of Storm Water to be Retained (ft³)
A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

Input: (Pick data from drop down lists or enter calculated values)

A_r = 6,043 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = D: 0 - 0.05 in/hr infiltration (transmission) rate
Select post development ground cover description⁽⁵⁾ = Impervious - Paved Parking, Rooftop, Driveways

OR: CN-POST = 81
Composite post development CN⁽⁹⁾ = 93

NOTE:
Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Solution:

Volume of storm water - Post Development

$S_{POST} = 0.75269 in$ Where: S_{POST}= Post development potential maximum retention after runoff (in).
 $Q_{POST} = 0.04898 ft$ Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.
 $V_{GOAL} = 295.99 ft^3$ V_{GOAL}= Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$$Q_{\text{TREATMENT}} = (0.2 \text{ in/hr})(A_r)(C_{\text{POST}})(K) \text{ cfs}$$

Where:

 $Q_{\text{TREATMENT}}$ = Design flow rate required to be treated (cfs) C_{POST} = Rational method runoff coefficient for the developed condition^[10] A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres) K = Seasonal Precipitation Factor^[7]**Input:**

$$\begin{array}{l} A_r = 6.043 \text{ ft}^2 = 0.13873 \text{ Acres} \\ C_{\text{POST}}^{[10]} = 0.63 \\ K^{[7]} = 1.2 \end{array}$$

Solution:

$$Q_{\text{TREATMENT}} = 0.02029 \text{ cfs}$$

$$Q_{\text{TREATMENT}} = (0.2)(0.1387)(0.63)(1.17)$$

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, **must** be achieved.

C value note:

The C value used for this calculation is smaller than the value used for hydraulic Flood Control design.

[The table of values can be found here](#)

This smaller value should not be used to size the overflow bypass.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:S= Potential maximum retention after runoff (in)^[6]
CN= Curve Number^[5]

$$Q = \frac{[(P-K) - (0.2 \cdot S)]^2}{[(P-K) + (0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:Q= Runoff depth (ft)^[6]

P= Precipitation (in) = 0.92 0.92 Inches in the Santa Rosa area, based on local historical data.

K= Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:V= Volume of Storm Water to be Retained (ft³)A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)**Input:** (Pick data from drop down lists or enter calculated values)

A _r =	6,043 ft ²
K ^[7] =	1.2

Drop down ListsSelect hydrologic soil type within tributary area^[8] **D: 0 - 0.05 in/hr infiltration (transmission) rate**Select predevelopment ground cover description^[9] **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**Select post development ground cover description^[9] **Impervious - Paved Parking, Rooftop, Driveways**

CN _{PRE} =	82
CN _{POST} =	98
Composite Predevelopment CN ^[9] =	80
Composite Post development CN ^[9] =	93

ORComposite Predevelopment CN^[9] = 80
Composite Post development CN^[9] = 93**Solution:****Pre Development Storm Water Runoff Volume**

$$S_{PRE} = \frac{1000}{98} - 10$$

$$S_{PRE} = \frac{1000}{80} - 10$$

Where:S_{PRE}= Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{PRE}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = \frac{54.39 \text{ ft}^3}{(0.00900) / (6,043)}$$

$$V_{PRE} = (0.00900) / (6,043)$$

V_{PRE}= Pre Development Volume of Storm Water Generated (ft³)**Post Development Storm Water Runoff Volume**

$$S_{POST} = \frac{1000}{93} - 10$$

$$S_{POST} = \frac{1000}{93} - 10$$

Where:S_{POST}= Post development potential maximum retention after runoff (in).

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 0.75)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.75)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 0.75)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.75)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = \frac{257.79 \text{ ft}^3}{(0.04266) / (6,043)}$$

$$V_{POST} = (0.04266) / (6,043)$$

V_{POST}= Post Development Volume of Storm Water Generated (ft³)**Solution: Volume Capture Requirement**

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (257.79) - (54.39)$$

Where:

Delta Volume Capture= The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

$$V_{DELTA} = 203.41 \text{ ft}^3$$

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, must be achieved.

NOTE:

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

LID BMP Sizing Tool: 100% Volume Capture Goal; V_{GOAL} **Formulas:**

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{672.70}{\text{ft}^3}$$

$$A_{LID\ GOAL} = (W)(L) = \frac{149.04}{\text{ft}^2}$$

Where:

 $V_{LID\ GOAL}$ = Required volume of soil in LID BMP. $A_{LID\ GOAL}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{GOAL} = \frac{296}{\text{ft}^3}$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.4

D = 4.5 ft

W = 3.2 ft

L = 46.0 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

Solution:

$$\text{Percent of Goal Achieved} = \frac{99.70}{\%} = [(4.5 \times 149) / 673] \times 100$$

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

NOTE:

LID Sizing Tool only applicable for volume based BMPs. Not required if site requires treatment only.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA} **Formulas:**

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{\#DIV/0!}{\text{ft}^3}$$

$$A_{LID\ DELTA} = (W)(L) = \frac{0.00}{\text{ft}^2}$$

Where:

 $V_{LID\ DELTA}$ = Required volume of soil in LID BMP $A_{LID\ DELTA}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{DELTA} = \frac{203.41}{\text{ft}^3}$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.0

D = 0.0 ft

W = 0.0 ft

L = 0.0 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

Solution:

$$\text{Percent of Requirement Achieved} = \frac{\#DIV/0!}{\%} = \#DIV/0!$$

INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.

NOTE:

LID Sizing Tool only applicable for volume based BMPs. Not required if site requires treatment only.

STORM WATER CALCULATOR*

*For example only, go to www.srcity.org/stormwaterlid for the latest version of the calculator

Project:	4081.02
Address/Location:	Emerald Isle
Designer:	Brejle & Race Consulting Engineers
Date:	March 20, 2017
Inlet Number/Tributary Area/BMP:	10

NOTE: In order for this calculator to function properly macros must be enabled.

Physical Tributary Area that drains to Inlet/BMP = 8.476 ft²

This portion of the Storm water Calculator is designed to account for pollution prevention measures implemented on site. Additional information and description of these measures can be found in the Fact Sheets in Appendix F and in Chapter 4 of the narrative.

Disconnected Roof Drains^[1]**Input:**

Select disconnection condition:

Runoff is directed across landscape; Width of area: 5' to 9'

Condition Factor = 0.25

Method 1: Based on the total rooftop drainage area - to be used if rooftop information is known.

Input:

Enter amount of rooftop area that drain to disconnected downspouts = 0 ft²

Rooftop Area Factor = 0.00

Rooftop Area Factor= (Total Rooftop Disconnected Area/Tributary Area)

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Rooftop Area Factor)

(8.476 x 0.25 x 0.00) = 0.00 ft²

Rooftop Drainage Area Reduction

Method 2: Based on density (units per acre) - to be used if rooftop information is unknown.

Input:

Enter percent of rooftop area to be disconnected from downspouts: 0 %

Select Density:

3-4

Units per Acre

Density Reduction Factor= 0.19

Solution:

Area reduction = (Physical Tributary Area x Conditional Factor x Percent Disconnected x Density Factor)

(8.476 x 0.25 x 0.00 x 0.19) = 0.00 ft²

Density Reduction

NOTE:
Either Method 1 (rooftop area) or Method 2 (density) can be used. Providing input for both methods will cause an error. If rooftop area information is available, Method 1 should be used.

[1] See "Impervious Area Disconnection" Fact Sheet in Appendix E for further details.

[2] See "Interceptor Trees" Fact Sheet in Appendix E for further details and see "Plant and Tree List" in Appendix G for approved trees.

[3] See "Vegetated Buffer Strip" and "Bovine Terrace" Fact Sheets in Appendix E for further details.

[4] Total area reductions due to pollution Prevention Measures cannot exceed 50% of the physical Tributary Area.

[5] Per the "Urban Hydrology For Small Watersheds" TR-55 manual.

[6] Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

[7] From Sonoma County Water Agency Flood Control Design Criteria.

[8] Hydrologic soil type based of infiltration rate of native soil as defined by "Urban Hydrology For Small Watersheds" TR-55 Manual.

[9] Composite CN calculated per "Worksheet 2 Part 1 of the Urban Hydrology For Small Watersheds" TR-55 manual.

[10] From "Using Site Design to Meet Development Standards For Storm water Quality" by the Bay Area Storm water Management Agencies Association (BASMAA).

Paved Area Disconnection ^[1]

Paved Area Type (select from drop down list):

Multiplier = **Not Directly-connected Paved Area** 1

Enter area of alternatively designed paved area:

0 ft²

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit for driveways designed to minimize runoff. Enter type and area of alternate design.

Interceptor Trees ^[2]

Number of new *Evergreen Trees* that qualify as interceptor trees=

0

New Evergreen Trees

Area Reduction due to new *Evergreen Trees*=

0 ft²

(200 ft²/tree)

Number of new *Deciduous Trees* that qualify as interceptor trees=

0

New Deciduous Trees

Area Reduction due to new *Deciduous Trees*=

0 ft²

(100 ft²/tree)

Enter square footage of qualifying *existing tree canopy* =

0

Existing Tree Canopy

Allowed reduction credit for existing tree canopy= 0 ft²
Allowed credit for existing tree canopy = 50 % of actual canopy square footage

Area Reduction = 0 ft² = Sum of areas managed by evergreen + deciduous + existing canopy

INSTRUCTIONS:

Calculates the area reductions credit due to interceptor trees. Includes both new and existing trees. Enter the number of new deciduous and evergreen trees and the canopy area of existing trees.

NOTE:

Total Interceptor Area Reduction is limited to 50% of the physical tributary area.

Buffer Strips & Bovine Terraces ^[3]

Enter area draining to a Buffer Strip or Bovine Terrace =

0 ft²

Buffer Factor =

0.7

Solution:

Area Reduction = (Area draining to Buffer Strip or Bovine Terrace) x (Buffer Factor) =

Area Reduction = 0.00 ft²

INSTRUCTIONS:

Calculates the area reduction credit due to buffer strips and/or bovine terraces. Runoff Must be direct to these features as sheet flow. Enter the area draining to these features.



