



# Traffic Impact Study for the Memorial Hospital MOB Project



## Final Report

Prepared for the City of Santa Rosa

Submitted by  
**W-Trans**

September 8, 2017



**TRAFFIC ENGINEERING  
TRANSPORTATION PLANNING**  
*Balancing Functionality and Livability since 1995*  
w-trans.com



# Table of Contents

---

Executive Summary .....	1
Introduction.....	2
Transportation Setting.....	4
Capacity Analysis .....	8
Alternative Modes .....	18
Access and Circulation.....	19
Parking.....	21
Conclusions and Recommendations.....	22
Study Participants and References.....	24

## Figures

1. Study Area and Lane Configurations.....	3
2. Existing Traffic Volumes.....	10
3. Future Traffic Volumes .....	12
4. Site Plan .....	13
5. Project Traffic Volumes.....	16

## Tables

1. Collision Rates at the Study Intersections.....	5
2. Bicycle Facility Summary .....	7
3. Intersection Level of Service Criteria .....	8
4. Existing Peak Hour Intersection Levels of Service .....	9
5. Future Peak Hour Intersection Levels of Service .....	11
6. Trip Generation Summary.....	14
7. Trip Distribution Assumptions.....	15
8. Existing and Existing plus Project Peak Hour Intersection Levels of Service .....	15
9. Future and Future plus Project Peak Hour Levels of Service .....	17
10. Parking Analysis Summary .....	21

## Appendices

- A. Collision Rate Calculations
- B. Intersection Level of Service Calculations
- C. Pedestrian Crossing Warrant
- D. Traffic Signal Warrant





# Executive Summary

---

The proposed Memorial Hospital medical office building project would result in construction of 92,000 square feet of medical office space, with the potential to include 2,500 square feet for a credit union, and 1,500 square feet for a café. The anticipated trip generation for the project includes up to 3,324 daily trips, with 276 trips during the a.m. peak hour and 336 trips during the p.m. peak hour.

The project would have two access points; one driveway on Montgomery Drive and one on Sotoyome Street. It was assumed the southbound left-turn restriction on Sotoyome Street at Sonoma Avenue would be removed upon the development of the project as discussions with City staff indicate that this restriction may no longer be needed. The southbound approach would remain one lane.

The study area includes six intersections near the project site. Analysis indicates that these intersections are operating acceptably under Existing conditions and would be expected to continue doing so under Existing plus Project volumes. Under anticipated Future volumes the study intersections are expected to continue operating acceptably. With the project-added volumes, the study intersections are expected to operate at the same levels of service as without the project. A traffic signal is warranted at the intersection of Sonoma Avenue/Sotoyome Street.

Transit facilities in the vicinity are adequate. Pedestrian facilities will be adequate with the construction of RRFBs at the proposed mid-block crossing on Montgomery Drive which will replace two existing mid-block crosswalks between Sotoyome Street and Doyle Park Drive. Bicycle facilities will be adequate with the addition of short- and long-term bicycle parking and two showers on-site. The proposed parking exceeds City of Santa Rosa minimum requirements and is expected to be adequate to meet the demand from this project.

Sight distance at the project driveways is generally adequate, though 35 feet of red curb should be installed on either side of the driveway on Sotoyome Street to increase sight lines for exiting vehicles. Any new vegetation should be planted so that it is low-lying and any trees maintained so they are at least seven feet above the ground.

# Introduction

---

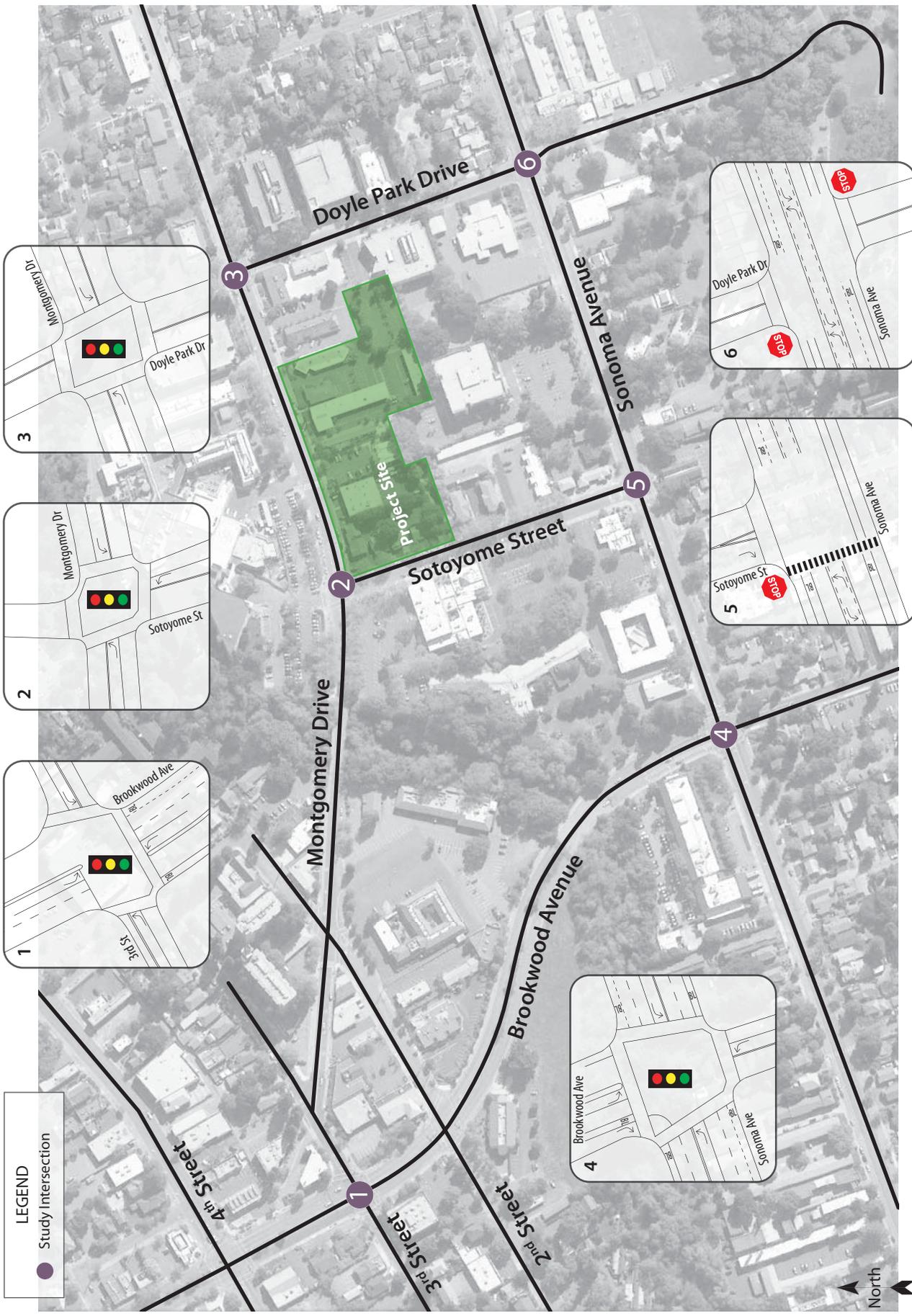
This report presents an analysis of the potential traffic impacts that would be associated with development of a proposed medical office building to be located on the east side of Sotoyome Street between Montgomery Drive and Sonoma Avenue in the City of Santa Rosa. The traffic study was completed in accordance with the criteria established by the City of Santa Rosa, and is consistent with standard traffic engineering techniques.

## Prelude

The purpose of a traffic impact study is to provide City staff and policy makers with data that they can use to make an informed decision regarding the potential traffic impacts of a proposed project, and any associated improvements that would be required in order to mitigate these impacts to a level of insignificance as defined by the City's General Plan or other policies. Vehicular traffic impacts are typically evaluated by determining the number of new trips that the proposed use would be expected to generate, distributing these trips to the surrounding street system based on existing travel patterns or anticipated travel patterns specific to the proposed project, then analyzing the impact the new traffic would be expected to have on critical intersections or roadway segments. Impacts relative to access for pedestrians, bicyclists, and to transit are also addressed.

## Project Profile

The project site is bounded by Sotoyome Street to the west, Montgomery Drive to the north, Doyle Park Drive to the east, and Sonoma Avenue to the south, as shown in Figure 1. Memorial Hospital is proposing to develop a 92,000 square-foot medical office building, potentially including with a 2,500 square-foot credit union and 1,500 square-foot café. Access would be provided on via driveways on Sotoyome Street and Montgomery Drive.



Traffic Impact Study for the Memorial Hospital MOB Project  
**Figure 1 – Study Area and Lane Configurations**

# Transportation Setting

---

## Operational Analysis

### Study Area and Periods

The study area consists of the following intersections:

1. Brookwood Avenue/3<sup>rd</sup> Street
2. Montgomery Drive/Sotoyome Street
3. Montgomery Drive/Doyle Park Drive
4. Sonoma Avenue/Brookwood Avenue
5. Sonoma Avenue/ Sotoyome Street
6. Sonoma Avenue/Doyle Park Drive

Operating conditions during the a.m. and p.m. peak periods were evaluated to capture the highest potential impacts for the proposed project as well as the highest volumes on the local transportation network. The morning peak hour occurs between 7:00 and 9:00 a.m. and reflects conditions during the home to work or school commute, while the p.m. peak hour occurs between 4:00 and 6:00 p.m. and typically reflects the highest level of congestion during the homeward bound commute.

### Study Intersections

**Brookwood Avenue/3<sup>rd</sup> Street** is a signalized intersection with protective left-turn phasing on all four approaches and marked crosswalks across the east, west, and south legs.

**Montgomery Drive/Sotoyome Street** is a signalized intersection with an access point to Memorial Hospital as the north leg. There is permissive left-turn phasing on all four approaches and there are marked crosswalks across all four legs.

**Montgomery Drive/Doyle Park Drive** is a four-legged signalized intersection with permissive left-turn phasing on all four approaches and marked crosswalks across all four legs.

**Sonoma Avenue/Brookwood Avenue** is a signalized intersection with protective left-turn phasing on the northbound and southbound Brookwood Avenue approaches, a right-turn overlap on the southbound approach, and protected-permitted left-turn phasing on the eastbound-westbound Sonoma Avenue approaches. There are marked crosswalks on all four legs of the intersection.

**Sonoma Avenue/Sotoyome Street** is an unsignalized tee-intersection. The stop-controlled southbound approach is currently restricted to right turns only. There are marked crosswalks across the west and north legs of the intersection.

**Sonoma Avenue/Doyle Park Drive** is an off-set, four-legged unsignalized intersection with stop controls on the northbound and southbound Doyle Park Drive approaches. There are marked crosswalks across the north and south legs of the intersection.

The locations of the study intersections and the existing lane configurations and controls are shown in Figure 1.

## Collision History

The collision histories for the study intersections were reviewed to determine any trends or patterns that may indicate a safety issue. Collision rates were calculated based on records available from the California Highway Patrol as published in their Statewide Integrated Traffic Records System (SWITRS) reports. The most current five-year period available is May 1, 2011 through April 30, 2016.

As presented in Table 1, the calculated collision rates for the study intersections were compared to average collision rates for similar facilities statewide, as indicated in *2012 Collision Data on California State Highways*, California Department of Transportation (Caltrans).

**Table 1 – Collision Rates at the Study Intersections**

<b>Study Intersection</b>	<b>Number of Collisions (2010-2015)</b>	<b>Calculated Collision Rate (c/mve)</b>	<b>Statewide Average Collision Rate (c/mve)</b>
1. Brookwood Ave/3 <sup>rd</sup> St	10	<b>0.39</b>	0.27
2. Montgomery Dr/Sotoyome St	4	0.25	0.27
3. Montgomery Dr/Doyle Park Dr	2	0.14	0.27
4. Sonoma Ave/Brookwood Ave	21	<b>0.49</b>	0.27
5. Sonoma Ave/Sotoyome St	3	0.11	0.18
6. Sonoma Ave/Doyle Park Dr	0	0.00	0.15

Note: c/mve = collisions per million vehicles entering

At Brookwood Avenue/3<sup>rd</sup> Street, there were ten reported collisions over the five year period, resulting in a collision rate of 0.39 collisions per million vehicle miles (c/mvm). The majority of the collisions (seven of ten) were broadside crashes with a primary collision factor of “traffic signals and signs.” The City may want to consider adding a red-clearance interval, or increasing the time if one exists, to ensure that conflicting approaches are cleared.

The intersection of Sonoma Avenue/Brookwood Avenue had 21 reported collisions over the five-year study period for a calculated collision rate of 0.49 c/mvm, which is higher than the statewide average of 0.27 c/mvm for similar intersections. Eight collisions were between a vehicle turning left and an oncoming vehicle proceeding straight in the east-west direction, though no more than three such collisions occurred in any 12-month period and only five occurred in the last three years of the study period. Sonoma Avenue at Brookwood Avenue has protected-permitted left-turn phasing in the east-west directions on Sonoma Avenue. The City should monitor crashes at this location, and may want to consider implementing protected-only phasing on Sonoma Avenue should the incidence of such collisions increase. Of the six reported collisions between a northbound or southbound vehicle with an eastbound or westbound vehicle, four were due to “traffic signals and signs.” Similar to indicated above for Brookwood Avenue/3<sup>rd</sup> Street, programming a red clearance interval on all approaches, or increasing the interval if one already exists, may reduce crashes. The collision rate calculations are provided in Appendix A.

## Alternative Modes

### Pedestrian Facilities

Pedestrian facilities include sidewalks, crosswalks, pedestrian signal phases, curb ramps, curb extensions, and various streetscape amenities such as lighting, benches, etc. In general, a network of sidewalks, crosswalks, pedestrian signals, and curb ramps provide access for pedestrians in the vicinity of the proposed project site.

- **Brookwood Avenue** – Intermittent sidewalk coverage is provided on Brookwood Avenue with a significant gap on the west side of the street between 2<sup>nd</sup> Street and Sonoma Avenue. Sidewalks are provided along developed property frontages. There are curb ramps and crosswalks at each of the side street approaches along Brookwood Avenue between 3<sup>rd</sup> Street and Sonoma Avenue.
- **Montgomery Drive** – Continuous sidewalks are provided on both sides of Montgomery Drive between 3<sup>rd</sup> Street and Hahman Drive. There are curb ramps and crosswalks at each of the side street approaches along Montgomery Drive between 3<sup>rd</sup> Street and Talbot Avenue.
- **Sonoma Avenue** – Montgomery Drive between 3<sup>rd</sup> Street and Rosedale Avenue has continuous sidewalks on both sides.
- **Doyle Park Drive** – Continuous sidewalks are provided on both sides of Doyle Park Drive between Montgomery Drive and Sonoma Avenue.
- **Sotoyome Street** – On Sotoyome Street, continuous sidewalks are provided on both sides between Montgomery Drive and Sonoma Avenue.

## Bicycle Facilities

The *Highway Design Manual*, Caltrans, 2012, classifies bikeways into three categories:

- **Class I Multi-Use Path** – a completely separated right-of-way for the exclusive use of bicycles and pedestrians with cross flows of motorized traffic minimized.
- **Class II Bike Lane** – a striped and signed lane for one-way bike travel on a street or highway.
- **Class III Bike Route** – signing only for shared use with motor vehicles within the same travel lane on a street or highway.

Guidance for Class IV Bikeways is provided in *Design Information Bulletin Number 89: Class IV Bikeway Guidance (Separated Bikeways/Cycle Tracks)*, Caltrans, 2015.

- **Class IV Bikeway** – also known as a separated bikeway, a Class IV Bikeway is for the exclusive use of bicycles and includes a separation between the bikeway and the motor vehicle traffic lane. The separation may include, but is not limited to, grade separation, flexible posts, inflexible physical barriers, or on-street parking.

In the project area, Class II bike lanes exist on Brookwood Avenue between 3<sup>rd</sup> Street and Sonoma Avenue and on Sonoma Avenue between Santa Rosa Avenue and Hahman Drive. Bicyclists ride in the roadway and/or on sidewalks along all other streets within the project study area. Table 2 summarizes the existing and planned bicycle facilities in the project vicinity, as contained in the *City of Santa Rosa Bicycle and Pedestrian Transportation Plan*, 2010.

**Table 2 – Bicycle Facility Summary**

<b>Status Facility</b>	<b>Class</b>	<b>Length (miles)</b>	<b>Begin Point</b>	<b>End Point</b>
<b>Existing</b>				
Brookwood Avenue	II	0.27	3 <sup>rd</sup> Street	Sonoma Avenue
Sonoma Avenue	II	1.56	Santa Rosa Avenue	Hahman Drive
Montgomery Drive	II	0.16	Talbot Avenue	Alderbrook Drive
Brookwood Avenue	III	0.48	Sonoma Avenue	Maple Avenue
<b>Planned</b>				
Santa Rosa Creek	I	0.90	Santa Rosa Avenue	Memorial Hospital
Montgomery Drive	II	0.47	3 <sup>rd</sup> Street	Talbot Avenue
Montgomery Drive	II	0.46	Alderbrook Drive	Hahman Drive
Brookwood Avenue	II	0.20	College Avenue	3 <sup>rd</sup> Street
Brookwood Avenue	II	0.48	Sonoma Avenue	Maple Avenue
Doyle Park Drive	III	0.44	Leonard Ave	Doyle Community Park

Notes: \* All or portions of these bikeways are located within the City of Santa Rosa

Source: *City of Santa Rosa Bicycle and Pedestrian Transportation Plan, 2010*

## **Transit Facilities**

### ***Santa Rosa CityBus***

The Santa Rosa CityBus provides fixed route bus service in the City of Santa Rosa. Santa Rosa CityBus Route 4/4A provides service between downtown Santa Rosa and Rincon Valley, Mission Boulevard, and Calistoga Road. The route stops on Sonoma Avenue at Sotoyome Street and Doyle Park Drive and operates Monday through Friday with 30-minute headways between 6:00 a.m. and 8:00 p.m. On Saturdays, service is provided from 6:00 a.m. to 8:00 p.m. with one-hour headways, and Sundays with one-hour headways between 10:00 a.m. and 5:00 p.m.

Two bicycles can be carried on most Santa Rosa CityBus buses. Bike rack space is on a first come, first served basis. Additional bicycles are allowed on Santa Rosa CityBus buses at the discretion of the driver.

Dial-a-ride, also known as paratransit, or door-to-door service, is available for those who are unable to independently use the transit system due to a physical or mental disability. Santa Rosa Paratransit is designed to serve the needs of individuals with disabilities within the City of Santa Rosa and the greater Santa Rosa area.

### ***Sonoma County Transit***

Sonoma County Transit (SCT) provides fixed route bus service in the City of Santa Rosa and surrounding communities. The routes operate between the City of Santa Rosa and the City of Sonoma, with stops in both directions at Santa Rosa Memorial Hospital. Route 30 has approximately one- to two-hour headways between 5:00 a.m. and 9:00 p.m. on Monday through Friday and runs four times on the weekends between 7:30 a.m. and 7:00 p.m. Route 34 offers weekday service with one morning trip from Santa Rosa through the Sonoma Valley to the City of Sonoma and one evening trip from Sonoma to Santa Rosa.

# Capacity Analysis

## Intersection Level of Service Methodologies

Level of Service (LOS) is used to rank traffic operation on various types of facilities based on traffic volumes and roadway capacity using a series of letter designations ranging from A to F. Generally, Level of Service A represents free flow conditions and Level of Service F represents forced flow or breakdown conditions. A unit of measure that indicates a level of delay generally accompanies the LOS designation.

The study intersections were analyzed using methodologies published in the *Highway Capacity Manual* (HCM), Transportation Research Board, 2000. This source contains methodologies for various types of intersection control, all of which are related to a measurement of delay in average number of seconds per vehicle.

The Levels of Service for the intersections with side-street stop controls, or those which are unsignalized and have one or two approaches stop controlled, were analyzed using the “Two-Way Stop-Controlled” intersection capacity method from the HCM. This methodology determines a level of service for each minor turning movement by estimating the level of average delay in seconds per vehicle. Results are presented for individual movements together with the weighted overall average delay for the intersection.

The study intersections that are currently controlled by a traffic signal were evaluated using the signalized methodology from the HCM. This methodology is based on factors including traffic volumes, green time for each movement, phasing, whether or not the signals are coordinated, truck traffic, and pedestrian activity. Average stopped delay per vehicle in seconds is used as the basis for evaluation in this LOS methodology. For purposes of this study, delays were calculated using optimized signal timing.

The ranges of delay associated with the various levels of service are indicated in Table 3.

<b>LOS</b>	<b>Two-Way Stop-Controlled</b>	<b>Signalized</b>
A	Delay of 0 to 10 seconds. Gaps in traffic are readily available for drivers exiting the minor street.	Delay of 0 to 10 seconds. Most vehicles arrive during the green phase, so do not stop at all.
B	Delay of 10 to 15 seconds. Gaps in traffic are somewhat less readily available than with LOS A, but no queuing occurs on the minor street.	Delay of 10 to 20 seconds. More vehicles stop than with LOS A, but many drivers still do not have to stop.
C	Delay of 15 to 25 seconds. Acceptable gaps in traffic are less frequent, and drivers may approach while another vehicle is already waiting to exit the side street.	Delay of 20 to 35 seconds. The number of vehicles stopping is significant, although many still pass through without stopping.
D	Delay of 25 to 35 seconds. There are fewer acceptable gaps in traffic, and drivers may enter a queue of one or two vehicles on the side street.	Delay of 35 to 55 seconds. The influence of congestion is noticeable, and most vehicles have to stop.
E	Delay of 35 to 50 seconds. Few acceptable gaps in traffic are available, and longer queues may form on the side street.	Delay of 55 to 80 seconds. Most, if not all, vehicles must stop and drivers consider the delay excessive.
F	Delay of more than 50 seconds. Drivers may wait for long periods before there is an acceptable gap in traffic for exiting the side streets, creating long queues.	Delay of more than 80 seconds. Vehicles may wait through more than one cycle to clear the intersection.

Reference: *Highway Capacity Manual*, Transportation Research Board, 2000

## Traffic Operation Standards

The City of Santa Rosa's adopted Level of Service (LOS) Standard is contained in *Santa Rosa General Plan 2035*. Standard TD-1 states that the City will try to maintain a Level of Service (LOS) D or better along all major corridors. Exceptions to meeting this standard are allowed where attainment would result in significant environmental degradation; where topography or environmental impacts make the improvement impossible; or where attainment would ensure loss of an area's unique character.

While a corridor level of service is applied by the City in its analysis of the entire City as part of the environmental documentation supporting the General Plan, this type of analysis only provides relevant data when performed on a much longer segment than the one included as the study area for the project. Therefore, although the City's standard does not specify criteria for intersections, for the purposes of this study a minimum operation of LOS D for the overall operation of signalized intersections was applied.

## Existing Conditions

The Existing Conditions scenario provides an evaluation of current operation based on existing traffic volumes during the a.m. and p.m. peak periods. This condition does not include project-generated traffic volumes. Volume data was collected when local schools were in session on Tuesday, November 3, 2015.

## Intersection Levels of Service

Under existing conditions, all study intersections are operating acceptably at LOS C or better during both the a.m. and p.m. peak periods. A summary of the intersection level of service calculations is contained in Table 4. Existing traffic volumes are shown in Figure 2, and copies of the Level of Service calculations are provided in Appendix B.

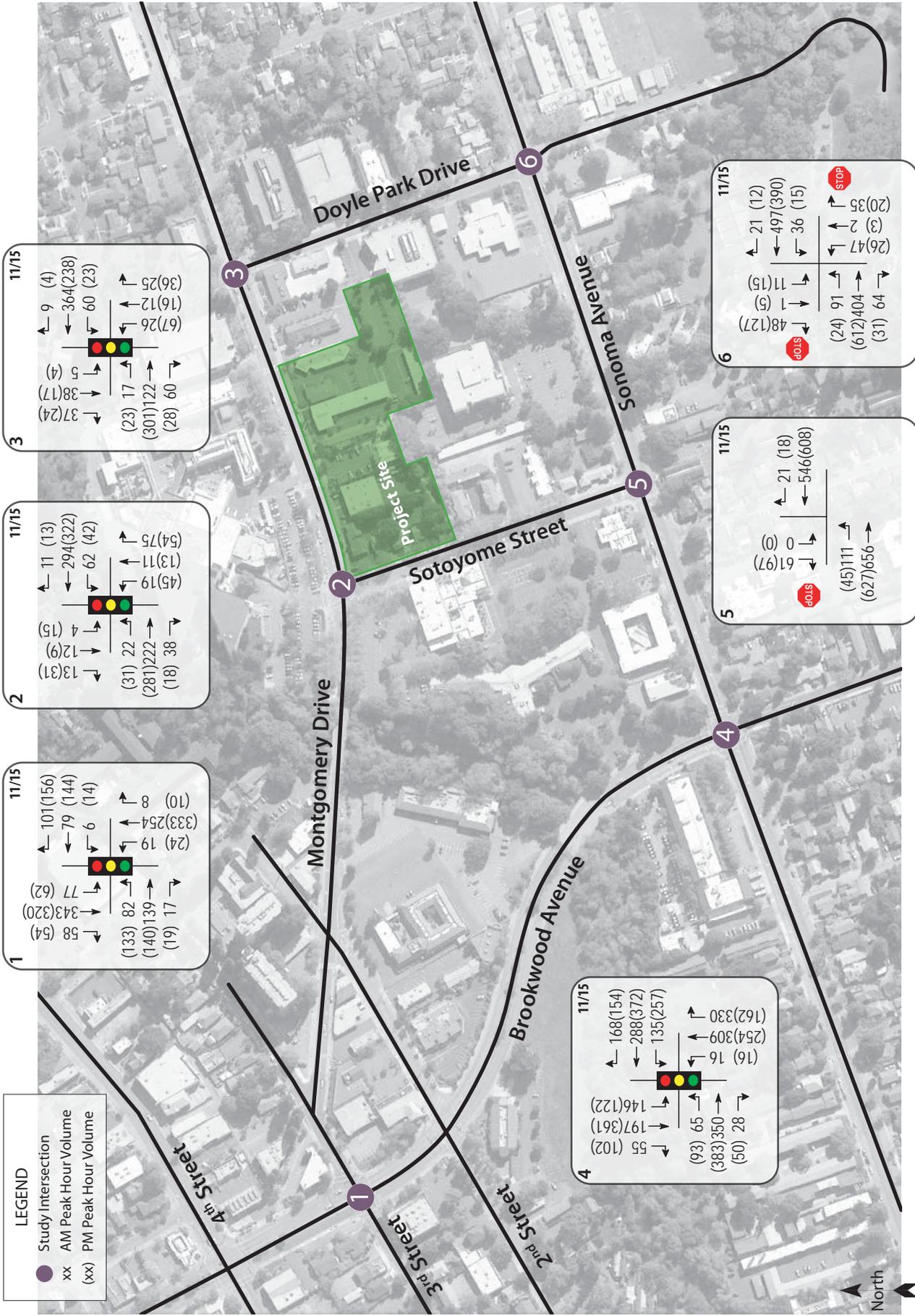
**Table 4 – Existing Peak Hour Intersection Levels of Service**

Study Intersection <i>Approach</i>	AM Peak		PM Peak	
	Delay	LOS	Delay	LOS
1. Brookwood Ave/3 <sup>rd</sup> St	15.8	B	18.5	B
2. Montgomery Dr/Sotoyome St	5.8	A	6.0	A
3. Montgomery Dr/Doyle Park Dr	3.7	A	4.7	A
4. Sonoma Ave/Brookwood Ave	34.3	C	27.2	C
5. Sonoma Ave/Sotoyome St	1.3	A	1.4	A
<i>Southbound (Sotoyome St) Approach</i>	<i>13.3</i>	<i>B</i>	<i>15.7</i>	<i>C</i>
6. Sonoma Ave/Doyle Park Dr	2.8	A	2.5	A
<i>Northbound (Doyle Park Dr) Approach</i>	<i>18.8</i>	<i>C</i>	<i>16.9</i>	<i>C</i>
<i>Southbound (Doyle Park Dr) Approach</i>	<i>14.3</i>	<i>B</i>	<i>14.0</i>	<i>B</i>

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*

## Future Conditions

Segment volumes for the horizon year of 2040 were obtained from the SCTA's gravity demand model and translated to turning movement volumes at each of the study intersections using the "Furness" method and factoring. The Furness method is an iterative process that employs existing turn movement data, existing link volumes, and future link volumes to project likely future turning movement volumes at intersections.



Traffic Impact Study for the Memorial Hospital MOB Project  
**Figure 2 – Existing Traffic Volumes**

Under the anticipated Future volumes, the study intersections are expected to operate acceptably overall at LOS D or better.

Future operating conditions are summarized in Table 5, and Future volumes are shown in Figure 3.

<b>Study Intersection Approach</b>	<b>AM Peak</b>		<b>PM Peak</b>	
	<b>Delay</b>	<b>LOS</b>	<b>Delay</b>	<b>LOS</b>
1. Brookwood Ave/3 <sup>rd</sup> St	18.8	B	20.8	C
2. Montgomery Dr/Sotoyome St	5.8	A	6.0	A
3. Montgomery Dr/Doyle Park Dr	4.4	A	4.3	A
4. Sonoma Ave/Brookwood Ave	37.8	D	28.0	C
5. Sonoma Ave/Sotoyome St	1.6	A	1.7	A
<i>Southbound (Sotoyome St) Approach</i>	<i>14.5</i>	<i>B</i>	<i>16.7</i>	<i>C</i>
6. Sonoma Ave/Doyle Park Dr	3.2	A	2.8	A
<i>Northbound (Doyle Park Dr) Approach</i>	<i>21.2</i>	<i>C</i>	<i>17.3</i>	<i>C</i>
<i>Southbound (Doyle Park Dr) Approach</i>	<i>14.7</i>	<i>C</i>	<i>14.5</i>	<i>B</i>

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*

## Project Description

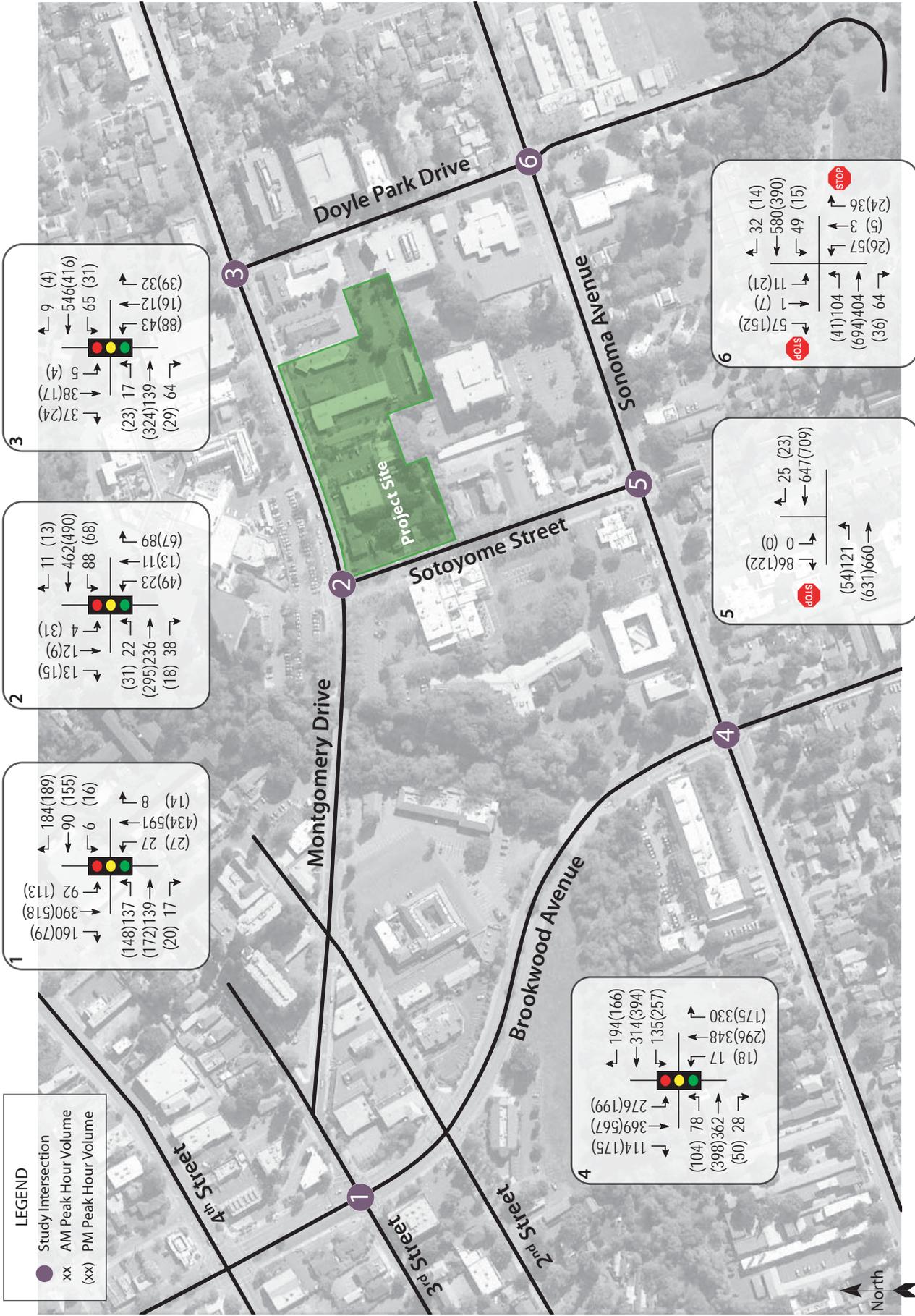
The project consists of development of a 92,000 square-foot medical office building which could include a 2,500 square-foot credit union and a 1,500 square-foot café. The site is bounded by Sotoyome Street to the west, Montgomery Drive to the north, Doyle Park Drive to the east, and Sonoma Avenue to the south. Access would be provided via one driveway each on Sotoyome Street and Montgomery Drive. The proposed project site plan is shown in Figure 4.