APPENDIX C
STORM WATER CALCULATOR

Revised Tributary Area due to Pollution Prevention Measures

Physical Tributary Area = 8,476 ft²

Tributary Area Reduction due to Pollution Prevention Measures ⁽⁴⁾ = 0.00 ft²

Reduced Tributary Area to be used for Calculations = 8,476 ft²

This worksheet calculates the quantity of storm water that needs to be addressed (captured and/or treated) to comply with the NPDES Storm Water Permit issued to the City of Santa Rosa and County of Sonoma by the North Coast Regional Water Quality Control Board.

Design Goal: 100% Volume Capture

Capture (infiltration and/or reuse) of 100% of the volume of runoff generated by the 85th percentile 24 hour storm event.

Formulas:

$S = \frac{1000}{CN} - 10$

Where:

S = Potential maximum retention after runoff (in)⁽⁶⁾
CN = Curve Number⁽⁵⁾

$Q = \frac{[(P-K)(0.2+S)]^2}{[(P-K)+(0.8+S)]} \times \frac{1ft}{12"} \times 1.00$

Where:

Q = Runoff depth (ft)⁽⁶⁾
P = Precipitation (in) = 1.00
K = Seasonal Precipitation Factor⁽⁷⁾

****1.00 Per 2016 MS4 ***
0.92 inches in the Santa
Rosa area, based on local
historical data.**

$V = (Q)(A_r)$

Where:

V = Volume of Storm Water to be Retained (ft³)
A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A_r = 8,476 ft²
K⁽⁷⁾ = 1.17

Drop down Lists

Select post development hydrologic soil type within tributary area⁽⁸⁾ = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
Select post development ground cover description⁽⁵⁾ = **Impervious - Paved Parking, Rooftop, Driveways**

OR: Composite post development CN⁽⁹⁾ = 95

NOTE:

Entering a calculated composite CN will override selections made from the pull down menu above. Calculation worksheet should be used for all composite calculations and included with submittal.

Volume of storm water - Post Development

S_{POST} = 0.52410 in

S_{POST} = 1000 - 10
95

Where:

S_{POST} = Post development potential maximum retention after runoff (in).

Q_{POST} = 0.05971 ft

Q_{POST} = $\frac{[(1.00 * 1.17) - (0.2 * 0.52)]^2}{[(1.00 * 1.17) + (0.8 * 0.52)]} \times \frac{1ft}{12in}$

Q_{POST} = Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

V_{GOAL} = 506.10 ft³

V_{GOAL} = (0.05971)(8,476)

V_{GOAL} = Post Development Volume of Storm Water to be Retained (ft³)

INSTRUCTIONS:

This Design Goal of 100% Capture is the ideal condition and if achieved satisfies all requirements so that no additional treatment is required and pages 4 and 5 of this calculator do not need to be completed.

NOTE:

If the Design Goal of 100% Capture is not achieved, 100% Treatment AND Volume Capture must be achieved and both pages 4 and 5 of this calculator need to be completed.

Requirement 1: 100% Treatment

Treatment of 100% of the flow generated by 85th percentile 24 hour mean annual rain event (0.2 in/hr).

Formula:

$Q_{TREATMENT} = (0.2 \text{ in/hr})(A_r)(C_{POST})(K) \text{ cfs}$

Where:

$Q_{TREATMENT}$ = Design flow rate required to be treated (cfs)

C_{POST} = Rational method runoff coefficient for the developed condition^[10]

A_r = Reduced Tributary Area including credit for Pollution Prevention Measures (in Acres)

K = Seasonal Precipitation Factor^[7]

Input:

$A_r = 8.476 \text{ ft}^2 = 0.19458 \text{ Acres}$
 $C_{POST}^{[10]} = 0.62$
 $K^{[7]} = 1.2$

Solution:

$Q_{TREATMENT} = 0.02823 \text{ cfs}$

$Q_{TREATMENT} = (0.2)(0.1946)(0.62)(1.17)$

NOTE:

The Flow Rate calculated here should only be used to size the appropriate BMP. All associated overflow inlets and systems should be sized for the Flood Control event.

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, this page of the calculator, AND Requirement 2- Volume Capture, page 5 of the calculator, **must** be achieved.

C value note:

The C value used for this calculation is smaller than the value used for hydraulic Flood Control design.

The table of values can be found here

This smaller value should not be used to size the overflow bypass.

Requirement 2: Delta Volume Capture

No increase in volume of runoff leaving the site due to development for the 85th percentile 24 hour storm event.

Formulas:

$$S = \frac{1000}{CN} - 10$$

Where:
S= Potential maximum retention after runoff (in)^[6]
CN= Curve Number^[5]

$$Q = \frac{[(P-K) - (0.2 \cdot S)]^2}{[(P-K) + (0.8 \cdot S)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Where:

Q= Runoff depth (ft)^[6]

P= Precipitation (in) = 0.92 0.92 Inches in the Santa Rosa area, based on local historical data.

K= Seasonal Precipitation Factor^[7]

$$V = (Q/A_r)$$

Where:

V= Volume of Storm Water to be Retained (ft³)

A_r= Reduced Tributary Area including credit for Pollution Prevention Measures (ft²)

Input: (Pick data from drop down lists or enter calculated values)

A _r =	8,476 ft ²
K ^[7] =	1.2

Drop down Lists

Select hydrologic soil type within tributary area^[8] = **D: 0 - 0.05 in/hr infiltration (transmission) rate**
 Select predevelopment ground cover description^[9] = **Woods (50%), grass (50%) combination (orchard or tree farm) - Fair**
 Select post development ground cover description^[9] = **Impervious - Paved Parking, Rooftop, Driveways**

CN _{PRE} =	82
CN _{POST} =	98
Composite Predevelopment CN ^[9] =	80
Composite Post development CN ^[9] =	95

OR

Composite Predevelopment CN^[9] = 80
 Composite Post development CN^[9] = 95

Solution:

Pre Development Storm Water Runoff Volume

$$S_{PRE} = \frac{1000}{2.50} - 10$$

$$S_{PRE} = \frac{1000}{80} - 10$$

Where:
S_{PRE}= Pre development potential maximum retention after runoff (in).

$$Q_{PRE} = 0.00900 \text{ ft}$$

$$Q_{PRE} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 2.50)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 2.50)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{PRE}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{PRE} = 76.28 \text{ ft}^3$$

$$V_{PRE} = (0.00900)(8,476)$$

V_{PRE}= Pre Development Volume of Storm Water Generated (ft³)

Post Development Storm Water Runoff Volume

$$S_{POST} = \frac{1000}{0.52410} - 10$$

$$S_{POST} = \frac{1000}{95} - 10$$

Where:
S_{POST}= Post development potential maximum retention after runoff (in).

$$Q_{POST} = 0.05280 \text{ ft}$$

$$Q_{POST} = \frac{[(0.92 \cdot 1.17) - (0.2 \cdot 0.52)]^2}{[(0.92 \cdot 1.17) + (0.8 \cdot 0.52)]} \times \frac{1 \text{ ft}}{12 \text{ in}}$$

Q_{POST}= Q in feet of depth as defined by the "Urban Hydrology For Small Watersheds" TR-55 Manual.

$$V_{POST} = 447.53 \text{ ft}^3$$

$$V_{POST} = (0.05280)(8,476)$$

V_{POST}= Post Development Volume of Storm Water Generated (ft³)

Solution: Volume Capture Requirement

Increase in volume of storm water that must be retained onsite (may be infiltrated or reused).

$$\text{Delta Volume Capture} = (V_{POST} - V_{PRE})$$

$$\text{Delta Volume Capture} = (447.53) - (76.28)$$

Where:

Delta Volume Capture= The increase in volume of storm water generated by the 85th percentile 24 hour storm event due to development that must be retained onsite (may be infiltrated or reused).

$$V_{DELTA} = 371.25 \text{ ft}^3$$

INSTRUCTIONS:

If the Design Goal of 100% Capture on page 3 of this calculator is not achieved; then Requirement 1-100% Treatment, page 4 of the calculator, AND Requirement 2- Volume Capture, this page of the calculator, must be achieved.

NOTE:

If the amount of volume generated after development is less than or equal to that generated before development, Requirement 2-Volume Capture is not required.

$$(C_{POST} \leq C_{PRE} \text{ or } CN_{POST} \leq CN_{PRE})$$

LID BMP Sizing Tool: 100% Volume Capture Goal: V_{GOAL}

Formulas:

$$V_{LID\ GOAL} = ((V_{GOAL}) / (P)) = \frac{1265.25}{0.4} \text{ ft}^3$$

$$A_{LID\ GOAL} = (W)(L) = \frac{633.75}{6.5} \text{ ft}^2$$

Where:

$V_{LID\ GOAL}$ = Required volume of soil in LID BMP.

$A_{LID\ GOAL}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{GOAL} = 506 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.4

D = 2.0 ft

W = 6.5 ft

L = 97.5 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Goal Achieved} = \frac{(D)(A_{LID\ GOAL})}{V_{LID\ GOAL}} \times 100$$

Solution:

$$\text{Percent of Goal Achieved} = 100.18\%$$

$$= [(2.0 \times 634) / 1,265] \times 100$$

INSTRUCTIONS:

The 100% volume capture sizing tool helps the designer appropriately size a LID BMP to achieve the design goal of 100% volume capture of the post-development condition. Enter the percent porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Goal" equals 100%.