## Trip Generation

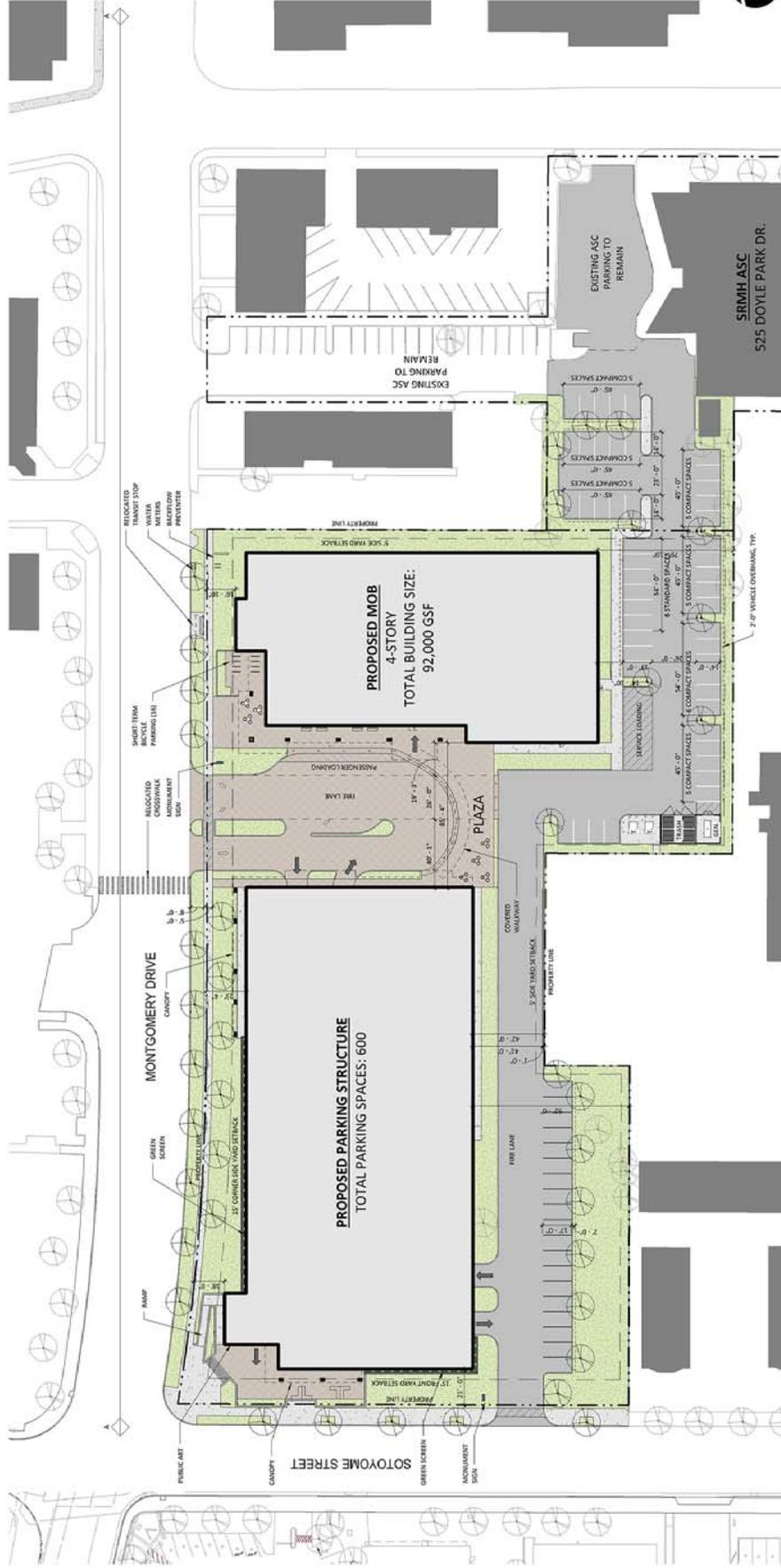
In order to provide the most conservative assessment, the potential ancillary uses of a credit union and café were broken out and trips estimated for them separately. The anticipated trip generation for the proposed project was estimated using standard rates published by the Institute of Transportation Engineers (ITE) in *Trip Generation Manual*, 9<sup>th</sup> Edition, 2012 for “Medical Office Building” (ITE LU #720), “Walk-In Bank” (ITE LU #911), and “Coffee/Donut Shop without Drive-Thru” (ITE LU #936). The tenants of the existing buildings are expected to be relocated to adjacent buildings, so no existing trip credits were applied.

## Pass-by Trips

Some portion of traffic associated with the proposed credit union and café is drawn from existing traffic on nearby streets. These vehicle trips are not considered “new,” but are instead comprised of drivers who are already driving on the adjacent street system and choose to make an interim stop, and are referred to as “pass-by.” The percentage of these pass-by trips was developed based on information provided in the *Trip Generation Manual*. This reference includes pass-by data collected at numerous locations for many land uses, such as fast food restaurant, gas station and retail uses. For this analysis, pass-by rates for fast food restaurant and high-



Traffic Impact Study for the Memorial Hospital MOB Project  
**Figure 3 – Future Traffic Volumes**



Traffic Impact Study for the Memorial Hospital MOB Project  
**Figure 4 – Site Plan**



turnover sit-down restaurant were averaged and applied to the café and walk-in bank. At the proposed project, pass-by trips would in essence be “captured” from traffic on the roadway network.

## Internal Capture Trips

The *Trip Generation Manual* also includes data and methodologies that can be applied to determine the proportion of internal trips that may occur within a development area that includes a variety of land uses. Internal trips occur at mixed-use developments, and in the case of the proposed project would consist of employees and visitors to the medical office building patronizing the adjacent café and credit union. The majority of these trips would be made by walking, and the few that would be made by automobile would only travel on-site, so would not affect the adjacent street network. These rates were applied as a deduction to the overall trips generated by the project after deducting pass-by trips.

## Total Project Trip Generation

The expected trip generation potential for the proposed project is indicated in Table 6, with deductions taken for pass-by and internal capture. After deductions are taken into account, the project would be expected to generate 276 new trips during the morning peak hour and 336 during the evening peak hour; these new trips represent the increase in traffic associated with the project compared to existing volumes.

**Table 6 – Trip Generation Summary**

Land Use	Units	AM Peak Hour				PM Peak Hour			
		Rate	Trips	In	Out	Rate	Trips	In	Out
<b>Proposed</b>									
Medical Office Building	88 ksf	2.39	210	166	44	3.57	314	88	226
Credit Union	2.5 ksf	No a.m. rates (closed)				12.13	30	13	17
Café	1.5 ksf	108.38	163	83	80	40.75	61	50	11
<i>Pass-By for Credit Union and Café</i>		<i>-47%</i>	<i>-77</i>	<i>-39</i>	<i>-38</i>	<i>-47%</i>	<i>-43</i>	<i>-21</i>	<i>-22</i>
Net New after Pass-By			296	210	86		362	111	251
<i>Internal Capture</i>		<i>-7%</i>	<i>-20</i>	<i>-14</i>	<i>-6</i>	<i>-7%</i>	<i>-26</i>	<i>-8</i>	<i>-18</i>
<b>Total New Trips</b>			<b>276</b>	<b>196</b>	<b>80</b>		<b>336</b>	<b>103</b>	<b>233</b>

Note: ksf = 1,000 square feet

On a daily basis, the 92,000 square foot medical office building has an estimated trip generation of 3,324 trips based on the daily rate of 36.13 trips per thousand square feet of floor space. It is noted that the site is currently occupied by numerous small buildings with a total of 37,137 square feet of floor space used for medical offices. While the elimination of this space was not deducted from the project trip generation to provide a more conservative analysis, the net new trip generation associated with the proposed increase of 54,682 square feet is 1,982 daily trips. Further, this project would provide additional medical services in an area where substantial interaction between uses is possible, thereby resulting in the likelihood that some of the trips would be captured from other nearby facilities rather than being a new, primary trip. The estimate of 1,982 new daily trips may therefore still be conservative as it overstates the trip generation due to there being no specific data that would allow the extent of internal capture to be estimated.

## Trip Distribution

The pattern used to allocate new project trips to the street network was based on turning movement data collected at the study intersections. The applied distribution assumptions and resulting trips are shown in Table 7.

**Table 7 – Trip Distribution Assumptions**

Route	Percent	AM Trips	PM Trips
1. Brookwood Ave to/from North	6%	17	20
2. Brookwood Ave to/from South	14%	39	47
3. Doyle Park Dr to/from North	2%	5	7
4. Doyle Park Dr to/from South	2%	5	7
5. Montgomery Dr to/from East	15%	41	50
6. Montgomery Dr to/from West	9%	25	30
7. Sonoma Ave to/from East	30%	83	101
8. Sonoma Ave to/from West	22%	61	74
<b>TOTAL</b>	<b>100%</b>	<b>276</b>	<b>336</b>

## Intersection Operation

### Existing plus Project Conditions

Upon the addition of project-related traffic to the Existing volumes, the study intersections are expected to operate acceptably at the same levels of service as without the project. With the Sonoma Avenue road diet and installation of the two-way left-turn lane on Sonoma Avenue through the intersection of Sonoma Avenue/Sotoyome Street back in 2012, the southbound left-turn restriction is no longer warranted. It was assumed the turn restriction at Sonoma Avenue/Sotoyome Street would be removed as part of the project. The southbound approach would remain stop-controlled, with a single, shared lane for both left and right turns. These results are summarized in Table 8. Project traffic volumes are shown in Figure 5.

**Table 8 – Existing and Existing plus Project Peak Hour Intersection Levels of Service**

Study Intersection Approach	Existing Conditions				Existing plus Project			
	AM Peak		PM Peak		AM Peak		PM Peak	
	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
1. Brookwood Ave/3 <sup>rd</sup> St	15.8	B	18.5	B	16.0	B	19.2	B
2. Montgomery Dr/Sotoyome St	5.8	A	6.0	A	5.8	A	6.2	A
3. Montgomery Dr/Doyle Park Dr	3.7	A	4.7	A	3.8	A	4.9	A
4. Sonoma Ave/Brookwood Ave	34.3	C	27.2	C	38.8	D	29.5	C
5. Sonoma Ave/Sotoyome St	1.3	A	1.4	A	2.6	A	6.8	A
<i>Southbound (Sotoyome St) Approach</i>	<i>13.3</i>	<i>B</i>	<i>15.7</i>	<i>C</i>	<i>19.4</i>	<i>C</i>	<i>40.0</i>	<i>E</i>
6. Sonoma Ave/Doyle Park Dr	2.8	A	2.5	A	2.9	A	2.5	A
<i>Northbound (Doyle Park Dr) Approach</i>	<i>18.8</i>	<i>C</i>	<i>16.9</i>	<i>C</i>	<i>21.3</i>	<i>C</i>	<i>18.4</i>	<i>C</i>
<i>Southbound (Doyle Park Dr) Approach</i>	<i>14.3</i>	<i>B</i>	<i>14.0</i>	<i>B</i>	<i>15.2</i>	<i>C</i>	<i>14.7</i>	<i>C</i>

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*

**Finding** – The study intersections are expected to continue operating acceptably at the same levels of service upon the addition of project-generated traffic.



**Recommendation** – The existing turn restrictions at Sonoma Avenue/Sotoyome Street should be eliminated as part of the project.

## Future plus Project Conditions

When the project is built, it is planned that the intersection of Sonoma Avenue/Sotoyome Street will be modified with southbound left-turns, which are currently prohibited, allowed. Upon the addition of project-generated traffic to the anticipated Future volumes, the study intersections are expected to operate acceptably at the same levels of service as without the project. The Future plus Project operating conditions are summarized in Table 9.

**Table 9 – Future and Future plus Project Peak Hour Levels of Service**

Study Intersection <i>Approach</i>	Future Conditions				Future plus Project			
	AM Peak		PM Peak		AM Peak		PM Peak	
	Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
1. Brookwood Ave/3 <sup>rd</sup> St	18.8	B	20.8	C	19.2	B	22.7	C
2. Montgomery Dr/Sotoyome St	5.8	A	6.0	A	5.8	A	6.3	A
3. Montgomery Dr/Doyle Park Dr	4.4	A	4.3	A	4.1	A	4.3	A
4. Sonoma Ave/Brookwood Ave	37.8	D	28.0	C	40.9	D	30.1	C
5. Sonoma Ave/Sotoyome St	1.6	A	1.7	A	2.8	A	6.4	A
<i>Southbound (Sotoyome St) Approach</i>	<i>14.5</i>	<i>B</i>	<i>16.7</i>	<i>C</i>	<i>20.3</i>	<i>C</i>	<i>37.3</i>	<i>E</i>
6. Sonoma Ave/Doyle Park Dr	3.2	A	2.8	A	3.3	A	2.8	A
<i>Northbound (Doyle Park Dr) Approach</i>	<i>21.2</i>	<i>C</i>	<i>17.3</i>	<i>C</i>	<i>24.4</i>	<i>C</i>	<i>18.8</i>	<i>C</i>
<i>Southbound (Doyle Park Dr) Approach</i>	<i>14.7</i>	<i>C</i>	<i>14.5</i>	<i>B</i>	<i>15.5</i>	<i>C</i>	<i>15.4</i>	<i>C</i>

Notes: Delay is measured in average seconds per vehicle; LOS = Level of Service; Results for minor approaches to two-way stop-controlled intersections are indicated in *italics*

**Finding** – The study intersections will continue operating acceptably with project traffic added, at the same Levels of Service as without the project.

# Alternative Modes

---

## Pedestrian Facilities

Given the proximity of the downtown area and commercial, retail, and residential land uses surrounding the project site, it is reasonable to assume that some project trips will occur by foot, bicycle, or public transit to reach the project site. The two existing midblock crossings on Montgomery Drive between Sotoyome Street and Doyle Park Drive will be consolidated into one crossing on the west side of the proposed driveway. This is an appropriate modification to the pedestrian facilities in the area as it decreases the number of locations for potential pedestrian-vehicle conflicts on that section of Montgomery Drive. Analysis was completed to determine if the crosswalk should be enhanced. Based on a major road volume of 679 vehicles on Montgomery Drive between Sotoyome Street and Doyle Park Drive during the peak hour and 61 pedestrians crossing Montgomery Drive during the p.m. peak hour, an active or enhanced crossing is warranted, as shown in Appendix C. The applicant should install a crossing with Rapid Rectangular Flashing Beacons (RRFBs).

**Project Site** – Sidewalks exist along all of the project frontages. A multi-use path paralleling the Santa Rosa Creek Trail is proposed within the project site.

**Finding** – Upon the installation of a crosswalk with RRFBs at the new crossing location on Montgomery Drive between Sotoyome Street and Doyle Park Drive, pedestrian facilities serving the project site will be adequate.

## Bicycle Facilities

Existing bicycle facilities, including bike lanes on Brookwood Avenue, Montgomery Drive, and Sonoma Avenue, together with shared use of minor streets provide adequate access for bicyclists.

## Bicycle Storage

The Santa Rosa City Code stipulates that a minimum bicycle requirement for a Medical service land use is one space per 6,000 square feet. At that rate, 15 bicycle parking spaces should be provided on-site; 25 percent of the spaces should be provided in facilities adequate for long-term use. Additionally, one shower for each gender is required for office uses between 50,000 square-feet and 149,999 square-feet.

The project description indicates there will be long- and short-term bicycle parking and shower facilities provided in accordance with the Santa Rosa City Code requirements.

**Finding** – Upon the addition of 15 bicycle parking spaces and two showers, bicycle facilities serving the project site will be adequate.

## Transit

Existing transit routes are adequate to accommodate project-generated transit trips. Existing stops are within acceptable walking distance of the site. In addition to the existing stops, a bus stop area with a shelter is proposed in front of the project.

**Finding** – Existing and proposed transit facilities serving the project site are adequate.

# Access and Circulation

---

## Site Access

Access would be provided via driveways on Sotoyome Street and Montgomery Drive.

## Sight Distance

At driveways, a substantially clear line of sight should be maintained between the driver of a vehicle waiting at the crossroad and the driver of an approaching vehicle. Adequate time must be provided for the waiting vehicle to either cross, turn left, or turn right, without requiring the through traffic to radically alter their speed. Sight distance should be measured from a 3.5-foot height at the location of the driver on the minor road to a 4.25-foot object height in the center of the approaching lane of the major road. Set-back for the driver on the crossroad shall be a minimum of 15 feet, measured from the edge of the traveled way.

Sight distances along Sotoyome Street and Montgomery Drive at the proposed driveways were evaluated based on criteria contained in the *Highway Design Manual* published by Caltrans. The recommended sight distance at driveways is based on stopping sight distance and the approach travel speeds. Additionally, the stopping sight distance needed for a following driver to stop if there is a vehicle waiting to turn into a driveway is evaluated based on stopping sight distance criterion and the approach speed on the major street. The minimum stopping sight distance for the project driveways on Sotoyome Street and Montgomery Drive is 200 feet based on the posted 25-mph speed limits.

From a review of the proposed site plan as well as site observations, sight distance for vehicles exiting the project site on Montgomery Drive is adequate in both directions as the driveway is on the outside of a curve. Sight distance along Sotoyome Street is limited due to on-street parking along the project frontage. However, once a vehicle creeps out from the project driveway to make a left turn, and while still clear of the travel lane, sight distance is clear to at least 300 feet in each direction, or to the adjacent intersection.

In order to maintain adequate sight lines for vehicles leaving the site, it is recommended that landscaping at both driveways be limited to low-lying vegetation no greater than three feet in height. In addition, signs and monuments planned along the project's frontages should be placed in a manner that does not obstruct sight distance at the project driveways. Finally, parking should be prohibited through installation of red curbing for a distance of 35 feet on either side of the project driveway on Sotoyome Street.

At the project driveways, the required 200 feet of sight distance would be provided and would therefore be adequate.

## Traffic Signal Warrants

Although the intersection of Sonoma Avenue/Sotoyome Street is expected to operate acceptably overall under all scenarios evaluated, because the project would substantially increase delay on the stop-controlled approach, a signal warrant analysis was performed to determine potential need for a traffic signal to offset project impacts.