LID BMP Sizing Tool Delta Volume Capture Requirement: V_{DELTA}

Formulas:

$$V_{LID\ DELTA} = ((V_{DELTA}) / (P)) = \frac{371.25}{0.4} \text{ ft}^3$$

$$A_{LID\ DELTA} = (W)(L) = \frac{0.00}{6.5} \text{ ft}^2$$

Where:

$V_{LID\ DELTA}$ = Required volume of soil in LID BMP

$A_{LID\ DELTA}$ = Footprint of LID BMP area for a given depth (below perforated pipe if present).

$$V_{DELTA} = 371.25 \text{ ft}^3$$

Where:

P = Porosity (enter as a decimal)

D = Depth below perforated pipe if present (in decimal feet)

W = Width (in decimal feet)

L = Length (in decimal feet)

Input:

P = 0.0

D = 0.0 ft

W = 0.0 ft

L = 0.0 ft

as a decimal
Below perforated pipe if present

$$\text{Percent of Requirement Achieved} = \frac{(D)(A_{LID\ DELTA})}{V_{LID\ DELTA}} \times 100$$

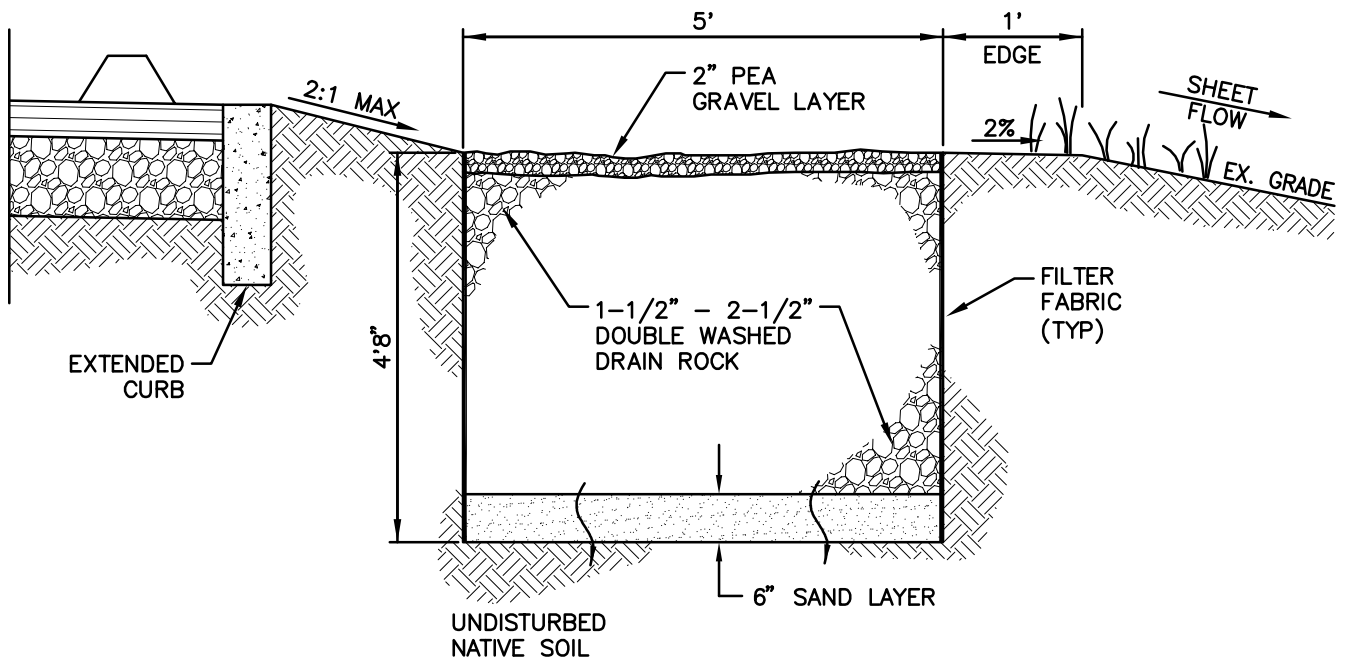
Solution:

$$\text{Percent of Requirement Achieved} = \#DIV/0!\%$$

$$= \#DIV/0!$$

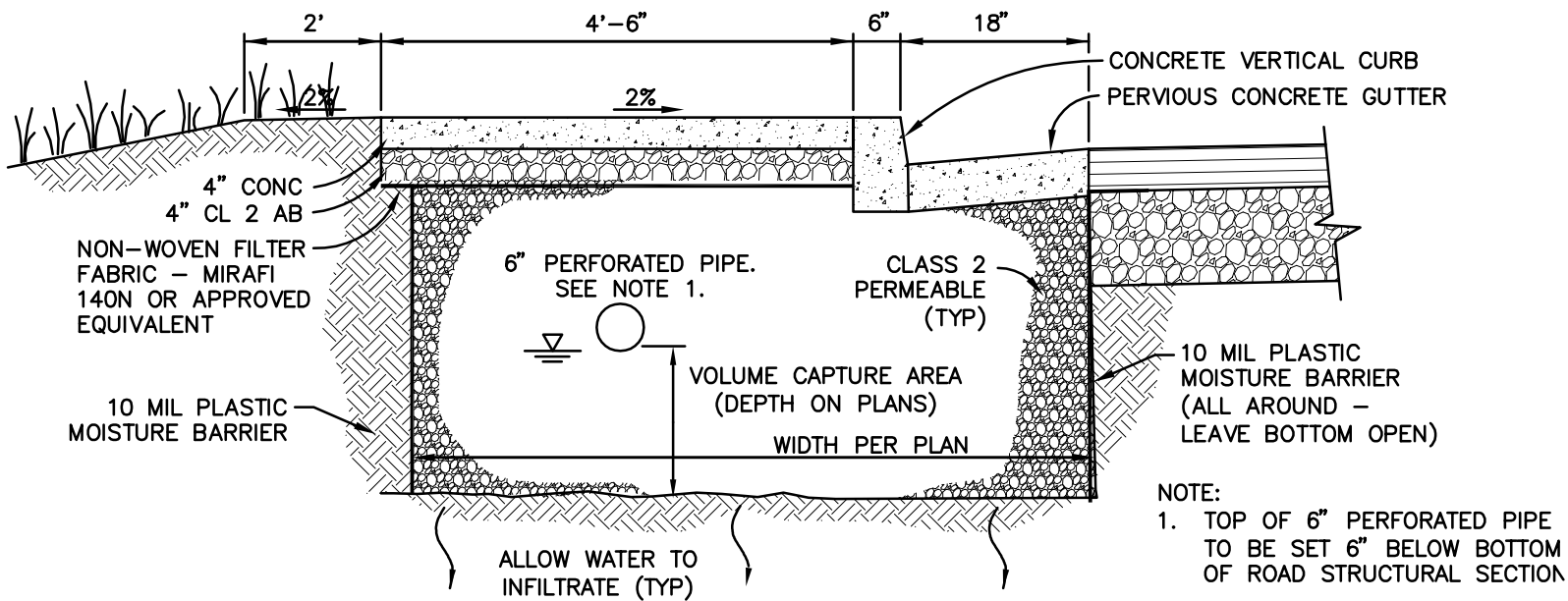
INSTRUCTIONS:

The Delta Volume Capture sizing tool helps the designer appropriately size a LID BMP to achieve the design requirement of the delta volume capture. Enter the percent of porosity of the specified soil and depth below perforated pipe (if present). The width and length entries will need to be interactively adjusted until "Percent of Requirement achieved" reaches 100%.



INFILTRATION TRENCH

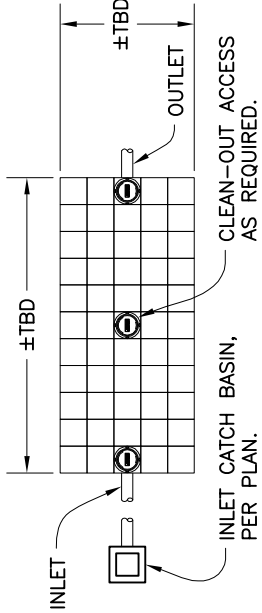
NOT TO SCALE



BIORETENTION BENEATH SIDEWALK

NOT TO SCALE

FOR AREAS 5 AND 10



PLAN VIEW

1 TO 4 MODULE STACKS
SCALE: NONE

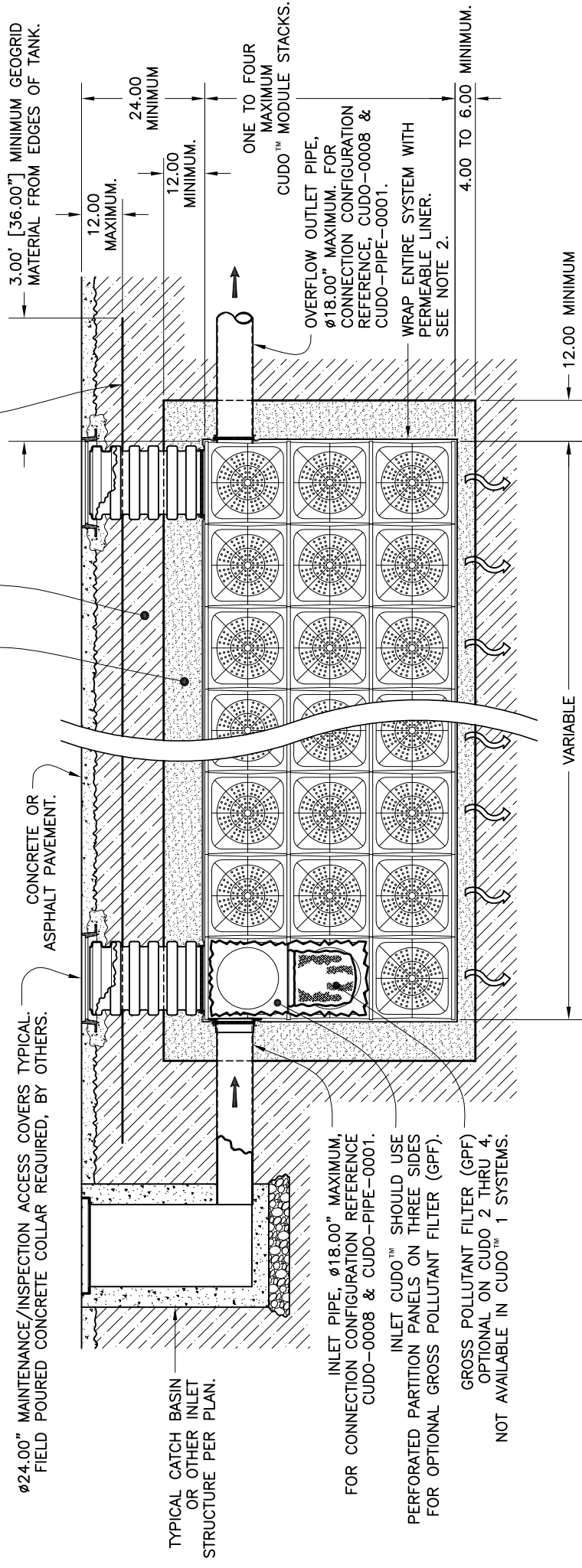
COMPACTED SOIL AS APPROVED AND RECOMMENDED
BY PROJECT GEOTECHNICAL ENGINEER.