Chapter 4C of the *California Manual on Uniform Traffic Control Devices* (CA-MUTCD) provides guidance on when a traffic signal should be considered. There are nine different warrants, or criteria, presented, as follows:

- Warrant 1, Eight-Hour Vehicular Volume
- Warrant 2, Four-Hour Vehicular Volume
- Warrant 3, Peak Hour Volume

- Warrant 4, Pedestrian Volume
- Warrant 5, School Crossing
- Warrant 6, Coordinated Signal System
- Warrant 7, Crash Experience
- Warrant 8, Roadway Network
- Warrant 9, Intersection Near a Grade Crossing

For the purposes of this study, Warrant 3, the Peak Hour volume warrant, which determines the need for traffic control based on the highest volume hour of the day, was used as an initial indication of traffic control needs. The use of this signal warrant is common practice for planning studies. Other warrants, which are more generally applicable to existing traffic issues, require collection of traffic volumes for the highest four or eight hours of the day, review of the collision history, and evaluation of the system surrounding the location. It is noted that review of the collision data indicates that there are no safety issues that would warrant signalization.

**Warrant 3** assesses potential need for a traffic control signal if the criteria in either of the following two categories are met:

- A. If all three of the following conditions exist for the same one hour (any four consecutive 15-minute periods) of an average day:
  1. The total stopped time delay experienced by the traffic on one minor-street approach (one direction only) controlled by a STOP sign equals or exceeds: four vehicle-hours for a one-lane approach; or five vehicle-hours for a two-lane approach, and
  2. The volume on the same minor-street approach (one direction only) equals or exceeds 100 vehicles per hour for one moving lane of traffic or 150 vehicles per hour for two moving lanes, and
  3. The total entering volume serviced during the hour equals or exceeds 650 vehicles per hour for intersections with three approaches or 800 vehicles per hour for intersections with four or more approaches.
  
- B. The plotted point representing the vehicles per hour on the major street (total of both approaches) and the corresponding vehicles per hour on the higher-volume minor-street approach (one direction only) for one hour (any four consecutive 15-minute periods) of an average day falls above the applicable curve in Figure 4C-3 for the existing combination of approach lanes.

Under traffic volumes that are anticipated during both the a.m. and p.m. peak hours with project trips added to existing conditions, a signal is warranted at the intersection of Sonoma Avenue/Sotoyome Street. The applicant should consider installing a traffic signal to reduce impacts on local traffic. Traffic signal warrants are included in Appendix D.

# Parking

---

The project was analyzed to determine whether the proposed parking supply would be sufficient for the anticipated parking demand. The project site as proposed would provide a total of 619 standard parking spaces for the medical office building, café, and credit union.

Jurisdiction parking supply requirements are based on the Santa Rosa City Code, Chapter 20-36; Parking and Loading Standards. The parking supply as proposed substantially exceeds the parking required per the City's code for all uses at the site as shown in Table 10.

<b>Table 10 – Parking Analysis Summary</b>			
<b>Land Use</b>	<b>Units</b>	<b>City Requirements</b>	
		<b>Rate</b>	<b>Spaces Required</b>
Medical Office Building	88 ksf	1 space per 200 sf	440
Credit Union	2.5 ksf (incl. 1 ATM)	1 space per 250 sf + 2 spaces per ATM	12
Café	1.5 ksf	1 space per 75 sf	20
<b>Total</b>			<b>472</b>

Notes: ksf = 1,000 square feet; sf = square feet

**Finding** – The proposed parking supply for the project is adequate to meet the City's code.

# Conclusions and Recommendations

---

## Conclusions

- The proposed 92,000 square-foot medical office building, potentially including a 2,500 square-foot credit union and 1,500 square-foot café, is expected to generate an average of 276 new trips during the morning peak hour and 336 during the evening peak hour. It has an anticipated daily trip generation of 3,324 without consideration of potential to capture trips associated with other nearby medical services and offices.
- The intersections of Brookwood Avenue/3<sup>rd</sup> Street and Sonoma Avenue/Brookwood Avenue experienced collision rates higher than the statewide averages during the five-year period reviewed.
- All of the study intersections are currently operating acceptably, and they are expected to continue doing so under projected future volumes.
- Upon adding project trips to existing volumes and assuming full outbound access at Sonoma Avenue/Sotoyome Street, all of the study intersections are expected to operate acceptably.
- With project trips added to Future volumes, all of the study intersections will continue operating acceptably at the same levels of service.
- Upon the addition of RRFBs to the proposed mid-block crossing on Montgomery Drive, which would replace two existing mid-block crosswalks on the same block, pedestrian facilities serving the project site will be adequate.
- Generally, bicycle facilities serving the project site are adequate so long as 15 bicycle parking spaces and two showers are provided.
- Existing and proposed transit facilities serving the project site are adequate.
- Sight distance along Montgomery Drive at the project driveway is adequate. With the addition of red curb on either side of the Sotoyome Street driveway, access to the site is expected to function acceptably.
- A traffic signal is warranted at the intersection of Sonoma Avenue/Sotoyome Street.
- The proposed parking supply substantially exceeds the minimum required under the City's code.

## Recommendations

- The City should consider adding or increasing red-clearance intervals at the intersections of Brookwood Avenue/3<sup>rd</sup> Street and Sonoma Avenue/Brookwood Avenue. Additionally, the City should monitor the protected/permitted operation at Sonoma Avenue/Brookwood Avenue and consider converting Sonoma Avenue to protected lefts only should the incidence of correctible collisions increase.
- An enhanced crossing with RRFBs should be installed at the proposed mid-block crossing on Montgomery Drive.
- One shower for each gender should be provided on-site.
- Bicycle parking to adequately meet City requirements should be provided.

- Landscaping should be maintained such that foliage stays above seven feet and below three feet from the ground within the sight lines at the driveways. Signs or monuments to be installed along the project frontage should be placed so that sight distance is not obstructed at the project driveways. Further, red paint should be applied to the curbs for 35 feet on either side of the project driveway on Sotoyome Street to improve sight lines.
- The applicant should consider including installation of a traffic signal at the intersection of Sonoma Avenue/ Sotoyome Street as part of the project proposal.

# Study Participants and References

---

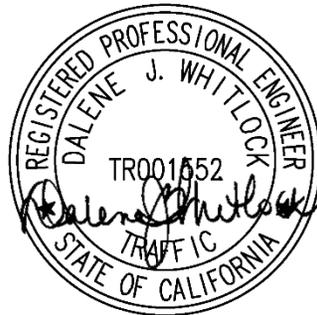
## Study Participants

Principal in Charge	Dalene J. Whitlock, PE, PTOE
Assistant Engineer	Lauren Davini, PE
Graphics/Editing/Formatting	Angela McCoy

## References

*2012 Collision Data on California State Highways*, California Department of Transportation, 2012  
*Design Information Bulletin Number 89: Class IV Bikeway Guidance (Separated Bikeways/Cycle Tracks)*, California Department of Transportation, 2015  
*Highway Capacity Manual*, Transportation Research Board, 2000  
*Highway Design Manual*, 6<sup>th</sup> Edition, California Department of Transportation, 2012  
*Santa Rosa Bicycle and Pedestrian Master Plan*, City of Santa Rosa, 2010  
*Santa Rosa City Code*, Quality Code Publishing, 2016  
Santa Rosa CityBus, [http://ci.santa-rosa.ca.us/departments/transit/citybus/maps\\_schedules/Pages/default.aspx](http://ci.santa-rosa.ca.us/departments/transit/citybus/maps_schedules/Pages/default.aspx)  
*Santa Rosa General Plan 2035*, City of Santa Rosa, 2014  
Sonoma County Transit, <http://sctransit.com/>  
*Statewide Integrated Traffic Records System*, California Highway Patrol, 2011-2016  
*Trip Generation Manual*, 9<sup>th</sup> Edition, Institute of Transportation Engineers, 2012

SRO376



# Appendix A

---

## Collision Rate Calculations



**Intersection Collision Rate Calculations**

**Santa Rosa Memorial Hospital MOB Project**

**Intersection # 1:** Brookwood Avenue & 3rd Street  
**Date of Count:** Wednesday, October 14, 2009

**Number of Collisions:** 10  
**Number of Injuries:** 5  
**Number of Fatalities:** 0  
**ADT:** 14100  
**Start Date:** May 1, 2011  
**End Date:** April 30, 2016  
**Number of Years:** 5

**Intersection Type:** Four-Legged  
**Control Type:** Signals  
**Area:** Urban

$$\text{collision rate} = \frac{\text{Number of Collisions} \times 1 \text{ Million}}{\text{ADT} \times 365 \text{ Days per Year} \times \text{Number of Years}}$$

$$\text{collision rate} = \frac{10}{14,100} \times \frac{1,000,000}{365 \times 5}$$

	Collision Rate	Fatality Rate	Injury Rate
<b>Study Intersection</b>	<b>0.39 c/mve</b>	<b>0.0%</b>	<b>50.0%</b>
<b>Statewide Average*</b>	<b>0.27 c/mve</b>	<b>0.4%</b>	<b>41.9%</b>

ADT = average daily total vehicles entering intersection  
 c/mve = collisions per million vehicles entering intersection  
 \* 2012 Collision Data on California State Highways, Caltrans

**Intersection # 2:** Montgomery Drive & Sotoyome Street  
**Date of Count:** Tuesday, November 03, 2015

**Number of Collisions:** 4  
**Number of Injuries:** 3  
**Number of Fatalities:** 0  
**ADT:** 8700  
**Start Date:** May 1, 2011  
**End Date:** April 30, 2016  
**Number of Years:** 5

**Intersection Type:** Four-Legged  
**Control Type:** Signals  
**Area:** Urban

$$\text{collision rate} = \frac{\text{Number of Collisions} \times 1 \text{ Million}}{\text{ADT} \times 365 \text{ Days per Year} \times \text{Number of Years}}$$

$$\text{collision rate} = \frac{4}{8,700} \times \frac{1,000,000}{365 \times 5}$$

	Collision Rate	Fatality Rate	Injury Rate
<b>Study Intersection</b>	<b>0.25 c/mve</b>	<b>0.0%</b>	<b>75.0%</b>
<b>Statewide Average*</b>	<b>0.27 c/mve</b>	<b>0.4%</b>	<b>41.9%</b>

ADT = average daily total vehicles entering intersection  
 c/mve = collisions per million vehicles entering intersection  
 \* 2012 Collision Data on California State Highways, Caltrans

**Intersection Collision Rate Calculaions**

**Santa Rosa Memorial Hospital MOB Project**

**Intersection # 3:** Montgomery Drive & Doyle Park Drive  
**Date of Count:** Tuesday, November 03, 2015

**Number of Collisions:** 2  
**Number of Injuries:** 2  
**Number of Fatalities:** 0  
**ADT:** 7800  
**Start Date:** May 1, 2011  
**End Date:** April 30, 2016  
**Number of Years:** 5

**Intersection Type:** Four-Legged  
**Control Type:** Signals  
**Area:** Urban

$$\text{collision rate} = \frac{\text{Number of Collisions} \times 1 \text{ Million}}{\text{ADT} \times 365 \text{ Days per Year} \times \text{Number of Years}}$$

$$\text{collision rate} = \frac{2}{7,800} \times \frac{1,000,000}{365 \times 5}$$

	<b>Collision Rate</b>	<b>Fatality Rate</b>	<b>Injury Rate</b>
<b>Study Intersection</b>	<b>0.14 c/mve</b>	<b>0.0%</b>	<b>100.0%</b>
<b>Statewide Average*</b>	<b>0.27 c/mve</b>	<b>0.4%</b>	<b>41.9%</b>

ADT = average daily total vehicles entering intersection  
c/mve = collisions per million vehicles entering intersection  
\* 2012 Collision Data on California State Highways, Caltrans

**Intersection # 4:** Sonoma Avenue & Brookwood Avenue  
**Date of Count:** Tuesday, November 03, 2015

**Number of Collisions:** 21  
**Number of Injuries:** 17  
**Number of Fatalities:** 0  
**ADT:** 23300  
**Start Date:** May 1, 2011  
**End Date:** April 30, 2016  
**Number of Years:** 5

**Intersection Type:** Four-Legged  
**Control Type:** Signals  
**Area:** Urban

$$\text{collision rate} = \frac{\text{Number of Collisions} \times 1 \text{ Million}}{\text{ADT} \times 365 \text{ Days per Year} \times \text{Number of Years}}$$

$$\text{collision rate} = \frac{21}{23,300} \times \frac{1,000,000}{365 \times 5}$$

	<b>Collision Rate</b>	<b>Fatality Rate</b>	<b>Injury Rate</b>
<b>Study Intersection</b>	<b>0.49 c/mve</b>	<b>0.0%</b>	<b>81.0%</b>
<b>Statewide Average*</b>	<b>0.27 c/mve</b>	<b>0.4%</b>	<b>41.9%</b>

ADT = average daily total vehicles entering intersection  
c/mve = collisions per million vehicles entering intersection  
\* 2012 Collision Data on California State Highways, Caltrans

**Intersection Collision Rate Calculations**

**Santa Rosa Memorial Hospital MOB Project**

**Intersection # 5:** Sonoma Avenue & Sotoyome Street  
**Date of Count:** Tuesday, November 03, 2015

**Number of Collisions:** 3  
**Number of Injuries:** 2  
**Number of Fatalities:** 0  
**ADT:** 14300  
**Start Date:** May 1, 2011  
**End Date:** April 30, 2016  
**Number of Years:** 5

**Intersection Type:** Tee  
**Control Type:** Stop & Yield Controls  
**Area:** Urban

$$\text{collision rate} = \frac{\text{Number of Collisions} \times 1 \text{ Million}}{\text{ADT} \times 365 \text{ Days per Year} \times \text{Number of Years}}$$

$$\text{collision rate} = \frac{3}{14,300} \times \frac{1,000,000}{365 \times 5}$$

	<b>Collision Rate</b>	<b>Fatality Rate</b>	<b>Injury Rate</b>
<b>Study Intersection</b>	<b>0.11 c/mve</b>	<b>0.0%</b>	<b>66.7%</b>
<b>Statewide Average*</b>	<b>0.18 c/mve</b>	<b>0.7%</b>	<b>36.4%</b>

ADT = average daily total vehicles entering intersection  
 c/mve = collisions per million vehicles entering intersection  
 \* 2012 Collision Data on California State Highways, Caltrans

**Intersection # 6:** Sonoma Avenue & Doyle Park Drive  
**Date of Count:** Tuesday, November 03, 2015

**Number of Collisions:** 0  
**Number of Injuries:** 0  
**Number of Fatalities:** 0  
**ADT:** 12800  
**Start Date:** May 1, 2011  
**End Date:** April 30, 2016  
**Number of Years:** 5

**Intersection Type:** Offset  
**Control Type:** Stop & Yield Controls  
**Area:** Urban

$$\text{collision rate} = \frac{\text{Number of Collisions} \times 1 \text{ Million}}{\text{ADT} \times 365 \text{ Days per Year} \times \text{Number of Years}}$$

$$\text{collision rate} = \frac{0}{12,800} \times \frac{1,000,000}{365 \times 5}$$

	<b>Collision Rate</b>	<b>Fatality Rate</b>	<b>Injury Rate</b>
<b>Study Intersection</b>	<b>0.00 c/mve</b>	<b>0.0%</b>	<b>0.0%</b>
<b>Statewide Average*</b>	<b>0.15 c/mve</b>	<b>1.0%</b>	<b>41.9%</b>

ADT = average daily total vehicles entering intersection  
 c/mve = collisions per million vehicles entering intersection  
 \* 2012 Collision Data on California State Highways, Caltrans



# Appendix B

---

## Intersection Level of Service Calculations





HCM Signalized Intersection Capacity Analysis  
 1: Brookwood Avenue & 3rd Street

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB	
Traffic Volume (vph)	82	139	17	6	79	101	19	254	8	77	343	58	
Future Volume (vph)	82	139	17	6	79	101	19	254	8	77	343	58	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Frb. ped/bikes	1.00	1.00	1.00	1.00	0.98	1.00	1.00	1.00	1.00	1.00	1.00	0.99	
Fllb. ped/bikes	1.00	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	1.00	1.00	0.98	
Frt	1.00	0.98	1.00	1.00	0.92	1.00	1.00	1.00	1.00	1.00	1.00	0.98	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	1770	1828	1754	1676	1743	3518	1770	3438	1770	3438	1770	3438	
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (perm)	1770	1828	1754	1676	1743	3518	1770	3438	1770	3438	1770	3438	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	89	151	18	7	86	110	21	276	9	84	373	63	
RTOR Reduction (vph)	0	4	0	0	54	0	0	3	0	0	15	0	
Lane Group Flow (vph)	89	165	0	7	142	0	21	282	0	84	421	0	
Confl. Peds. (#/hr)	20	20	20	20	20	20	20	20	20	20	20	20	
Turn Type	Prot	NA	NA	Prot	NA	NA	Prot	NA	Prot	NA	NA	NA	
Permitted Phases	5	2		1	6	3	8		7	4			
Protected Phases				6									
Actuated Green, G (s)	4.9	19.2	0.9	15.2	0.9	13.1	0.9	13.1	4.5	17.1			
Effective Green, g (s)	4.9	19.2	0.9	15.2	0.9	13.1	0.9	13.1	4.5	17.1			
Actuated q/C Ratio	0.10	0.38	0.02	0.30	0.02	0.26	0.02	0.26	0.09	0.34			
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	172	697	31	506	31	916	31	916	158	1168			
v/s Ratio Prot	c0.05	0.09	0.00	c0.08	0.01	0.08	0.01	0.08	c0.05	c0.12			
v/s Ratio Perm													
v/c Ratio	0.52	0.24	0.23	0.28	0.68	0.31	0.68	0.31	0.53	0.36			
Uniform Delay, d1	21.6	10.6	24.4	13.4	24.6	15.0	21.9	12.5	21.9	12.5			
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Incremental Delay, d2	2.6	0.2	3.7	0.3	45.6	0.2	45.6	0.2	3.4	0.2			
Delay (s)	24.2	10.7	28.0	13.7	70.2	15.1	70.2	15.1	25.3	12.7			
Level of Service	C	B	C	B	E	B	E	B	C	B			
Approach Delay (s)	15.4		14.2		18.9		14.7		14.7				
Approach LOS	B		B		B		B		B				
Intersection Summary													
HCM 2000 Control Delay	15.8											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.40												
Actuated Cycle Length (s)	50.3											Sum of lost time (s)	12.6
Intersection Capacity Utilization	54.5%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
 1: Brookwood Avenue & 3rd Street