BACKFILL, 12" MINIMUM ABOVE CUDO™ SYSTEM, COMPACTED
AS REQUIRED BY PROJECT GEOTECHNICAL ENGINEER.
BACKFILL MATERIAL PER CUSTOMER BEDDING/BACKFILL SPECIFICATION.

OPTIONAL GEOGRID BI-AXLE ON GRAVEL
OR SAND. (TENSAR™ BX1200 OR
EQUIVALENT) 12" MAXIMUM BELOW
FINISHED GRADE AT SHALLOWEST POINT.
SEE NOTE 1.

Ø24.00" MAINTENANCE/INSPECTION ACCESS COVERS TYPICAL.
FIELD POURED CONCRETE COLLAR REQUIRED, BY OTHERS.

CONCRETE OR
ASPHALT PAVEMENT.



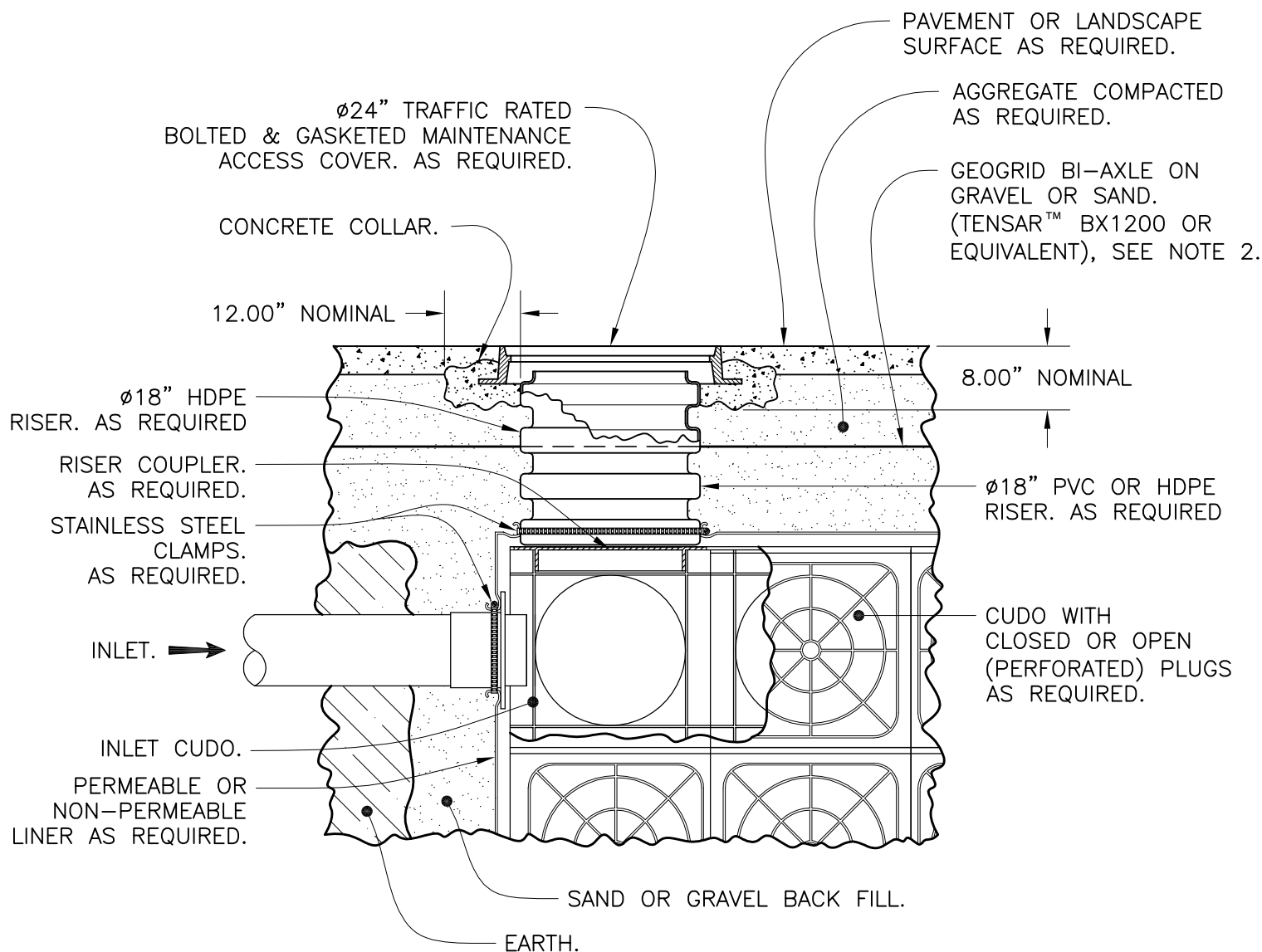
NOTES:

1. INSTALL GEOGRID LAYER, (TENSAR™ BX1200 OR EQUIVALENT) IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS.
2. SYSTEM ENCASED ENTIRELY WITH NONWOVEN POLYPROPYLENE GEOTEXTILE FILTER FABRIC AS REQUIRED.
3. FIELD POURED CONCRETE COLLAR REQUIRED AROUND ALL ACCESS COVERS & HATCHES, BY OTHERS.
4. ALL EXTERNAL PIPING & ANGLES BY OTHERS. REFER TO PLANS.

SECTION / CUTAWAY VIEW

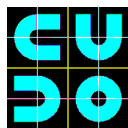
SCALE: NONE

 KriStar Enterprises, Inc. 360 Sutton Place, Santa Rosa, CA 95407 Ph: 800.579.8819, Fax: 707.524.8186, www.kristar.com	CUDO™ Stormwater Infiltration System Typical Installation Detail Impermeable Pavement Area	0005 JPR 3/20/12	CUDO-IN-0001	B	SHEET 1 OF 1
	KRISTAR'S STANDARD DETAILS DEMONSTRATE RECOMMENDATIONS FOR INSTALLATION AND/OR STANDARD DIMENSIONS OF KRISTAR PRODUCTS. KRISTAR'S DETAILS DO NOT SUPERSEDE ANY FEDERAL, STATE OR LOCAL AGENCY REGULATIONS. IT IS THE RESPONSIBILITY OF THE ENGINEER TO ENSURE COMPLIANCE WITH ALL REGULATORY AND DESIGN RELATED REQUIREMENTS. KRISTAR HAS NOT AUTHORIZED ANY UNAUTHORIZED ALTERATIONS OR ADAPTATIONS TO THE KRISTAR STANDARD DETAILS.				



NOTES:

1. REFER TO CUDO INSTALLATION DETAILS.
2. INSTALL GEOGRID LAYER, (TENSAR™ BX1200 OR EQUIVALENT) IN ACCORDANCE WITH MANUFACTURER RECOMMENDATIONS IN AREAS SUBJECT TO VEHICLE TRAFFIC,