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB	
Traffic Volume (vph)	133	140	19	14	144	156	24	333	10	62	320	54	
Future Volume (vph)	133	140	19	14	144	156	24	333	10	62	320	54	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Frb. ped/bikes	1.00	1.00	1.00	1.00	0.98	1.00	1.00	1.00	1.00	1.00	1.00	0.99	
Fllb. ped/bikes	1.00	1.00	1.00	1.00	0.92	1.00	1.00	1.00	1.00	1.00	1.00	0.98	
Frt	1.00	0.98	1.00	1.00	0.92	1.00	1.00	1.00	1.00	1.00	1.00	0.98	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	1770	1823	1770	1691	1770	3519	1770	3519	1770	3436	1770	3436	
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (perm)	1770	1823	1770	1691	1770	3519	1770	3519	1770	3436	1770	3436	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	145	152	21	15	157	170	26	362	11	67	348	59	
RTOR Reduction (vph)	0	4	0	0	42	0	0	2	0	0	16	0	
Lane Group Flow (vph)	145	169	0	15	285	0	26	371	0	67	391	0	
Confl. Peds. (#/hr)	20	20	20	20	20	20	20	20	20	20	20	20	
Turn Type	Prot	NA	NA	Prot	NA	NA	Prot	NA	Prot	NA	NA	NA	
Permitted Phases	5	2		1	6	3	8		7	4			
Protected Phases				6									
Actuated Green, G (s)	7.9	25.6	1.0	18.7	2.5	13.9	2.5	13.9	4.5	16.3			
Effective Green, g (s)	7.9	25.6	1.0	18.7	2.5	13.9	2.5	13.9	4.5	16.3			
Actuated q/C Ratio	0.14	0.44	0.02	0.32	0.04	0.24	0.04	0.24	0.08	0.28			
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2		
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	242	810	30	548	76	849	76	849	138	972			
v/s Ratio Prot	c0.08	0.09	0.01	c0.17	0.01	0.11	0.01	0.11	c0.04	c0.11			
v/s Ratio Perm													
v/c Ratio	0.60	0.21	0.50	0.52	0.34	0.44	0.34	0.44	0.49	0.40			
Uniform Delay, d1	23.4	9.8	28.1	15.8	26.8	18.5	25.4	16.7	25.4	16.7			
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Incremental Delay, d2	4.0	0.1	12.5	0.9	2.7	0.4	2.7	0.4	2.7	0.3			
Delay (s)	27.3	9.9	40.6	16.7	29.4	18.9	28.1	17.0	28.1	17.0			
Level of Service	C	A	D	B	C	B	C	B	C	B			
Approach Delay (s)	17.9		17.7		19.6		17.7		17.7				
Approach LOS	B		B		B		B		B				
Intersection Summary													
HCM 2000 Control Delay	18.5											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.50												
Actuated Cycle Length (s)	57.6											Sum of lost time (s)	12.6
Intersection Capacity Utilization	59.1%											ICU Level of Service	B
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
 2. Sotoyome Street & Montgomery Drive

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	2	2	2	2	2	2	2	2	2	2	2
Traffic Volume (vph)	22	222	38	62	294	11	19	11	75	4	12
Future Volume (vph)	22	222	38	62	294	11	19	11	75	4	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fpb. ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.95	1.00
Fllb. ped/bikes	0.99	1.00	0.99	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00
Frt	1.00	0.98	1.00	0.99	1.00	1.00	1.00	0.85	1.00	0.94	1.00
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.97	1.00	0.99	1.00	0.99	1.00
Satd. Flow (prot)	1744	1812	1749	1850	1712	1517	1650	1650	1712	1517	1650
Flt Permitted	0.54	1.00	0.58	1.00	0.89	1.00	0.89	1.00	0.98	1.00	0.98
Satd. Flow (perm)	998	1812	1077	1850	1564	1517	1629	1629	1564	1517	1629
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Adj. Flow (vph)	24	239	41	67	316	12	20	12	81	4	13
RTOR Reduction (vph)	0	12	0	0	3	0	0	0	58	0	10
Lane Group Flow (vph)	24	268	0	67	325	0	0	32	23	0	21
Confl. Peds. (#/hr)	24	18	18	24	109	28	28	28	28	28	109
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	2	2	2	2	4	4	4	4	4
Prohibited Phases											
Actuated Green, G (s)	23.8	23.8	23.8	23.8	23.8	11.8	11.8	11.8	11.8	11.8	11.8
Effective Green, g (s)	23.8	23.8	23.8	23.8	23.8	11.8	11.8	11.8	11.8	11.8	11.8
Actuated g/C Ratio	0.57	0.57	0.57	0.57	0.57	0.28	0.28	0.28	0.28	0.28	0.28
Clearance Time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	2.0	2.0	2.0	2.0	2.0	2.0
Lane Grp Cap (vph)	568	1031	613	1053	441	428	459	459	428	459	459
v/s Ratio Prot	0.02	0.15	0.06	0.06	c0.18	c0.02	0.02	0.02	0.02	0.01	0.01
v/s Ratio Perm	0.04	0.26	0.11	0.31	0.11	0.07	0.05	0.05	0.05	0.05	0.05
Uniform Delay, d1	4.0	4.5	4.1	4.7	11.0	10.9	10.9	10.9	10.9	10.9	10.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.0	0.1	0.1	0.2	0.1	0.0	0.0	0.0	0.0	0.0	0.0
Delay (s)	4.0	4.7	4.2	4.9	11.0	10.9	10.9	10.9	10.9	10.9	10.9
Level of Service	A	A	A	A	A	B	B	B	B	B	B
Approach Delay (s)	4.6	4.6	4.6	4.8	11.0	10.9	10.9	10.9	10.9	10.9	10.9
Approach LOS	A	A	A	A	A	B	B	B	B	B	B
Intersection Summary											
HCM 2000 Control Delay	5.8										
HCM 2000 Volume to Capacity ratio	0.23										
Actuated Cycle Length (s)	41.8										
Intersection Capacity Utilization	63.3%										
Analysis Period (min)	15										
c Critical Lane Group											

Traffic Study for the Santa Rosa Memorial Hospital  
 AM Existing

Synchro 8 Report

HCM Signalized Intersection Capacity Analysis  
 2. Sotoyome Street & Montgomery Drive

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	3	1	1	3	1	1	3	1	1	3	1
Traffic Volume (vph)	31	281	18	42	322	13	45	13	54	15	9
Future Volume (vph)	31	281	18	42	322	13	45	13	54	15	9
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Fpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.97	1.00	0.95	1.00
Fllb. ped/bikes	0.98	1.00	0.98	1.00	1.00	1.00	0.95	1.00	1.00	1.00	1.00
Frt	1.00	0.99	1.00	0.99	1.00	1.00	1.00	0.85	1.00	0.92	1.00
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.96	1.00	0.96	1.00	0.99	1.00
Satd. Flow (prot)	1736	1841	1740	1848	1705	1529	1608	1608	1705	1529	1608
Flt Permitted	0.51	1.00	0.54	1.00	0.81	1.00	0.81	1.00	0.94	1.00	0.94
Satd. Flow (perm)	934	1841	995	1848	1436	1529	1535	1535	1436	1529	1535
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	34	309	20	46	354	14	49	14	59	16	10
RTOR Reduction (vph)	0	5	0	0	3	0	0	0	43	0	25
Lane Group Flow (vph)	34	324	0	46	365	0	0	63	16	0	35
Confl. Peds. (#/hr)	34	28	28	34	85	18	18	18	18	18	85
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	2	2	2	2	4	4	4	4	4
Prohibited Phases											
Actuated Green, G (s)	24.3	24.3	24.3	24.3	24.3	11.7	11.7	11.7	11.7	11.7	11.7
Effective Green, g (s)	24.3	24.3	24.3	24.3	24.3	11.7	11.7	11.7	11.7	11.7	11.7
Actuated g/C Ratio	0.58	0.58	0.58	0.58	0.58	0.28	0.28	0.28	0.28	0.28	0.28
Clearance Time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	2.0	2.0	2.0	2.0	2.0	2.0
Lane Grp Cap (vph)	537	1060	613	1064	398	423	425	425	398	423	425
v/s Ratio Prot	0.04	0.18	0.05	0.05	c0.20	c0.04	0.01	0.01	0.01	0.02	0.02
v/s Ratio Perm	0.06	0.31	0.08	0.34	0.08	0.16	0.04	0.04	0.04	0.08	0.08
Uniform Delay, d1	3.9	4.6	4.0	4.7	11.5	11.1	11.1	11.1	11.1	11.1	11.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.0	0.2	0.1	0.2	0.1	0.0	0.1	0.0	0.0	0.0	0.0
Delay (s)	4.0	4.8	4.0	4.9	11.6	11.2	11.3	11.3	11.2	11.3	11.3
Level of Service	A	A	A	A	A	B	B	B	B	B	B
Approach Delay (s)	4.7	4.7	4.7	4.8	11.4	11.4	11.3	11.3	11.4	11.3	11.3
Approach LOS	A	A	A	A	A	B	B	B	B	B	B
Intersection Summary											
HCM 2000 Control Delay	6.0										
HCM 2000 Volume to Capacity ratio	0.28										
Actuated Cycle Length (s)	42.2										
Intersection Capacity Utilization	61.6%										
Analysis Period (min)	15										
c Critical Lane Group											

Traffic Study for the Santa Rosa Memorial Hospital  
 PM Existing

Synchro 8 Report

HCM Signalized Intersection Capacity Analysis  
 3: Doyle Park Drive & Montgomery Drive

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	17	122	60	60	364	9	26	12	25	5	38	37	
Traffic Volume (vph)	17	122	60	60	364	9	26	12	25	5	38	37	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (vphpl)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Lane Util. Factor	1.00	0.99	1.00	1.00	1.00	1.00	0.99	0.99	1.00	1.00	0.98	0.98	
Frbp. ped/bikes	0.99	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.95	1.00	1.00	1.00	1.00	0.95	0.98	1.00	1.00	0.94	0.94	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	0.98	1.00	1.00	1.00	0.97	0.97	
Satd. Flow (prot)	1745	1754	1756	1854	1700	1714	1700	1714	1700	1714	1714	1714	
Flt Permitted	0.62	1.00	0.63	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.97	0.97	
Satd. Flow (perm)	1130	1754	1171	1854	1735	1672	1735	1672	1735	1672	1672	1672	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	18	133	65	65	396	10	28	13	27	5	41	40	
RTOR Reduction (vph)	0	37	0	0	2	0	0	23	0	0	34	0	
Lane Group Flow (vph)	18	161	0	65	404	0	0	45	0	0	52	0	
Confl. Peds. (#/hr)	35	16	16	16	35	25	17	17	17	17	25	25	
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA	
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4	
Prohibited Phases													
Actuated Green, G (s)	6.5	6.5	6.5	6.5	6.5	6.5	2.4	2.4	2.4	2.4	2.4	2.4	
Effective Green, g (s)	6.5	6.5	6.5	6.5	6.5	6.5	2.4	2.4	2.4	2.4	2.4	2.4	
Actuated g/C Ratio	0.42	0.42	0.42	0.42	0.42	0.42	0.16	0.16	0.16	0.16	0.16	0.16	
Clearance Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lane Grp Cap (vph)	480	745	497	787	272	262	272	262	272	262	262	262	
v/s Ratio Prot	0.09			c0.22									
v/s Ratio Perm	0.02			0.06			0.03					c0.03	
v/c Ratio	0.04	0.22	0.13	0.51	0.17	0.20	0.17	0.20	0.17	0.20	0.20	0.20	
Uniform Delay, d1	2.6	2.8	2.7	3.2	5.6	5.6	2.6	2.4	2.6	2.6	2.6	2.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.0	0.1	0.0	0.2	0.1	0.1	0.1	0.1	0.1	0.1	0.1	0.1	
Delay (s)	2.6	2.8	2.7	3.5	5.7	5.8	2.6	2.4	2.6	2.6	2.6	2.6	
Level of Service	A	A	A	A	A	A	A	A	A	A	A	A	
Approach Delay (s)	2.8			3.4			5.7		5.7		5.8		
Approach LOS	A			A			A		A		A		
Intersection Summary													
HCM 2000 Control Delay	3.7											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.43												
Actuated Cycle Length (s)	15.3											Sum of lost time (s)	6.4
Intersection Capacity Utilization	44.7%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
 3: Doyle Park Drive & Montgomery Drive

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	23	301	28	23	238	4	67	16	36	4	17	24	
Traffic Volume (vph)	23	301	28	23	238	4	67	16	36	4	17	24	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (vphpl)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99	1.00	1.00	0.97	0.97	
Frbp. ped/bikes	0.98	1.00	0.99	1.00	1.00	1.00	0.98	0.98	1.00	1.00	0.93	0.93	
Frt	1.00	0.99	1.00	1.00	1.00	1.00	0.96	0.96	1.00	1.00	0.93	0.93	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	0.97	0.97	1.00	1.00	0.97	0.97	
Satd. Flow (prot)	1729	1834	1751	1857	1687	1668	1687	1668	1687	1668	1668	1668	
Flt Permitted	0.78	1.00	0.78	1.00	1.00	1.00	0.80	0.80	1.00	1.00	0.96	0.96	
Satd. Flow (perm)	1427	1834	1446	1857	1384	1612	1384	1612	1427	1384	1384	1384	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	25	327	30	25	259	4	73	17	39	4	18	26	
RTOR Reduction (vph)	0	9	0	0	1	0	0	29	0	0	19	0	
Lane Group Flow (vph)	25	348	0	25	262	0	0	100	0	0	29	0	
Confl. Peds. (#/hr)	47	26	26	47	64	38	38	64	38	38	64	64	
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA	
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4	
Prohibited Phases													
Actuated Green, G (s)	5.1	5.1	5.1	5.1	5.1	5.1	4.1	4.1	4.1	4.1	4.1	4.1	
Effective Green, g (s)	5.1	5.1	5.1	5.1	5.1	5.1	4.1	4.1	4.1	4.1	4.1	4.1	
Actuated g/C Ratio	0.33	0.33	0.33	0.33	0.33	0.33	0.26	0.26	0.26	0.26	0.26	0.26	
Clearance Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lane Grp Cap (vph)	466	599	472	607	363	423	363	423	466	363	363	363	
v/s Ratio Prot	0.02			0.02			c0.07					0.02	
v/s Ratio Perm	0.05	0.58	0.05	0.43	0.28	0.07	0.28	0.07	0.28	0.07	0.07	0.07	
Uniform Delay, d1	3.6	4.4	3.6	4.1	4.6	4.6	3.6	3.6	3.6	3.6	3.6	3.6	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.0	0.9	0.0	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	
Delay (s)	3.6	5.3	3.6	4.3	4.7	4.7	3.6	3.6	3.6	3.6	3.6	3.6	
Level of Service	A	A	A	A	A	A	A	A	A	A	A	A	
Approach Delay (s)	5.2			4.2			4.7		4.7		4.3		
Approach LOS	A			A			A		A		A		
Intersection Summary													
HCM 2000 Control Delay	4.7											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.44												
Actuated Cycle Length (s)	15.6											Sum of lost time (s)	6.4
Intersection Capacity Utilization	40.2%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
 4: Brookwood Avenue & Sonoma Avenue

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	65	350	28	135	288	168	16	309	330	146	197	55
Future Volume (vph)	65	350	28	135	288	168	16	309	330	146	197	55
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.6	3.0	3.6	3.0	3.6	3.0	3.2	3.0	3.6	3.0	3.0
Lane Util. Factor	1.00	0.95	1.00	0.99	1.00	1.00	1.00	0.98	1.00	1.00	1.00	0.98
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	0.99	1.00	0.94	1.00	0.94	1.00	0.92	1.00	1.00	1.00	0.85
Frt	1.00	0.99	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Flt Protected	1766	3484	1764	3311	1770	1679	1770	1679	1770	1863	1544	1544
Flt Permitted	0.33	1.00	0.24	1.00	0.24	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	614	3484	446	3311	1770	1679	1770	1679	1770	1863	1544	1544
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	72	389	31	150	320	187	18	343	367	162	219	61
RTOR Reduction (vph)	0	5	0	0	70	0	0	27	0	0	0	24
Lane Group Flow (vph)	72	415	0	150	437	0	18	683	0	162	219	37
Confl. Peds. (#/hr)	15	15	15	15	15	15	15	15	15	15	15	15
Turn Type	pm+pl	NA	NA	pm+pl	NA	NA	Prot	NA	Prot	NA	pm+ov	NA
Permitted Phases	5	2		1	6		3	8		7	4	5
Prohibited Phases	2			6								4
Actuated Green, G (s)	22.9	16.7		30.3	21.1		2.9	42.1		12.7	51.5	57.7
Effective Green, g (s)	22.9	16.7		30.3	21.1		2.9	42.1		12.7	51.5	57.7
Actuated g/C Ratio	0.24	0.18		0.32	0.22		0.03	0.44		0.13	0.54	0.61
Clearance Time (s)	3.0	3.6		3.0	3.6		3.0	3.2		3.0	3.6	3.0
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lane Grp Cap (vph)	223	613		289	736		54	744		236	1011	938
v/s Ratio Prot	0.02	0.12		0.06	0.13		0.01	0.41		0.09	0.12	0.00
v/s Ratio Perm	0.06	0.11		0.11	0.11		0.11	0.11		0.09	0.12	0.02
v/c Ratio	0.32	0.68		0.52	0.59		0.33	0.92		0.69	0.22	0.04
Uniform Delay, d1	28.6	36.6		24.7	33.1		45.1	24.8		39.2	11.2	7.5
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.3	2.3		0.7	0.9		1.3	16.1		6.5	0.1	0.0
Delay (s)	28.9	38.9		25.4	33.9		46.4	40.9		45.7	11.4	7.5
Level of Service	C	D		C	C		D	D		D	B	A
Approach Delay (s)												23.4
Approach LOS												C

Intersection Summary			
HCM 2000 Control Delay	34.3	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.79		
Actuated Cycle Length (s)	94.9	Sum of lost time (s)	13.2
Intersection Capacity Utilization	82.4%	ICU Level of Service	E
Analysis Period (min)	15		
c Critical Lane Group			

Traffic Study for the Santa Rosa Memorial Hospital  
 AM Existing

HCM Signalized Intersection Capacity Analysis  
 4: Brookwood Avenue & Sonoma Avenue

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	93	383	50	257	372	154	16	264	162	122	361	102
Future Volume (vph)	93	383	50	257	372	154	16	264	162	122	361	102
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.6	3.0	3.6	3.0	3.6	3.0	3.2	3.0	3.6	3.0	3.0
Lane Util. Factor	1.00	0.95	1.00	0.99	1.00	1.00	1.00	0.98	1.00	1.00	1.00	0.98
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	0.98	1.00	0.96	1.00	0.96	1.00	0.94	1.00	1.00	1.00	0.85
Frt	1.00	0.99	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Flt Protected	1767	3455	1765	3359	1770	1725	1770	1725	1770	1863	1550	1550
Flt Permitted	0.32	1.00	0.23	1.00	0.23	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	587	3455	423	3359	1770	1725	1770	1725	1770	1863	1550	1550
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	104	430	56	289	418	173	18	285	182	137	406	115
RTOR Reduction (vph)	0	9	0	0	35	0	0	19	0	0	0	51
Lane Group Flow (vph)	104	477	0	289	556	0	18	448	0	137	406	64
Confl. Peds. (#/hr)	15	15	15	15	15	15	15	15	15	15	15	15
Turn Type	pm+pl	NA	NA	pm+pl	NA	NA	Prot	NA	Prot	NA	pm+ov	NA
Permitted Phases	5	2		1	6		3	8		7	4	5
Prohibited Phases	2			6								4
Actuated Green, G (s)	26.5	17.6		34.5	22.6		2.6	29.9		11.8	38.7	47.6
Effective Green, g (s)	26.5	17.6		34.5	22.6		2.6	29.9		11.8	38.7	47.6
Actuated g/C Ratio	0.31	0.20		0.40	0.26		0.03	0.35		0.14	0.45	0.55
Clearance Time (s)	3.0	3.6		3.0	3.6		3.0	3.2		3.0	3.6	3.0
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lane Grp Cap (vph)	302	707		386	882		53	599		242	838	857
v/s Ratio Prot	0.04	0.14		0.12	0.17		0.01	0.26		0.08	0.22	0.01
v/s Ratio Perm	0.07	0.18		0.18	0.18		0.18	0.18		0.08	0.22	0.03
v/c Ratio	0.34	0.68		0.75	0.63		0.34	0.75		0.57	0.48	0.07
Uniform Delay, d1	22.0	31.6		19.4	28.0		40.9	24.7		34.7	16.6	8.9
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.3	2.0		6.8	1.0		1.4	5.1		1.8	0.4	0.0
Delay (s)	22.3	33.6		26.2	29.0		42.2	29.8		36.5	17.1	9.0
Level of Service	C	C		C	C		D	C		D	B	A
Approach Delay (s)												19.7
Approach LOS												B

Intersection Summary			
HCM 2000 Control Delay	27.2	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.75		
Actuated Cycle Length (s)	86.0	Sum of lost time (s)	13.2
Intersection Capacity Utilization	71.4%	ICU Level of Service	C
Analysis Period (min)	15		
c Critical Lane Group			

Traffic Study for the Santa Rosa Memorial Hospital  
 PM Existing

HCM Unsignalized Intersection Capacity Analysis  
5: Sonoma Avenue & Sotoyome Street

12/14/2016



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	1	1	1	1	1
Traffic Volume (veh/h)	111	656	546	21	0	61
Future Volume (Veh/h)	111	656	546	21	0	61
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Hourly flow rate (vph)	119	705	587	23	0	66
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		TWLT/L	TWLT/L			
Median storage (veh)		2	2			
Upstream signal (ft)		595				
pX platoon unblocked				0.87		
VC, conflicting volume	610			1542	598	
VC1, stage 1 conf vol				598		
VC2, stage 2 conf vol				943		
VCu, unblocked vol	610			1548	598	
IC, single (s)	4.1			6.4	6.2	
IC, 2 stage (s)				5.4		
p0 queue free %	88			100	87	
IF (s)	2.2			3.5	3.3	
CM capacity (veh/h)	969			279	502	
Direction, Lane #	EB 1	EB 2	WB 1	WB 1	SB 1	SB 1
Volumes Total	119	705	610	66		
Volume Left	119	0	0	0		
Volume Right	0	0	23	66		
cSH	969	1700	1700	502		
Volumes to Capacity	0.12	0.41	0.36	0.13		
Queue Length 95th (ft)	10	0	0	11		
Control Delay (s)	9.2	0.0	0.0	13.3		
Lane LOS	A			B		
Approach Delay (s)	1.3		0.0	13.3		
Approach LOS			B			
Intersection Summary						
Average Delay	1.3					
Intersection Capacity Utilization	42.5%					
Analysis Period (min)	15					
	ICU Level of Service					
	A					

Traffic Study for the Santa Rosa Memorial Hospital  
AM Existing

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
5: Sonoma Avenue & Sotoyome Street

12/14/2016



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	1	1	1	1	1	1
Traffic Volume (veh/h)	45	627	608	18	0	97
Future Volume (Veh/h)	45	627	608	18	0	97
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	51	704	683	20	0	109
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		TWLT/L	TWLT/L			
Median storage (veh)		2	2			
Upstream signal (ft)		595				
pX platoon unblocked				0.83		
VC, conflicting volume	703			1499	693	
VC1, stage 1 conf vol				693		
VC2, stage 2 conf vol				806		
VCu, unblocked vol	703			1499	693	
IC, single (s)	4.1			6.4	6.2	
IC, 2 stage (s)				5.4		
p0 queue free %	94			100	75	
IF (s)	2.2			3.5	3.3	
CM capacity (veh/h)	895			320	443	
Direction, Lane #	EB 1	EB 2	WB 1	WB 1	SB 1	SB 1
Volumes Total	51	704	703	109		
Volume Left	51	0	0	0		
Volume Right	0	0	20	109		
cSH	895	1700	1700	443		
Volumes to Capacity	0.06	0.41	0.41	0.25		
Queue Length 95th (ft)	5	0	0	24		
Control Delay (s)	9.3	0.0	0.0	15.7		
Lane LOS	A			C		
Approach Delay (s)	0.6		0.0	15.7		
Approach LOS			C			
Intersection Summary						
Average Delay	1.4					
Intersection Capacity Utilization	45.8%					
Analysis Period (min)	15					
	ICU Level of Service					
	A					

Traffic Study for the Santa Rosa Memorial Hospital  
PM Existing

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
6: Doyle Park Drive & Sonoma Avenue

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (veh/h)	91	404	64	36	497	21	47	2	35	11	1	48
Future Volume (Veh/h)	91	404	64	36	497	21	47	2	35	11	1	48
Sign Control	Free											
Grade	0%											
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Hourly flow rate (vph)	100	444	70	40	546	23	52	2	38	12	1	53
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)	1											
Median type	TWLTL											
Median storage (veh)	2											
Upstream signal (ft)												
pX platoon unblocked												
VC, conflicting volume	569		514			1358	1328	479	1302	1352		558
VC1, stage 1 conf vol			679			679	679		638	638		638
VC2, stage 2 conf vol			664			664	664		664	664		664
VCu, unblocked vol	569		514			1358	1328	479	1302	1352		558
IC, single (s)	4.1		4.1			7.1	6.5	6.2	7.1	6.5		6.2
IC, 2 stage (s)						6.1	5.5		6.1	5.5		5.5
p0 queue free %	2.2		2.2			3.5	4.0	3.3	3.5	4.0		3.3
p0 queue free %	90		96			78	99	94	96	100		90
CM capacity (veh/h)	1003		1052			242	286	587	283	296		530
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 1	SB 1	SB 1				
Volume Total	100	514	40	569	92	66						
Volume Left	100	0	40	0	52	12						
Volume Right	0	70	0	23	38	53						
cSH	1003	1700	1052	1700	415	453						
Volumes to Capacity	0.10	0.30	0.04	0.33	0.22	0.15						
Queue Length 95th (ft)	8	0	3	0	21	13						
Control Delay (s)	9.0	0.0	8.6	0.0	18.8	14.3						
Lane LOS	A	A	A	A	C	B						
Approach Delay (s)	1.5		0.6		18.8	14.3						
Approach LOS			C		B							
Intersection Summary												
Average Delay	2.8											
Intersection Capacity Utilization	52.8%											
Analysis Period (min)	15											
ICU Level of Service	A											

Traffic Study for the Santa Rosa Memorial Hospital  
AM Existing

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
6: Doyle Park Drive & Sonoma Avenue

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (veh/h)	24	612	31	15	390	12	26	3	20	15	5	127
Future Volume (Veh/h)	24	612	31	15	390	12	26	3	20	15	5	127
Sign Control	Free											
Grade	0%											
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	26	665	34	16	424	13	28	3	22	16	5	138
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)	1											
Median type	TWLTL											
Median storage (veh)	2											
Upstream signal (ft)												
pX platoon unblocked												
VC, conflicting volume	437		699			1330	1203	682	1192	1214		430
VC1, stage 1 conf vol			734			734	734		462	462		462
VC2, stage 2 conf vol			664			664	664		730	730		751
VCu, unblocked vol	437		699			1330	1203	682	1192	1214		430
IC, single (s)	4.1		4.1			7.1	6.5	6.2	7.1	6.5		6.2
IC, 2 stage (s)						6.1	5.5		6.1	5.5		5.5
p0 queue free %	2.2		2.2			3.5	4.0	3.3	3.5	4.0		3.3
p0 queue free %	98		98			90	99	95	95	99		78
CM capacity (veh/h)	1123		898			273	355	450	326	346		625
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 1	SB 1	SB 1				
Volume Total	26	699	16	437	53	159						
Volume Left	26	0	16	0	28	16						
Volume Right	0	34	0	13	22	138						
cSH	1123	1700	898	1700	480	559						
Volumes to Capacity	0.02	0.41	0.02	0.26	0.11	0.28						
Queue Length 95th (ft)	2	0	1	0	9	29						
Control Delay (s)	8.3	0.0	9.1	0.0	16.9	14.0						
Lane LOS	A	A	A	A	C	B						
Approach Delay (s)	0.3		0.3		16.9	14.0						
Approach LOS			C		B							
Intersection Summary												
Average Delay	2.5											
Intersection Capacity Utilization	56.4%											
Analysis Period (min)	15											
ICU Level of Service	B											

Traffic Study for the Santa Rosa Memorial Hospital  
PM Existing

Synchro 8 Report

HCM Signalized Intersection Capacity Analysis  
 1: Brookwood Avenue & 3rd Street

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	137	139	17	6	90	184	27	591	8	92	390	160	
Traffic Volume (vph)	137	139	17	6	90	184	27	591	8	92	390	160	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2	
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.98	
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Fllb. ped/bikes	1.00	0.98	1.00	0.90	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	
Frt	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Fill Protected	1770	1827	1770	1637	1770	1637	1770	3530	1770	3330	1770	3330	
Satd. Flow (prot)	1770	1827	1770	1637	1770	1637	1770	3530	1770	3330	1770	3330	
Fill Permitted	1770	1827	1770	1637	1770	1637	1770	3530	1770	3330	1770	3330	
Satd. Flow (perm)	1770	1827	1770	1637	1770	1637	1770	3530	1770	3330	1770	3330	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	137	139	17	6	90	184	27	591	8	92	390	160	
RTOR Reduction (vph)	0	4	0	0	92	0	0	1	0	0	47	0	
Lane Group Flow (vph)	137	152	0	6	182	0	27	598	0	92	503	0	
Confl. Peds. (#/hr)	20	20	20	20	20	20	20	20	20	20	20	20	
Turn Type	Prot	NA	NA	Prot	NA	NA	Prot	NA	NA	Prot	NA	NA	
Permitted Phases	5	2		1	6	6	3	8	8	7	4		
Prohibited Phases													
Actuated Green, G (s)	7.6	22.7	1.1	16.2	2.6	18.1	2.6	18.1	6.8	22.7	6.8	22.7	
Effective Green, g (s)	7.6	22.7	1.1	16.2	2.6	18.1	2.6	18.1	6.8	22.7	6.8	22.7	
Actuated g/C Ratio	0.12	0.37	0.02	0.26	0.04	0.30	0.04	0.30	0.11	0.37	0.11	0.37	
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	219	676	31	432	75	1042	75	1042	196	1233	196	1233	
v/s Ratio Prot	c0.08	0.08	0.00	c0.11	0.02	c0.17	0.02	c0.17	c0.05	0.15	c0.05	0.15	
v/s Ratio Perm													
v/c Ratio	0.63	0.22	0.19	0.42	0.36	0.57	0.36	0.57	0.47	0.41	0.47	0.41	
Uniform Delay, d1	25.5	13.3	29.7	18.7	28.5	18.3	28.5	18.3	25.6	14.3	25.6	14.3	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	5.5	0.2	3.0	0.7	2.9	0.8	2.9	0.8	1.8	0.2	1.8	0.2	
Delay (s)	31.0	13.4	32.7	19.3	31.5	19.1	31.5	19.1	27.3	14.5	27.3	14.5	
Level of Service	C	B	C	B	C	B	C	B	C	B	C	B	
Approach Delay (s)	21.6			19.6			19.6		16.4		16.4		
Approach LOS	C			B			B		B		B		
Intersection Summary													
HCM 2000 Control Delay	18.8											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.52												
Actuated Cycle Length (s)	61.3											Sum of lost time (s)	12.6
Intersection Capacity Utilization	62.1%											ICU Level of Service	B
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
 1: Brookwood Avenue & 3rd Street

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	148	172	20	16	155	189	27	434	14	113	518	79	
Traffic Volume (vph)	148	172	20	16	155	189	27	434	14	113	518	79	
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Ideal Flow (vphpl)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2	
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.99	
Frbp. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Fllb. ped/bikes	1.00	0.98	1.00	0.92	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.98	
Frt	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Fill Protected	1770	1829	1770	1680	1770	1680	1770	3517	1770	3443	1770	3443	
Satd. Flow (prot)	1770	1829	1770	1680	1770	1680	1770	3517	1770	3443	1770	3443	
Fill Permitted	1770	1829	1770	1680	1770	1680	1770	3517	1770	3443	1770	3443	
Satd. Flow (perm)	1770	1829	1770	1680	1770	1680	1770	3517	1770	3443	1770	3443	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	148	172	20	16	155	189	27	434	14	113	518	79	
RTOR Reduction (vph)	0	4	0	0	49	0	0	2	0	0	13	0	
Lane Group Flow (vph)	148	188	0	16	295	0	27	446	0	113	584	0	
Confl. Peds. (#/hr)	20	20	20	20	20	20	20	20	20	20	20	20	
Turn Type	Prot	NA	NA	Prot	NA	NA	Prot	NA	NA	Prot	NA	NA	
Permitted Phases	5	2		1	6	6	3	8	8	7	4		
Prohibited Phases													
Actuated Green, G (s)	7.7	26.2	1.1	19.6	2.6	16.6	2.6	16.6	7.6	26.2	7.6	26.2	
Effective Green, g (s)	7.7	26.2	1.1	19.6	2.6	16.6	2.6	16.6	7.6	26.2	7.6	26.2	
Actuated g/C Ratio	0.12	0.41	0.02	0.31	0.04	0.26	0.04	0.26	0.12	0.41	0.12	0.41	
Clearance Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	212	747	30	513	71	910	71	910	209	1181	209	1181	
v/s Ratio Prot	c0.08	0.10	0.01	c0.18	0.02	0.13	0.02	0.13	c0.06	0.17	c0.06	0.17	
v/s Ratio Perm													
v/c Ratio	0.70	0.25	0.53	0.58	0.38	0.49	0.38	0.49	0.54	0.49	0.54	0.49	
Uniform Delay, d1	27.1	12.5	31.2	18.7	30.0	20.2	30.0	20.2	26.6	16.7	26.6	16.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	9.6	0.2	17.0	1.6	3.4	0.4	3.4	0.4	2.8	0.3	2.8	0.3	
Delay (s)	36.7	12.7	48.2	20.3	33.3	20.6	33.3	20.6	29.4	17.0	29.4	17.0	
Level of Service	D	B	D	C	D	C	D	C	C	B	C	B	
Approach Delay (s)	23.1			21.6			21.3		19.0		19.0		
Approach LOS	C			B			B		B		B		
Intersection Summary													
HCM 2000 Control Delay	20.8											HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.59												
Actuated Cycle Length (s)	64.1											Sum of lost time (s)	12.6
Intersection Capacity Utilization	66.2%											ICU Level of Service	C
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
 2. Sotoyome Street & Montgomery Drive