CUDO™ Water Storage System
TYPICAL MAINTENANCE ACCESS

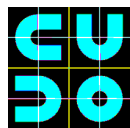
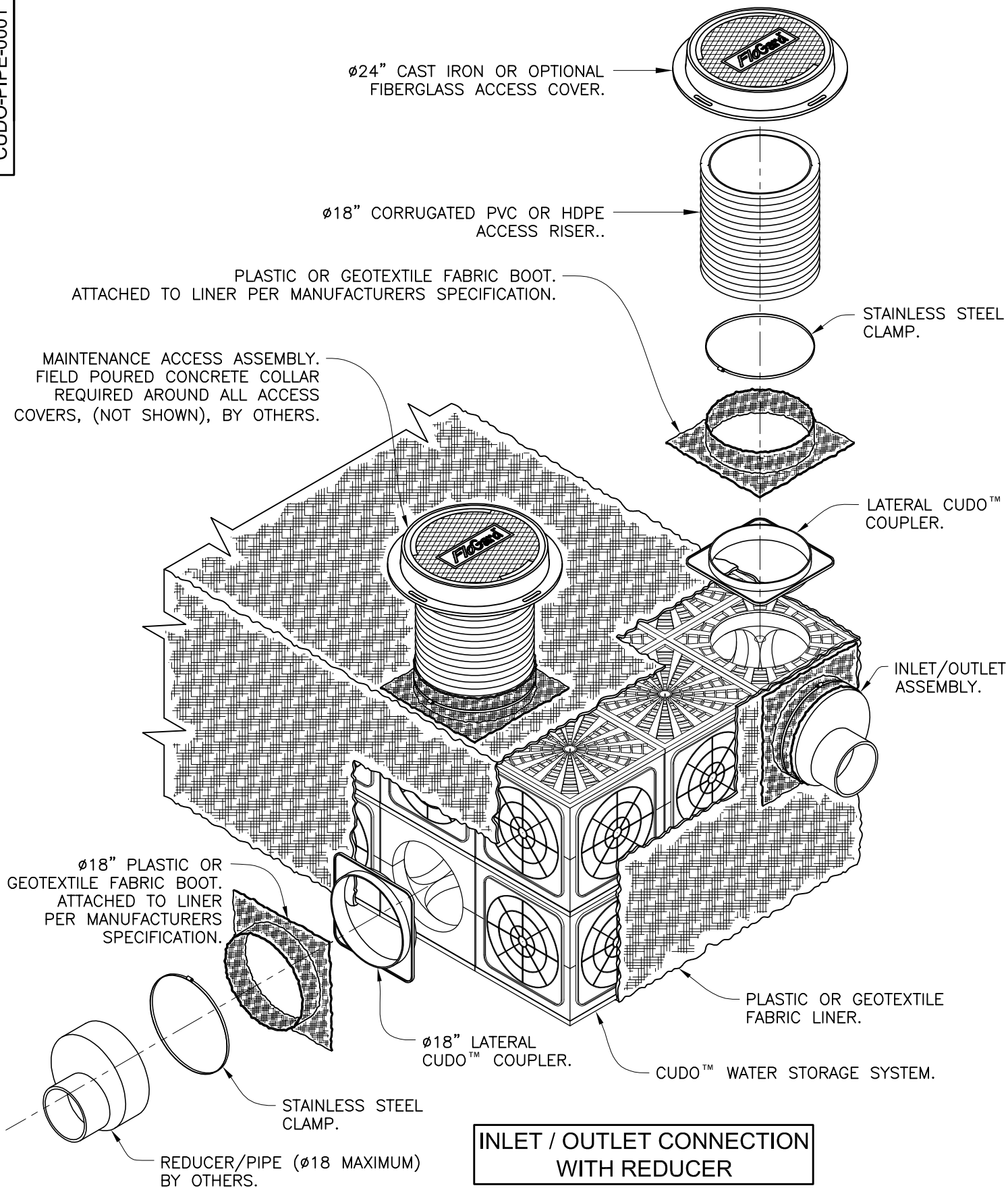


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DO NOT SCALE DRAWING	SCALE: NONE	DRAWN / DATE JPR 5/2/08	CUDO ECO 0006 JPR 3/26/12	DRAWING NO. CUDO-0008	REV B	SHEET 1 OF 1
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CUDO™ Water Storage System Pipe Connection & Maintenance Access Installation Diagram



KriStar Enterprises, Inc.

360 Sutton Place, Santa Rosa, CA 95407
 Ph: 800.579.8819, Fax: 707.524.8186, www.kristar.com

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DO NOT SCALE DRAWING	SCALE: NONE	DRAWN / DATE JPR 7/22/09	CUDO ECO 0005 JPR 3/20/12	DRAWING NO. CUDO-PIPE-0001	REV A	SHEET 2 OF 2
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PLANTER STRIP BIORETENTION- CHECKLIST

Planter Strip Bioretention

Inspection and Maintenance Checklist

(aka: Street Rain Garden, Roadside Bioretention, Bioretention Cell)

Date of Inspection: _____
Inspector(s): _____
BMP ID #: _____
Property Owner: _____

Location Description: _____

Type of Inspection: ☐ Pre-rainy Season (PRS) ☐ Rainy Season (RS) ☐ After-rainy Season (ARS)

This Inspection and Maintenance Checklist is to be used in conjunction with its corresponding LID Factsheet and Maintenance Plan. Please review these documents before performing the field inspection.

Inspection Category	When to Inspect	Maintenance Issue	Is the Issue Present?	Require Maintenance	Comments (Describe maintenance completed and if needed maintenance was not conducted, note when it will be done)
Drainage	RS	Is there standing or pooling of water in the Bioretention area after 3 days of dry weather?		• Check perforated pipe outlet for obstruction or damage. * • Flush perforated pipe to remove obstructions/sediment. * • Remove and replace the first few inches of topsoil. • Remove soil and inspect perforated pipe. Repair or replace perforated pipe, replace with new soil and regrade.	
		Is water not draining into catch basin from the overflow pipe during a high intensity storm? *			
	PRS RS ARS	Is there sediment visible in the gutter?		• In dry weather, use a mechanical sweeper or a Vactor truck to clean gutter pan.	
	RS	Is there water flowing in the pervious concrete gutter section during a low intensity storm? *		• In wet weather, use a Vactor truck to clean gutter pan.	

* If perforated pipe is present.

PLANTER STRIP BIORETENTION- CHECKLIST

Inspection Category	When to Inspect	Maintenance Issue	Is the Issue Present?	Require Maintenance	Comments (Describe maintenance completed and if needed maintenance was not conducted, note when it will be done)
Erosion	RS ARS	Is there under cutting or washouts along the sidewalks and/or curbs abutting the planter strip?		• Fill in eroded areas and regrade.	
	RS ARS	Is there channelization (gully) forming along the length of the planter area?		• Fill in eroded areas and regrade.	
	RS ARS	Is there accumulation of sediment (sand, dirt, mud) in the planter?		• Remove sediment and check the grading. Add replacement soil and/or mulch.	
	PRS RS ARS	Is the mulch unevenly distributed in the planter area?		• Redistribute and add additional mulch if needed. • Regrade planter area.	
	PRS RS ARS	Are there voids or deep holes present? Is there sediment present in the catch basin and in the overflow pipe?		• Check the perforated pipe for damage.*	
	PRS RS ARS	Is there evidence of animal activity such as holes or dirt mounds from digging or borrowing?		• Repair and fill in damage areas. • Rodent control activities must be in accordance with applicable laws and do not affect any protected species.	

* If perforated pipe is present.

PLANTER STRIP BIORETENTION- CHECKLIST

Inspection Category	When to Inspect	Maintenance Issue	Is the Issue Present?	Require Maintenance	Comments (Describe maintenance completed and if needed maintenance was not conducted, note when it will be done)
Vegetation	PRS RS ARS	Is the vegetation clogging the inlet flow areas?		Trim and/or remove the excess vegetation.	
	PRS RS ARS	Is the mulch distributed evenly throughout the planter area?		- Redistribute and add additional mulch if needed. - Regrade planter area.	
	PRS RS ARS	Are there dead or dry plants/weeds? Is the vegetation over grown?		- Remove dead and/or dry vegetation. Replace as needed. - Remove or trim any vegetation that is causing a visual barrier, trip, and or obstruction hazard.	

PLANTER STRIP BIORETENTION- CHECKLIST

Inspection Category	When to Inspect	Maintenance Issue	Is the Issue Present?	Require Maintenance	Comments (Describe maintenance completed and if needed maintenance was not conducted, note when it will be done)
BMP General	PRS RS ARS	Is there debris/trash in the planter area?		Remove all trash and debris.	
	PRS RS ARS	Is graffiti present?		Remove all graffiti from the area.	
	PRS RS ARS	Are there missing or disturbed aesthetics features?		- Replace and/or reposition aesthetics features to original placement. - Placement should not disrupt flow characteristics/design.	
	PRS RS ARS	Is the vegetation irrigation functional?		- Repaired broken missing spray/drip emitters. - Reposition and/or adjust to eliminate over spray and/or over watering.	
	PRS RS ARS	Are the aesthetic features firmly secured in placed?		Repair and/or replace loose or damage features.	
	PRS RS ARS	Check for damage sidewalk, curb, gutter, and catch basin including uplift and settling.		Remove and replace damaged areas.	



CUDO™ Stormwater Cube

(Underground Retention / Detention / Infiltration /
Water Reuse Systems)

Operations and Maintenance Manual

CUDO™ Stormwater Cube – Modular Stormwater Systems

Description / Basic Function

CUDO™ is a modular stormwater system comprised of a grouping of modular polypropylene or concrete cubes that when constructed form an underground storage area for stormwater. This system can be used for infiltration, retention, detention or water reuse. CUDO™ can help achieve runoff detainment and storage to help attenuate the peak flow to pre-construction levels and can help conform to current Low Impact Development requirements.

Infiltration

The purpose of a CUDO™ infiltration system is to capture stormwater runoff, store the runoff, and then allow it to percolate into the ground via the open space area of the cubes and perforations in the side wall. The system is backfilled with a Class I material defined by ASTM D2321 as a cleaned open graded rock or a Class II permeable sand. The rock or sand provide additional storage capacity but also allow for a percolation interface with the native material. The ground water is “recharged” with this type of system.

Detention

The purpose of a CUDO™ detention system is to capture stormwater runoff, store the runoff, and then allow it to be released at a controlled rate through an appropriately sized orifice control. A detention system helps attenuate the peak flow from the site assuring that pre-development runoff flows are not exceeded as a result of the development. A CUDO™ detention requires the cubes to be encapsulated with an impermeable liner for the polypropylene system or the seams of the concrete system to be sealed with a water proof mastic.

Retention

A CUDO™ retention system is a hybrid system. It is a combination of a detention system and an infiltration system. A retention system is utilized to attenuate peak flow as well as promote groundwater re-charge. A retention system is outfitted with an overflow pipe at the top of the system which allows the system to fill for infiltration but also outlet if the ground is saturated.

Water Reuse

The purpose of a water-reuse CUDO™ system is to capture and store water for future use. The system is constructed in a similar fashion to a detention system but instead of a controlled outlet the system is constructed with an emergency overflow. A water reuse system is a LID device that helps attenuate peak flows as well as conserve water. Water may be reused through an active pump system or passive irrigation.

Inspection/Cleanout Ports

Inspection and cleanout ports are 18-inch diameter vertical risers connected to the uppermost polypropylene CUDO™ cubes or up to 30-inch manhole access connected to the concrete CUDO™. They are used for entrance into the system, or for access to place vacuum truck hoses or water-jetting devices or CCTV equipment. Ports are strategically located near inlet and outlet pipes and in other areas or probable deposition in the system. It is recommended to keep surface level access lids sealed and bolted at all times when the system is in service.