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Traffic Volume (vph)	22	236	38	88	462	11	23	11	89	4	12
Future Volume (vph)	22	236	38	88	462	11	23	11	89	4	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.96	0.95
Fllb. ped/bikes	0.99	1.00	0.99	1.00	1.00	0.99	1.00	0.94	1.00	1.00	1.00
Frt	1.00	0.98	1.00	1.00	1.00	1.00	1.00	0.85	1.00	0.85	0.94
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.97	1.00	0.97	1.00	0.97	0.99
Satd. Flow (prot)	1748	1814	1747	1854	1688	1513	1643	1643	1688	1513	1643
Flt Permitted	0.44	1.00	0.59	1.00	1.00	0.87	1.00	0.87	1.00	0.98	0.98
Satd. Flow (perm)	808	1814	1086	1854	1517	1513	1619	1619	1513	1513	1619
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	22	236	38	88	462	11	23	11	89	4	12
RTOR Reduction (vph)	0	10	0	0	2	0	0	0	67	0	10
Lane Group Flow (vph)	22	264	0	88	471	0	34	22	0	19	0
Confl. Peds. (#/hr)	24	18	18	24	109	28	28	28	28	28	109
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	2	2	2	2	4	4	4	4	4
Prohibited Phases											
Actuated Green, G (s)	28.8	28.8	28.8	28.8	28.8	11.5	11.5	11.5	11.5	11.5	11.5
Effective Green, g (s)	28.8	28.8	28.8	28.8	28.8	11.5	11.5	11.5	11.5	11.5	11.5
Actuated g/C Ratio	0.62	0.62	0.62	0.62	0.62	0.25	0.25	0.25	0.25	0.25	0.25
Clearance Time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	2.0	2.0	2.0	2.0	2.0	2.0
Lane Grp Cap (vph)	500	1123	672	1148	375	374	400	400	374	374	400
v/s Ratio Prot	0.03	0.15	0.08	0.08	c0.25	c0.02	0.01	0.01	0.01	0.01	0.01
v/s Ratio Perm	0.04	0.23	0.13	0.41	0.13	0.09	0.06	0.05	0.06	0.05	0.05
Uniform Delay, d1	3.5	3.9	3.7	4.5	3.5	13.5	13.4	13.3	13.4	13.3	13.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.0	0.1	0.1	0.2	0.1	0.0	0.0	0.0	0.0	0.0	0.0
Delay (s)	3.5	4.1	3.8	4.8	3.8	13.5	13.4	13.3	13.4	13.3	13.3
Level of Service	A	A	A	A	A	B	B	B	B	B	B
Approach Delay (s)	4.0	4.0	4.0	4.6	4.6	13.4	13.4	13.3	13.4	13.3	13.3
Approach LOS	A	A	A	A	A	B	B	B	B	B	B
Intersection Summary											
HCM 2000 Control Delay	5.8										
HCM 2000 Level of Service	A										
HCM 2000 Volume to Capacity ratio	0.32										
Actuated Cycle Length (s)	46.5										
Sum of lost time (s)	6.2										
Intersection Capacity Utilization	64.7%										
ICU Level of Service	C										
Analysis Period (min)	15										
Critical Lane Group	c										

HCM Signalized Intersection Capacity Analysis  
 2. Sotoyome Street & Montgomery Drive

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Traffic Volume (vph)	31	295	18	68	490	13	49	13	67	31	15
Future Volume (vph)	31	295	18	68	490	13	49	13	67	31	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.96	0.97
Fllb. ped/bikes	0.98	1.00	0.98	1.00	1.00	0.94	1.00	0.99	1.00	0.99	0.99
Frt	1.00	0.99	1.00	1.00	1.00	1.00	1.00	0.85	1.00	0.85	0.96
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.96	1.00	0.96	1.00	0.97	0.97
Satd. Flow (prot)	1740	1841	1736	1853	1691	1526	1685	1685	1526	1526	1685
Flt Permitted	0.42	1.00	0.56	1.00	1.00	0.80	1.00	0.80	1.00	0.86	0.86
Satd. Flow (perm)	768	1841	1023	1853	1412	1526	1498	1498	1526	1526	1498
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	31	295	18	68	490	13	49	13	67	31	15
RTOR Reduction (vph)	0	4	0	0	2	0	0	0	51	0	11
Lane Group Flow (vph)	31	309	0	68	501	0	62	16	0	44	0
Confl. Peds. (#/hr)	34	28	28	34	85	18	18	18	18	18	85
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	2	2	2	2	4	4	4	4	4
Prohibited Phases											
Actuated Green, G (s)	29.5	29.5	29.5	29.5	29.5	11.5	11.5	11.5	11.5	11.5	11.5
Effective Green, g (s)	29.5	29.5	29.5	29.5	29.5	11.5	11.5	11.5	11.5	11.5	11.5
Actuated g/C Ratio	0.62	0.62	0.62	0.62	0.62	0.24	0.24	0.24	0.24	0.24	0.24
Clearance Time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	2.0	2.0	2.0	2.0	2.0	2.0
Lane Grp Cap (vph)	480	1150	639	1158	344	371	364	364	371	364	364
v/s Ratio Prot	0.04	0.17	0.07	0.07	c0.27	c0.04	0.01	0.01	0.01	0.01	0.03
v/s Ratio Perm	0.06	0.27	0.11	0.43	0.11	0.18	0.04	0.04	0.04	0.04	0.12
Uniform Delay, d1	4.0	4.0	3.6	4.5	3.6	14.1	13.6	13.9	13.6	13.9	13.9
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.1	0.1	0.1	0.3	0.1	0.1	0.1	0.1	0.1	0.1	0.1
Delay (s)	3.5	4.1	3.6	4.8	3.6	14.2	13.7	14.0	13.7	14.0	14.0
Level of Service	A	A	A	A	A	B	B	B	B	B	B
Approach Delay (s)	4.1	4.1	4.1	4.7	4.7	13.9	13.9	14.0	13.9	14.0	14.0
Approach LOS	A	A	A	A	A	B	B	B	B	B	B
Intersection Summary											
HCM 2000 Control Delay	6.0										
HCM 2000 Level of Service	A										
HCM 2000 Volume to Capacity ratio	0.36										
Actuated Cycle Length (s)	47.2										
Sum of lost time (s)	6.2										
Intersection Capacity Utilization	66.0%										
ICU Level of Service	C										
Analysis Period (min)	15										
Critical Lane Group	c										

HCM Signalized Intersection Capacity Analysis  
3: Doyle Park Drive & Montgomery Drive

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	17	139	64	65	546	9	43	12	32	5	38	37
Traffic Volume (vph)	17	139	64	65	546	9	43	12	32	5	38	37
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	0.99	1.00	1.00	1.00	1.00	0.99	0.99	1.00	1.00	0.98	0.98
Frb. ped/bikes	0.99	1.00	0.99	1.00	1.00	1.00	0.99	0.99	1.00	1.00	0.98	0.98
Frt	1.00	0.95	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	0.94	0.94
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	0.98	0.98	1.00	1.00	0.94	0.94
Satd. Flow (prot)	1745	1756	1752	1857	1857	1695	1695	1695	1711	1711	1667	1667
Flt Permitted	0.41	1.00	0.63	1.00	1.00	0.81	0.81	0.81	0.97	0.97	0.97	0.97
Satd. Flow (perm)	750	1756	1163	1857	1403							
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	17	139	64	65	546	9	43	12	32	5	38	37
RTOR Reduction (vph)	0	32	0	0	1	0	0	26	0	0	30	0
Lane Group Flow (vph)	17	171	0	65	554	0	0	61	0	0	50	0
Confl. Peds. (#/hr)	35	16	16	16	16	35	25	17	17	17	17	25
Turn Type	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4
Prohibited Phases												
Actuated Green, G (s)	9.8	9.8	9.8	9.8	9.8	3.7	3.7	3.7	3.7	3.7	3.7	3.7
Effective Green, g (s)	9.8	9.8	9.8	9.8	9.8	3.7	3.7	3.7	3.7	3.7	3.7	3.7
Actuated g/C Ratio	0.49	0.49	0.49	0.49	0.49	0.19	0.19	0.19	0.19	0.19	0.19	0.19
Clearance Time (s)	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	3.1
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lane Grp Cap (vph)	369	864	572	914	260							
v/s Ratio Prot	0.02	0.10	0.06	0.06	c0.30							
v/s Ratio Perm	0.05	0.20	0.11	0.61	0.23							
Uniform Delay, d1	2.6	2.8	2.7	3.7	6.9							
Progression Factor	1.00	1.00	1.00	1.00	1.00							
Incremental Delay, d2	0.0	0.0	0.0	0.8	0.2							
Delay (s)	2.6	2.9	2.7	4.4	7.1							
Level of Service	A	A	A	A	A							
Approach Delay (s)	2.9	4.3	4.3	7.1	6.9							
Approach LOS	A	A	A	A	A							
Intersection Summary												
HCM 2000 Control Delay	4.4 HCM 2000 Level of Service A											
HCM 2000 Volume to Capacity ratio	0.50											
Actuated Cycle Length (s)	19.9 Sum of lost time (s) 6.4											
Intersection Capacity Utilization	55.0% ICU Level of Service B											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis  
3: Doyle Park Drive & Montgomery Drive

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	23	324	29	31	416	4	88	16	39	4	17	24
Traffic Volume (vph)	23	324	29	31	416	4	88	16	39	4	17	24
Future Volume (vph)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Ideal Flow (vphpl)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1
Total Lost time (s)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99	1.00	1.00	0.97	0.97
Frb. ped/bikes	0.98	1.00	0.99	1.00	1.00	1.00	0.98	0.98	1.00	1.00	0.93	0.93
Frt	1.00	0.99	1.00	1.00	1.00	1.00	0.96	0.96	1.00	1.00	0.93	0.93
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	0.97	0.97	1.00	1.00	0.93	0.93
Satd. Flow (prot)	1729	1834	1747	1859	1679	1679	1663	1663	1663	1663	1602	1602
Flt Permitted	0.52	1.00	0.55	1.00	1.00	0.79	0.79	0.79	0.96	0.96	0.96	0.96
Satd. Flow (perm)	941	1834	1011	1859	1360							
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	23	324	29	31	416	4	88	16	39	4	17	24
RTOR Reduction (vph)	0	7	0	0	1	0	0	30	0	0	19	0
Lane Group Flow (vph)	23	346	0	31	419	0	113	0	26	0	26	0
Confl. Peds. (#/hr)	47	26	26	47	64	38	38	38	64	38	38	64
Turn Type	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4
Prohibited Phases												
Actuated Green, G (s)	8.6	8.6	8.6	8.6	8.6	4.2	4.2	4.2	4.2	4.2	4.2	4.2
Effective Green, g (s)	8.6	8.6	8.6	8.6	8.6	4.2	4.2	4.2	4.2	4.2	4.2	4.2
Actuated g/C Ratio	0.45	0.45	0.45	0.45	0.45	0.22	0.22	0.22	0.22	0.22	0.22	0.22
Clearance Time (s)	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	3.1
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lane Grp Cap (vph)	421	821	452	832	297							
v/s Ratio Prot	0.02	0.19	0.03	0.03	c0.23							
v/s Ratio Perm	0.05	0.42	0.07	0.50	0.38							
Uniform Delay, d1	3.0	3.6	3.0	3.8	6.4							
Progression Factor	1.00	1.00	1.00	1.00	1.00							
Incremental Delay, d2	0.0	0.1	0.0	0.2	0.3							
Delay (s)	3.0	3.7	3.0	4.0	6.7							
Level of Service	A	A	A	A	A							
Approach Delay (s)	3.7	3.9	3.9	6.7	6.0							
Approach LOS	A	A	A	A	A							
Intersection Summary												
HCM 2000 Control Delay	4.3 HCM 2000 Level of Service A											
HCM 2000 Volume to Capacity ratio	0.46											
Actuated Cycle Length (s)	19.2 Sum of lost time (s) 6.4											
Intersection Capacity Utilization	48.1% ICU Level of Service A											
Analysis Period (min)	15											
c Critical Lane Group												

HCM Signalized Intersection Capacity Analysis  
4: Brookwood Avenue & Sonoma Avenue

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	78	362	28	135	314	194	17	348	330	276	369	114
Future Volume (vph)	78	362	28	135	314	194	17	348	330	276	369	114
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.6	3.6	3.0	3.6	3.6	3.0	3.2	3.0	3.6	3.6	3.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	1.00	0.98
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.99	1.00	1.00	0.94	1.00	0.93	1.00	0.93	1.00	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1767	3486	1762	3303	1770	1690	1770	1690	1770	1863	1545	1545
Flt Permitted	0.25	1.00	0.27	1.00	0.27	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	471	3486	499	3303	1770	1690	1770	1690	1770	1863	1545	1545
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	78	362	28	135	314	194	17	348	330	276	369	114
RTOR Reduction (vph)	0	5	0	0	83	0	0	25	0	0	0	40
Lane Group Flow (vph)	78	385	0	135	425	0	17	653	0	276	369	74
Confl. Peds. (#/hr)	15	15	15	15	15	15	15	15	15	15	15	15
Turn Type	pm+pl	NA	NA	pm+pl	NA	NA	Prot	NA	Prot	NA	NA	pm+ov
Permitted Phases	5	2		1	6		3	8		7		4
Prohibited Phases	2			6								4
Actuated Green, G (s)	23.9	15.8	27.7	17.7	17.7	3.0	41.9	3.0	41.9	15.1	53.6	61.7
Effective Green, g (s)	23.9	15.8	27.7	17.7	17.7	3.0	41.9	3.0	41.9	15.1	53.6	61.7
Actuated q/C Ratio	0.25	0.17	0.29	0.19	0.19	0.03	0.44	0.03	0.44	0.16	0.56	0.65
Clearance Time (s)	3.0	3.6	3.0	3.6	3.6	3.0	3.2	3.0	3.2	3.0	3.6	3.0
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	3.0	2.0	3.0	2.0	3.0	2.0
Lane Grp Cap (vph)	227	576	276	611	611	55	740	55	740	279	1044	997
v/s Ratio Prot	0.03	0.11	0.05	0.13	0.13	0.01	0.39	0.01	0.39	0.16	0.20	0.01
v/s Ratio Perm	0.06		0.09									0.04
v/c Ratio	0.34	0.67	0.49	0.70	0.70	0.31	0.88	0.31	0.88	0.99	0.35	0.07
Uniform Delay, d1	28.5	37.4	26.6	36.4	36.4	45.3	24.6	40.2	24.6	11.5	11.5	6.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.3	2.3	0.5	2.8	2.8	1.2	12.1	1.2	12.1	50.3	0.2	0.0
Delay (s)	288	39.7	27.1	39.2	39.2	46.5	36.7	46.5	36.7	90.4	11.7	6.3
Level of Service	C	D	C	D	D	D	D	D	D	F	B	A
Approach Delay (s)	37.9		36.7			36.9					39.5	
Approach LOS	D		D			D				D	D	

Intersection Summary	
HCM 2000 Control Delay	37.8
HCM 2000 Volume to Capacity ratio	0.84
Actuated Cycle Length (s)	95.6
Intersection Capacity Utilization	92.6%
Analysis Period (min)	15
c Critical Lane Group	

Traffic Study for the Santa Rosa Memorial Hospital  
AM Future

Synchro 8 Report

HCM Signalized Intersection Capacity Analysis  
4: Brookwood Avenue & Sonoma Avenue