Inlet Bay

Some systems are configured so that pretreatment of the stormwater occurs within the CUDO™ system. In this case the CUDO™ system will house an inlet bay. The inlet bay is separated from the rest of the CUDO™ system by sidewall plugs and is intended to separate gross pollutants, trash and debris and floatables from the CUDO™ system and pre-treatment device. The bay contains its own sump area and unique access ports.

Maintenance Overview for CUDO

State and Local regulations require that stormwater storage systems be maintained and serviced on a recurring basis. The purpose of maintaining a clean and obstruction free CUDO™ system is to ensure the system performs the intended function of the primary design. Trash and debris, floatables, gross pollutants and sediment can build up in the CUDO™ leading to clogging of the native soil interface or blockage of the inlet or outlet pipes. This can cause the system to function improperly by limiting storage volume, limiting the design percolation rates or impeding flow in and out of the system. Downstream and upstream, areas could run the risk of flooding and deleterious environmental impact.

Recommended Frequency of Service

It is recommended that the CUDO™ stormwater systems be serviced on a regularly occurring basis. Ultimately the frequency depends on the amount of runoff, pollutant loading, and interference from trash, debris and gross pollutants as well as proper maintenance of upstream pretreatment devices. However, it is recommended that each installation be inspected at least two times per year to assess service needs.

Recommended Timing of Service

Guidelines for the timing of service are as follows:

1. For areas with a definite rainy season the system should be serviced prior to and following the rainy season.
2. For areas subject to year-round rainfall service should occur on a regularly occurring basis. (A minimum of two times per year.)
3. For areas with winter snow and summer rain the system should be serviced prior to and after the snow season.
4. For installed devices that are subject to dry weather flows only (i.e. wash racks, parking garages, etc...) the unit should be serviced on a regularly occurring basis. (A minimum of two times per year.)

Inspection

An inspection should be performed when the system is new. This allows the owner to establish a baseline condition for comparison to future inspections. Sediment build up can typically be monitored without entering the system. (No confined space entry.) Initial and subsequent inspection data should be recorded and filed for reference. Some regulatory agencies require that the results of the inspections be documented and reported. Inspection reports should comply with regulatory requirements and be submitted as required.

Inspection Procedures

1. Locate the inspection, cleanout and access ports. Inspection and cleanout ports are typically 18-inch diameter. Access ports are typically 24-inch or 30-inch diameter. Pictures should be taken to document the location or a site map should be generated to detail the as-built locations of the ports.
2. Unbolt and remove the access port lids.
3. Insert a measuring device into the opening making note of a point of reference to determine the quantity of sediment and other accumulated material. If access is required to measure, ensure only certified confined space entry personnel having appropriate equipment are allowed to enter the system.
4. In addition, for accessible concrete CUDO™ systems personnel should utilize appropriate confined space entry procedures to enter the system and photograph its condition.
5. Inspect inlet and outlet locations for obstructions. Obstructions should be removed at this time.
6. Inspect the structural components of the system.
7. Fill in the CUDO™ Inspection/Maintenance Data Sheet and send a copy to the regulatory agency if necessary.

Disinfection of Water Reuse System

Periodic disinfection of water held for reuse may be required to abate bacteria and algae growth. This may be done using calcium hypochlorite tablets or by the addition of an ozone generator in a small recirculation system.

Maintenance

Cleanout of the CUDO™ system should be considered if there is sediment buildup of two or more inches at over 50% of the inspection ports. Cleaning shall be performed if sediment buildup is two inches or more over 75% of the system floor. In the event of a spill of a foreign substance, cleanout of the system should be considered.

Maintenance Procedures

1. Locate the inspection, cleanout and access ports. Inspection and cleanout ports are typically 18-inch diameter. Access ports are typically 24-inch or 30-inch diameter. Pictures should be taken to document the location or a site map should be generated to detail the as-built locations of the ports.
2. Unbolt and remove the access port lids.
3. Measure the sediment buildup at each port. If access is required to measure ensure only certified confined space entry personnel having appropriate equipment are allowed to enter the system.
4. A thorough cleaning of the system (inlets, outlets, ports, and inlet bays) shall be performed by either a vacuum truck or by manual methods.
5. Inspect inlet and outlet locations for obstructions. Obstructions should be removed at this time.
6. Inspect the structural components of the system.
7. Fill in the CUDO™ Inspection/Maintenance Data Sheet and send a copy to the regulatory agency if necessary.

Inspection / Maintenance Requirements

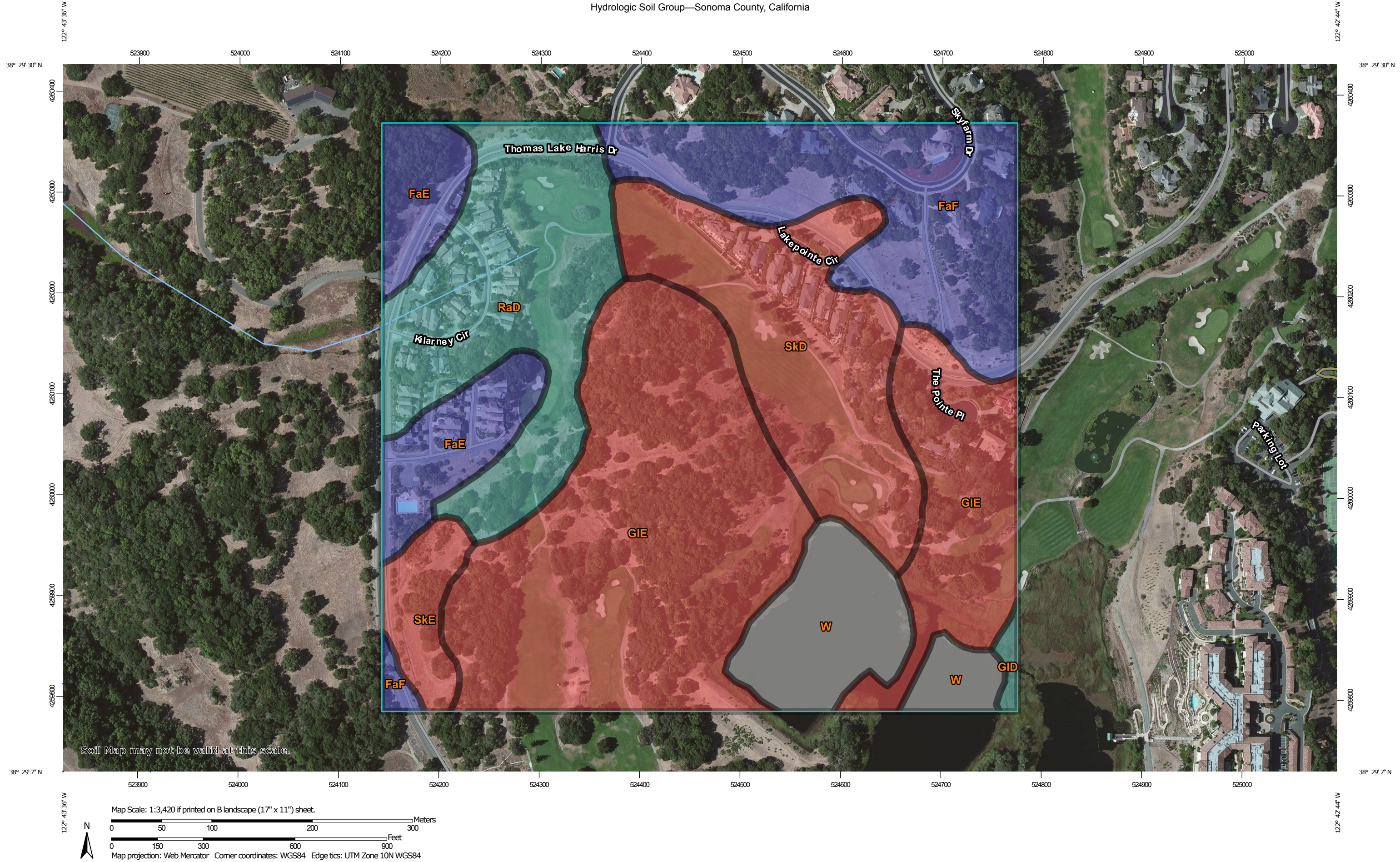
Listed below are some recommendations for equipment and training for personnel to inspect and maintain a CUDO™ system.

Personnel –	OSHA Confined Space Entry Training is a prerequisite for entrance into a system. In the state of California personnel should be CalOSHA certified.
Equipment –	Record Taking (pen, paper, voice recorder) Proper Clothing (appropriate footwear, gloves, hardhat, safety glasses, etc.) Flashlight Tape Measure Measuring Stick Pry Bar Traffic Control (Flagging, barricades, signage, cones, etc.) First aid materials Debris and Contaminant collectors Debris and Contaminant containers Vacuum Truck

Disposal of Gross Pollutants, Hydrocarbons, and Sediment

The collected gross pollutants, hydrocarbons, and sediment shall be offloaded from the vacuum truck into DOT approved containers for disposal. Once in the container the maintenance contractor has possession and is responsible for disposal in accordance with local, state and federal agency requirements.

Note: As the generator, the landowner is ultimately responsible for the proper disposal of the collected materials. Because the material likely contains petroleum hydrocarbons, heavy metals, and other harmful pollutants, the materials must be treated as EPA class 2 Hazardous Waste. Proper disposal is required.



MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons

A

A/D

B

B/D

C

C/D

D

Not rated or not available

Soil Rating Lines

A

A/D

B

B/D

C

C/D

D

Not rated or not available

Soil Rating Points

A

A/D

B

B/D

Water Features

Streams and Canals

Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

Background

Aerial Photography

C

C/D

D

Not rated or not available

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Sonoma County, California
Survey Area Data: Version 10, Sep 27, 2016

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Aug 14, 2011—Aug 15, 2011

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Hydrologic Soil Group— Summary by Map Unit — Sonoma County, California (CA097)				
Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
FaE	Felta very gravelly loam, 15 to 30 percent slopes	B	6.7	7.4%
FaF	Felta very gravelly loam, 30 to 50 percent slopes	B	13.9	15.2%
GID	Goulding cobbly clay loam, 5 to 15 percent slopes	C	0.4	0.4%
GIE	Goulding cobbly clay loam, 15 to 30 percent slopes	D	33.0	36.0%
RaD	Raynor clay, 9 to 15 percent slopes	C	14.3	15.6%
SkD	Spreckels loam, 9 to 15 percent slopes	D	13.6	14.9%
SkE	Spreckels loam, 15 to 30 percent slopes	D	2.7	3.0%
W	Water		6.9	7.6%
Totals for Area of Interest			91.6	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

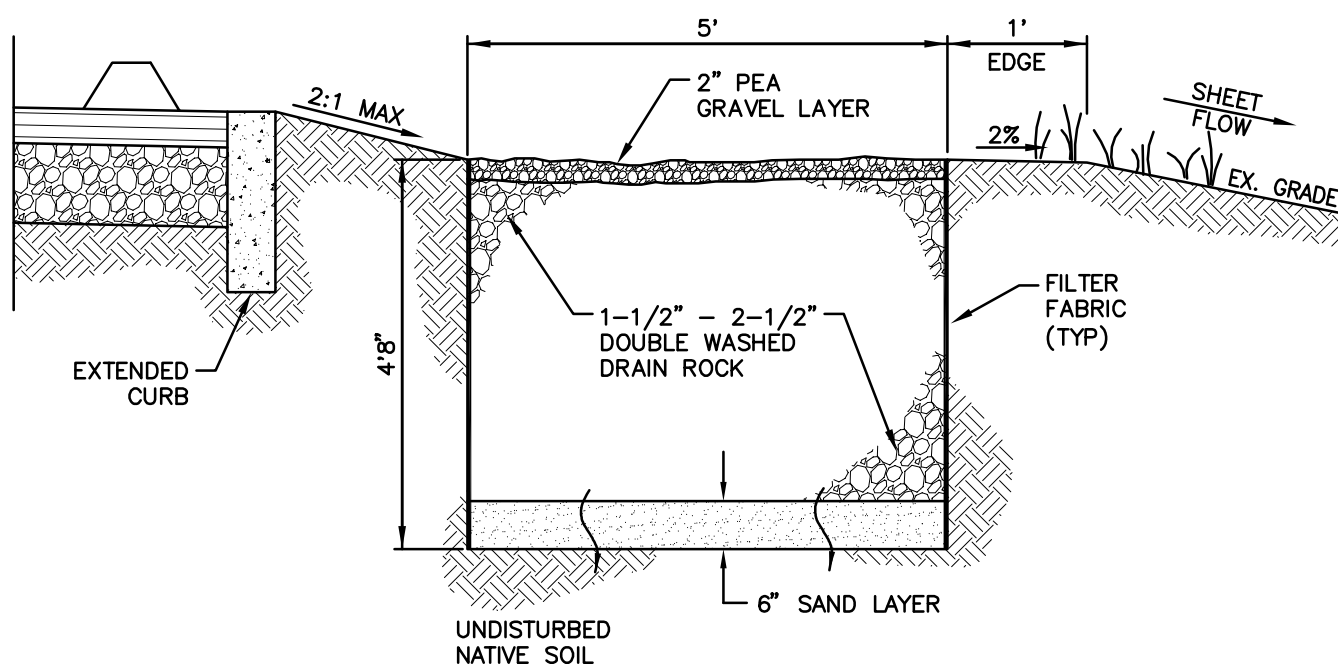
Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

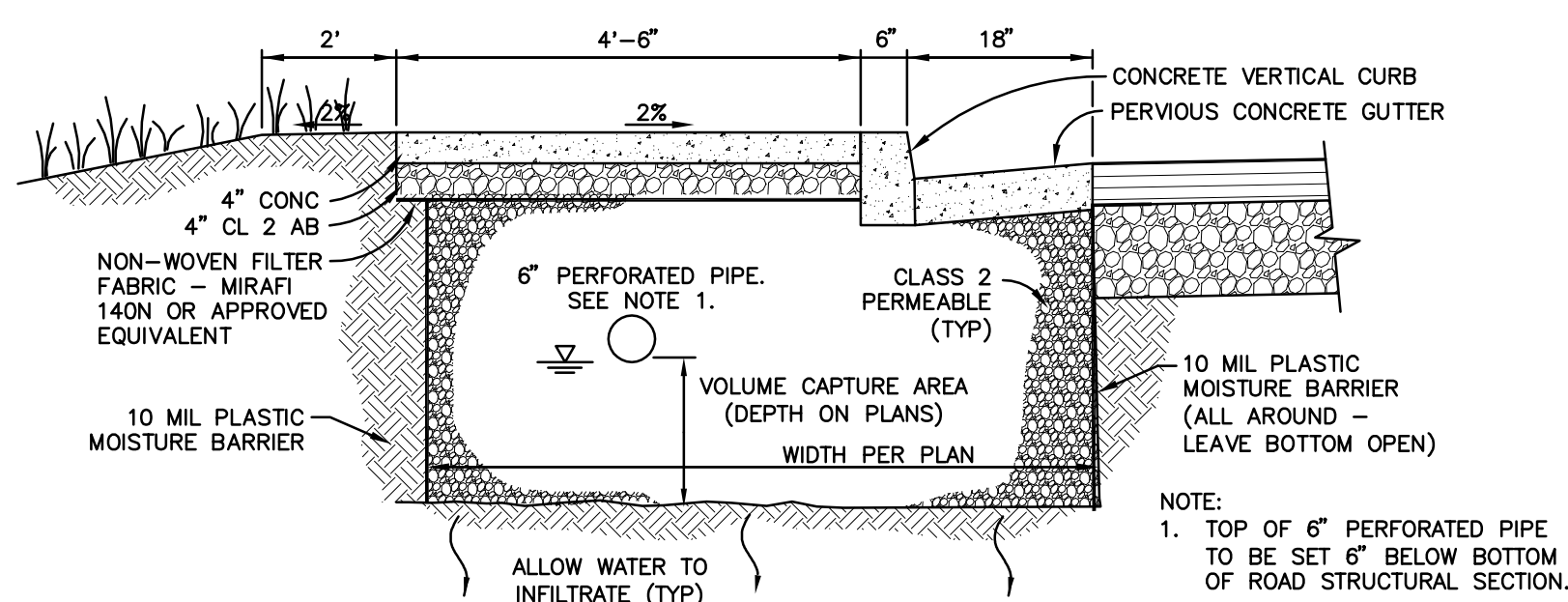
Tie-break Rule: Higher

NOTES:
K-VALUE OF 1.17 WAS USED
ALL SOILS HAVE BEEN ASSUMED TO BE CLASS D



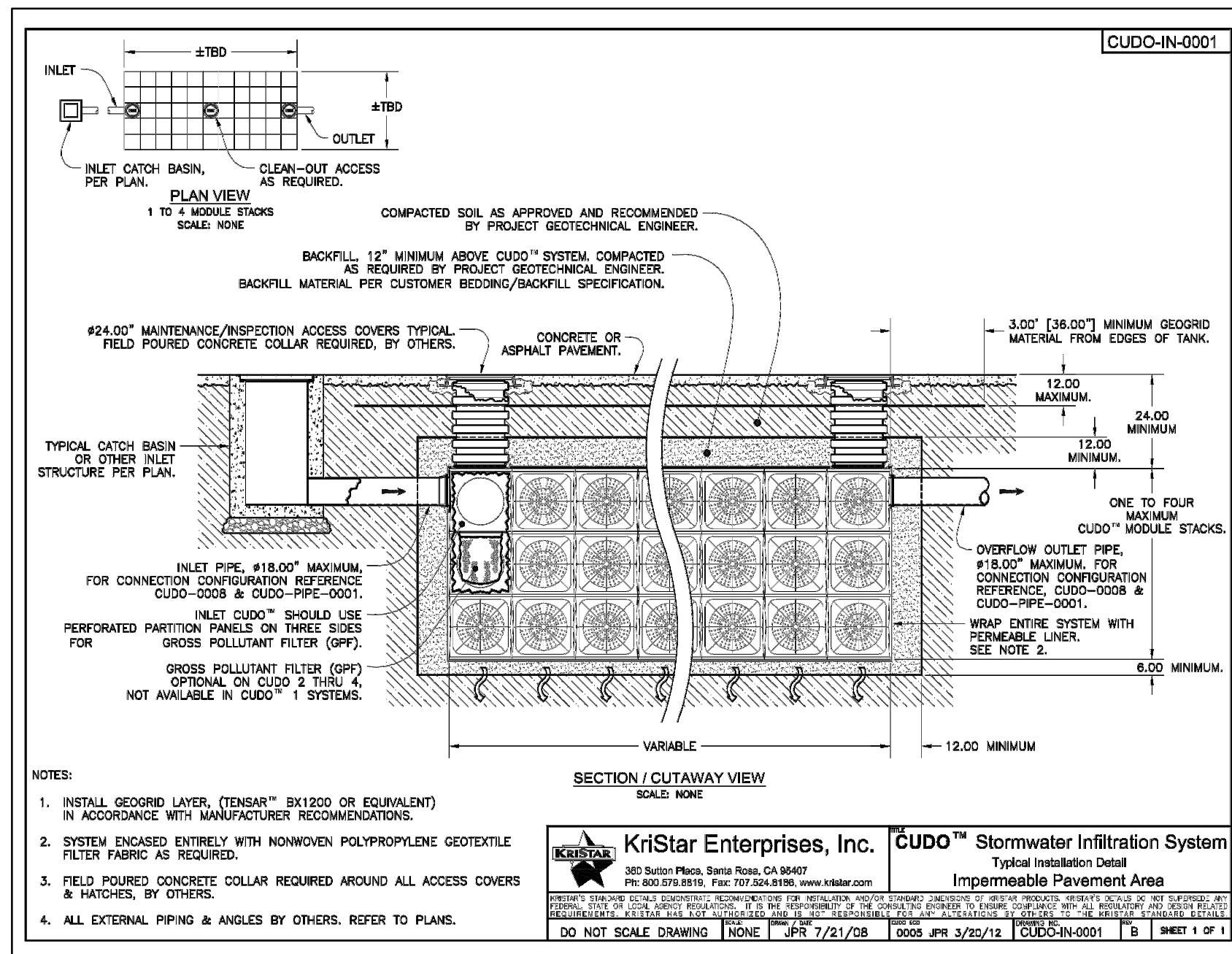
INFILTRATION TRENCH

NOT TO SCALE
FOR AREAS 1, 4, 7 AND 9



BIORETENTION BENEATH SIDEWALK

NOT TO SCALE
FOR AREAS 5 AND 10

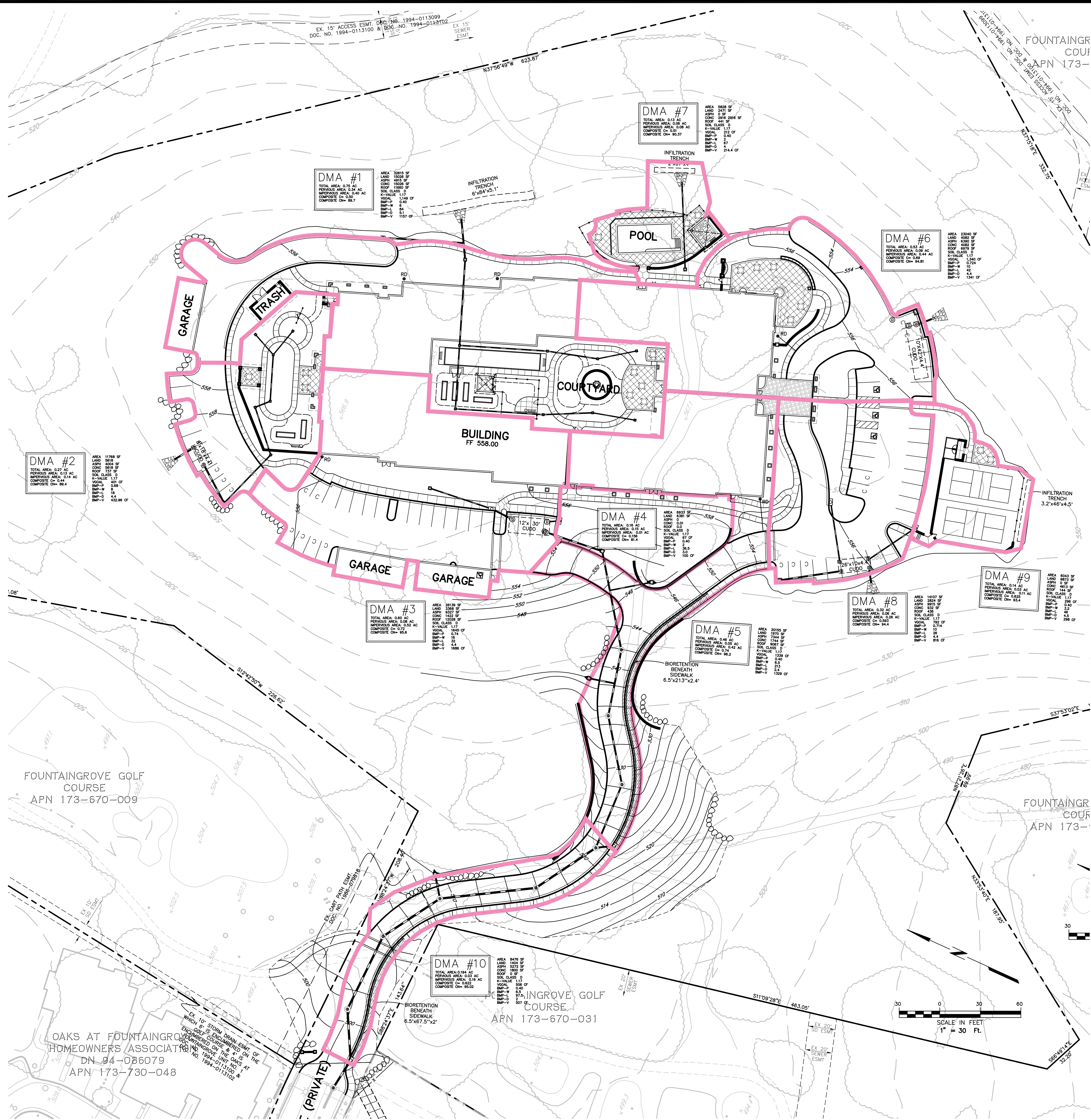


CUDO STORMWATER INFILTRATION SYSTEM

NOT TO SCALE
FOR AREAS 2, 3, 6 AND 8

LEGEND

DMA BOUNDARY



Brelje & Race
CONSULTING CIVIL ENGINEERS
475 Aviation Boulevard, Suite 120
Santa Rosa, CA 95403
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f: 707-576-0469
www.brce.com

**EMERALD ISLE
SKILLED NURSING**

GULLANE DRIVE
SANTA ROSA, CALIFORNIA

PRELIMINARY
FOR STUDY PURPOSES ONLY
DATE 3/17/17

REVISIONS		
NO.	DATE	DESCRIPTION

ON A FULL-SCALE DRAWING, LENGTH OF
BAR BELOW IS 1-INCH. IF BAR MEASURES
LESS THAN 1-INCH, THIS SHEET WAS
PLOTTED AT A REDUCED SCALE, WHICH
MAY REQUIRE ADJUSTMENT OF SCALE(S)
SHOWN ON DRAWING.

PROJECT 4081.02	DATE APRIL 2017
DRAWN BY JCH	CHECKED BY PVB

**PROPOSED
CONDITION
EXHIBIT**

SHEET NO.
1 OF **1**

FOUNTAINGROVE GOLF
COURSE
APN 173-670-005

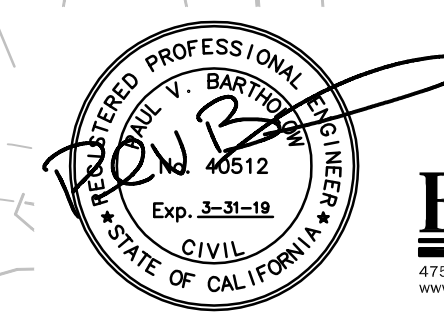
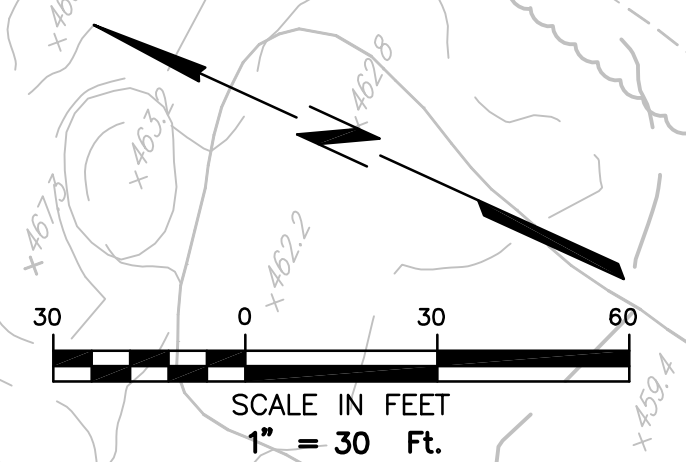
FOUNTAINGROVE GOLF
COURSE
APN 173-670-005

FOUNTAINGROVE GOLF
COURSE
APN 173-670-009

FOUNTAINGROVE GOLF
COURSE
APN 173-670-031

FOUNTAINGROVE GOLF
COURSE
APN 173-670-031

OAKS AT FOUNTAINGROVE
HOMEOWNERS ASSOCIATION
DN 94-086079
APN 173-730-048



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PRELIMINARY DESIGN REVIEW
EMERALD ISLE
PRELIMINARY GRADING PLAN
APRIL 10, 2017

COURSE
APN 173-670-005

APN 173-670-005

FOUNTAINGROVE GOLF
COURSE
APN 173-670-009

FOUNTAINGROVE GOLF
COURSE
APN 173-670-031

OAKS AT FOUNTAINGROVE
HOMEOWNERS ASSOCIATION
DN 94-086079
APN 173-730-048

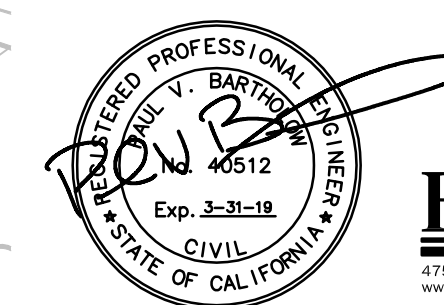
FOUNTAINGROVE LODGE
APN 173-670-034

FOUNTAINGROVE GOLF
COURSE
APN 173-670-031

UTILITY NOTES (ONLY NOTES RELEVANT TO THIS SHEET ARE SHOWN)

1. ALL WET UTILITIES ONSITE ARE PRIVATE.
2. REMOVE EXISTING SSCO, INSTALL PRIVATE SSMH.
3. SDCB'S ON ENTRY DRIVE TO HAVE UPSTREAM GALLEY INSTALLED.

SCALE IN FEET
1" = 30 FT.



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PRELIMINARY DESIGN REVIEW EMERALD ISLE

PRELIMINARY UTILITY PLAN

APRIL 10, 2017

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