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (vph)	104	398	50	257	394	166	18	296	175	199	567	175
Future Volume (vph)	104	398	50	257	394	166	18	296	175	199	567	175
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.6	3.6	3.0	3.6	3.6	3.0	3.2	3.0	3.6	3.6	3.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	1.00	0.98
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.98	1.00	1.00	0.96	1.00	0.94	1.00	0.94	1.00	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (prot)	1767	3457	1764	3356	1770	1731	1770	1731	1770	1863	1549	1549
Flt Permitted	0.30	1.00	0.24	1.00	0.24	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	560	3457	444	3356	1770	1731	1770	1731	1770	1863	1549	1549
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	104	398	50	257	394	166	18	296	175	199	567	175
RTOR Reduction (vph)	0	8	0	0	37	0	0	18	0	0	0	73
Lane Group Flow (vph)	104	440	0	257	523	0	18	453	0	199	567	102
Confl. Peds. (#/hr)	15	15	15	15	15	15	15	15	15	15	15	15
Turn Type	pm+pl	NA	NA	pm+pl	NA	NA	Prot	NA	Prot	NA	NA	pm+ov
Permitted Phases	5	2		1	6		3	8		7		4
Prohibited Phases	2			6								4
Actuated Green, G (s)	26.0	17.0	33.3	21.3	21.3	2.7	30.9	2.7	30.9	15.3	43.1	52.1
Effective Green, g (s)	26.0	17.0	33.3	21.3	21.3	2.7	30.9	2.7	30.9	15.3	43.1	52.1
Actuated q/C Ratio	0.29	0.19	0.37	0.24	0.24	0.03	0.35	0.03	0.35	0.17	0.48	0.58
Clearance Time (s)	3.0	3.6	3.0	3.6	3.6	3.0	3.2	3.0	3.2	3.0	3.6	3.0
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	3.0	2.0	3.0	2.0	3.0	2.0
Lane Grp Cap (vph)	284	658	362	800	800	53	988	53	988	303	899	903
v/s Ratio Prot	0.04	0.13	0.11	0.16	0.16	0.01	0.26	0.01	0.26	0.11	0.30	0.01
v/s Ratio Perm	0.07		0.16									0.05
v/c Ratio	0.37	0.67	0.71	0.65	0.65	0.34	0.76	0.34	0.76	0.66	0.63	0.11
Uniform Delay, d1	24.0	33.5	21.4	30.7	30.7	42.4	25.9	42.4	25.9	34.5	17.2	8.3
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	0.3	2.0	5.2	1.5	1.5	1.4	5.5	1.4	5.5	3.9	1.5	0.0
Delay (s)	24.3	35.5	26.5	32.1	32.1	43.8	31.4	43.8	31.4	38.4	18.6	8.3
Level of Service	C	D	C	C	C	D	D	D	D	D	B	A
Approach Delay (s)	33.4		30.4			31.8					20.9	
Approach LOS	C		C			C				C	C	

Intersection Summary	
HCM 2000 Control Delay	28.0
HCM 2000 Volume to Capacity ratio	0.75
Actuated Cycle Length (s)	89.3
Intersection Capacity Utilization	78.7%
Analysis Period (min)	15
c Critical Lane Group	

Traffic Study for the Santa Rosa Memorial Hospital  
PM Future

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
5: Sonoma Avenue & Sotoyome Street

09/07/2017



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	121	660	647	25	0	86
Traffic Volume (veh/h)	121	660	647	25	0	86
Future Volume (Veh/h)	121	660	647	25	0	86
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	121	660	647	25	0	86
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLT/L	TWLT/L	TWLT/L			
Median storage (veh)	2	2	2			
Upstream signal (ft)	595					
pX platoon unblocked					0.88	
VC conflicting volume	672				1562	660
VC1 stage 1 conf vol					660	
VC2 stage 2 conf vol					902	
VCu unblocked vol	672				1570	660
IC single (s)	4.1				6.4	6.2
IC 2 stage (s)					5.4	
IF (s)	2.2				3.5	3.3
p0 queue free %	87				100	81
CM capacity (veh/h)	919				284	463
Direction_Lane #	EB 1	EB 2	WB 1	WB 1	SB 1	SB 1
Volume Total	121	660	672	86		
Volume Left	121	0	0	0		
Volume Right	0	0	25	86		
cSH	919	1700	1700	463		
Volume to Capacity	0.13	0.39	0.40	0.19		
Queue Length 95th (ft)	11	0	0	17		
Control Delay (s)	9.5	0.0	0.0	14.5		
Lane LOS	A			B		
Approach Delay (s)	1.5		0.0	14.5		
Approach LOS			B			
Intersection Summary						
Average Delay	1.6					
Intersection Capacity Utilization	48.9%					
ICU Level of Service	A					
Analysis Period (min)	15					

Traffic Study for the Santa Rosa Memorial Hospital  
AM Future

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
5: Sonoma Avenue & Sotoyome Street

09/07/2017



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	54	631	709	23	0	122
Traffic Volume (veh/h)	54	631	709	23	0	122
Future Volume (Veh/h)	54	631	709	23	0	122
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	54	631	709	23	0	122
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	TWLT/L	TWLT/L	TWLT/L			
Median storage (veh)	2	2	2			
Upstream signal (ft)	595					
pX platoon unblocked					0.85	
VC conflicting volume	732				1460	720
VC1 stage 1 conf vol					720	
VC2 stage 2 conf vol					739	
VCu unblocked vol	732				1453	720
IC single (s)	4.1				6.4	6.2
IC 2 stage (s)					5.4	
IF (s)	2.2				3.5	3.3
p0 queue free %	94				100	71
CM capacity (veh/h)	873				334	428
Direction_Lane #	EB 1	EB 2	WB 1	WB 1	SB 1	SB 1
Volume Total	54	631	732	122		
Volume Left	54	0	0	0		
Volume Right	0	0	23	122		
cSH	873	1700	1700	428		
Volume to Capacity	0.06	0.37	0.43	0.29		
Queue Length 95th (ft)	5	0	0	29		
Control Delay (s)	9.4	0.0	0.0	16.7		
Lane LOS	A			C		
Approach Delay (s)	0.7		0.0	16.7		
Approach LOS			C			
Intersection Summary						
Average Delay	1.7					
Intersection Capacity Utilization	52.9%					
ICU Level of Service	A					
Analysis Period (min)	15					

Traffic Study for the Santa Rosa Memorial Hospital  
PM Future

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
6: Doyle Park Drive & Sonoma Avenue

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	104	404	64	49	580	32	57	3	36	11	1	57
Future Volume (Veh/h)	104	404	64	49	580	32	57	3	36	11	1	57
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	104	404	64	49	580	32	57	3	36	11	1	57
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	TWLT/L			TWLT/L								
Median storage (veh)												
Upstream signal (ft)												
VC, conflicting volume	612	468		1380	1354	436	1326	1370	1370	596		
VC1, stage 1 conf vol				644	644		694	694				
VC2, stage 2 conf vol				736	710		632	676				
VCu, unblocked vol	612	468		1380	1354	436	1326	1370	1370	596		
IC, single (s)	4.1	4.1		7.1	6.5	6.2	7.1	6.5	6.2	7.1	6.5	6.2
IC, 2 stage (s)				6.1	5.5		6.1	5.5		6.1	5.5	
p0 queue free %	89	96		74	99	94	96	100	89			
ICM capacity (veh/h)	967	1094		219	270	620	278	290	504			
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 1	SB 1					
Volumes Total	104	468	49	612	96	69						
Volume Left	104	0	49	0	57	11						
Volume Right	0	64	0	32	36	57						
cSH	967	1700	1094	1700	354	442						
Volumes to Capacity	0.11	0.28	0.04	0.36	0.27	0.16						
Queue Length 95th (ft)	9	0	4	0	27	14						
Control Delay (s)	9.2	0.0	8.4	0.0	21.2	14.7						
Lane LOS	A	A	A	C	C	B						
Approach Delay (s)	1.7		0.6		21.2	14.7						
Approach LOS			C		C	B						
Intersection Summary												
Average Delay	3.2											
Intersection Capacity Utilization	59.1%											
ICU Level of Service	B											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis  
6: Doyle Park Drive & Sonoma Avenue

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	41	694	36	15	390	14	26	5	24	21	7	152
Future Volume (Veh/h)	41	694	36	15	390	14	26	5	24	21	7	152
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	41	694	36	15	390	14	26	5	24	21	7	152
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	TWLT/L			TWLT/L								
Median storage (veh)												
Upstream signal (ft)												
VC, conflicting volume	404	730		1370	1228	712	1218	1239	397			
VC1, stage 1 conf vol				794	794		427	427				
VC2, stage 2 conf vol				576	434		790	812				
VCu, unblocked vol	404	730		1370	1228	712	1218	1239	397			
IC, single (s)	4.1	4.1		7.1	6.5	6.2	7.1	6.5	6.2	7.1	6.5	6.2
IC, 2 stage (s)				6.1	5.5		6.1	5.5		6.1	5.5	
p0 queue free %	96	98		90	99	94	93	98	77			
ICM capacity (veh/h)	1155	874		256	338	432	302	329	652			
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 1	SB 1					
Volumes Total	41	730	15	404	55	180						
Volume Left	41	0	15	0	26	21						
Volume Right	0	36	0	14	24	152						
cSH	1155	1700	874	1700	478	556						
Volumes to Capacity	0.04	0.43	0.02	0.24	0.12	0.32						
Queue Length 95th (ft)	3	0	1	0	10	35						
Control Delay (s)	8.2	0.0	9.2	0.0	17.3	14.5						
Lane LOS	A	A	A	C	C	B						
Approach Delay (s)	0.4		0.3		17.3	14.5						
Approach LOS			C		C	B						
Intersection Summary												
Average Delay	2.8											
Intersection Capacity Utilization	63.0%											
ICU Level of Service	B											
Analysis Period (min)	15											

HCM Signalized Intersection Capacity Analysis  
 1: Brookwood Avenue & 3rd Street

1/4/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB	
Traffic Volume (vph)	82	157	17	6	86	106	19	254	8	89	343	58	
Future Volume (vph)	82	157	17	6	86	106	19	254	8	89	343	58	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.95	1.00	0.95	
Fpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Fpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.99	1.00	0.92	1.00	0.92	1.00	1.00	1.00	1.00	0.98	1.00	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	1770	1832	1770	1677	1770	1677	1770	3517	1770	3437	1770	3437	
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (perm)	1770	1832	1770	1677	1770	1677	1770	3517	1770	3437	1770	3437	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	89	171	18	7	93	115	21	276	9	97	373	63	
RTOR Reduction (vph)	0	4	0	0	54	0	0	3	0	0	14	0	
Lane Group Flow (vph)	89	185	0	7	154	0	21	282	0	97	422	0	
Confl. Peds. (#/hr)	20	20	20	20	20	20	20	20	20	20	20	20	
Turn Type	Prot	NA	NA	Prot	NA	NA	Prot	NA	Prot	NA	NA	NA	
Permitted Phases	5	2		1	6	6	3	8	7	4			
Prohibited Phases													
Actuated Green, G (s)	5.1	19.0		1.0	14.9		1.0	13.6	6.9	19.9			
Effective Green, g (s)	5.1	19.0		1.0	14.9		1.0	13.6	6.9	19.9			
Actuated q/C Ratio	0.10	0.36		0.02	0.28		0.02	0.26	0.13	0.37			
Clearance Time (s)	3.0	3.0		3.0	3.0		3.0	3.6	3.0	3.2			
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0			
Lane Grp Cap (vph)	170	655		33	470		33	900	230	1288			
v/s Ratio Prot	c0.05	c0.10		0.00	0.09		0.01	0.08	c0.05	c0.12			
v/s Ratio Perm	0.52	0.28		0.21	0.33		0.64	0.31	0.42	0.33			
Uniform Delay, d1	22.8	12.2		25.7	15.1		25.9	16.0	21.3	11.8			
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00			
Incremental Delay, d2	2.9	0.2		3.2	0.4		33.9	0.2	1.2	0.1			
Delay (s)	25.7	12.4		28.9	15.5		59.7	16.2	22.5	12.0			
Level of Service	C	B		C	B		E	B	C	B			
Approach Delay (s)	16.7			16.0			19.2		13.9				
Approach LOS	B			B			B		B				
Intersection Summary													
HCM 2000 Control Delay	16.0											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.39												
Actuated Cycle Length (s)	53.1											Sum of lost time (s)	12.6
Intersection Capacity Utilization	54.9%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
 1: Brookwood Avenue & 3rd Street

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB	
Traffic Volume (vph)	133	149	19	14	165	170	24	333	10	68	320	54	
Future Volume (vph)	133	149	19	14	165	170	24	333	10	68	320	54	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.2	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.95	1.00	0.95	
Fpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Fpb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frt	1.00	0.98	1.00	0.92	1.00	0.92	1.00	1.00	1.00	1.00	0.98	1.00	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (prot)	1770	1825	1770	1695	1770	1695	1770	3519	1770	3435	1770	3435	
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	
Satd. Flow (perm)	1770	1825	1770	1695	1770	1695	1770	3519	1770	3435	1770	3435	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	145	162	21	15	179	185	26	362	11	74	348	59	
RTOR Reduction (vph)	0	4	0	0	39	0	0	2	0	0	16	0	
Lane Group Flow (vph)	145	179	0	15	325	0	26	371	0	74	391	0	
Confl. Peds. (#/hr)	20	20	20	20	20	20	20	20	20	20	20	20	
Turn Type	Prot	NA	NA	Prot	NA	NA	Prot	NA	Prot	NA	NA	NA	
Permitted Phases	5	2		1	6	6	3	8	7	4			
Prohibited Phases													
Actuated Green, G (s)	7.9	26.8		1.1	20.0		2.5	14.2	4.7	16.8			
Effective Green, g (s)	7.9	26.8		1.1	20.0		2.5	14.2	4.7	16.8			
Actuated q/C Ratio	0.13	0.45		0.02	0.34		0.04	0.24	0.08	0.28			
Clearance Time (s)	3.0	3.0		3.0	3.0		3.0	3.6	3.0	3.2			
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0			
Lane Grp Cap (vph)	235	823		32	570		74	841	140	971			
v/s Ratio Prot	c0.08	0.10		0.01	c0.19		0.01	0.11	c0.04	c0.11			
v/s Ratio Perm	0.62	0.22		0.47	0.57		0.35	0.44	0.53	0.40			
Uniform Delay, d1	24.3	9.9		28.9	16.2		27.7	19.2	26.3	17.2			
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00			
Incremental Delay, d2	4.8	0.1		10.5	1.3		2.9	0.4	3.6	0.3			
Delay (s)	29.1	10.1		39.3	17.5		30.5	19.6	29.9	17.5			
Level of Service	C	B		D	B		C	B	C	B			
Approach Delay (s)	18.5			18.3			20.3		19.4				
Approach LOS	B			B			C		B				
Intersection Summary													
HCM 2000 Control Delay	19.2											HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio	0.53												
Actuated Cycle Length (s)	59.4											Sum of lost time (s)	12.6
Intersection Capacity Utilization	61.0%											ICU Level of Service	B
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
 2. Sotoyome Street & Montgomery Drive

11/4/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	2	2	2	2	2	2	2	2	2	2	2	2	
Traffic Volume (vph)	22	228	62	65	296	11	29	11	76	4	12	13	
Future Volume (vph)	22	228	62	65	296	11	29	11	76	4	12	13	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.95	0.95	
Fllb. ped/bikes	0.99	1.00	0.99	1.00	0.99	1.00	0.94	1.00	0.85	1.00	1.00	1.00	
Frt	1.00	0.97	1.00	0.99	1.00	1.00	0.85	1.00	0.85	1.00	0.94	0.94	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.97	1.00	0.99	1.00	0.94	0.94	
Satd. Flow (prot)	1745	1789	1750	1850	1850	1689	1517	1651	1651	1651	1651	1651	
Flt Permitted	0.54	1.00	0.56	1.00	0.56	1.00	0.86	1.00	0.86	1.00	0.98	0.98	
Satd. Flow (perm)	994	1789	1026	1850	1850	1498	1517	1628	1628	1628	1628	1628	
Peak-hour factor, PHF	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	
Adj. Flow (vph)	24	245	67	70	318	12	31	12	82	4	13	14	
RTOR Reduction (vph)	0	20	0	0	3	0	0	0	0	0	0	0	
Lane Group Flow (vph)	24	292	0	70	327	0	43	23	0	21	0	109	
Confl. Peds. (#/hr)	24	18	18	24	109	24	109	28	28	28	28	109	
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA	
Permitted Phases	2	2	2	2	2	4	4	4	4	4	4	4	
Actuated Green, G (s)	23.7	23.7	23.7	23.7	23.7	11.8	11.8	11.8	11.8	11.8	11.8	11.8	
Effective Green, g (s)	23.7	23.7	23.7	23.7	23.7	11.8	11.8	11.8	11.8	11.8	11.8	11.8	
Actuated g/C Ratio	0.57	0.57	0.57	0.57	0.57	0.28	0.28	0.28	0.28	0.28	0.28	0.28	
Clearance Time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	564	1016	583	1051	1051	423	429	460	460	460	460	460	
v/s Ratio Prot	0.02	0.16	0.07	0.07	0.18	c0.18							
v/s Ratio Perm	0.04	0.29	0.12	0.31	0.31	0.10	0.05	0.05	0.05	0.05	0.05	0.05	
Uniform Delay, d1	4.0	4.6	4.2	4.7	4.7	11.0	10.9	10.9	10.9	10.9	10.9	10.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.0	0.2	0.1	0.2	0.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Delay (s)	4.0	4.8	4.3	4.9	4.9	11.1	10.9	10.9	10.9	10.9	10.9	10.9	
Level of Service	A	A	A	A	A	B	B	B	B	B	B	B	
Approach Delay (s)	4.7	4.8	4.8	4.8	4.8	11.0	11.0	10.9	10.9	10.9	10.9	10.9	
Approach LOS	A	A	A	A	A	B	B	B	B	B	B	B	
Intersection Summary													
HCM 2000 Control Delay	5.8											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.24												
Actuated Cycle Length (s)	41.7											Sum of lost time (s)	6.2
Intersection Capacity Utilization	64.4%											ICU Level of Service	C
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
 2. Sotoyome Street & Montgomery Drive

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	2	2	2	2	2	2	2	2	2	2	2	2	
Traffic Volume (vph)	31	284	30	44	329	13	73	13	58	15	9	31	
Future Volume (vph)	31	284	30	44	329	13	73	13	58	15	9	31	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.97	1.00	0.95	0.95	
Fllb. ped/bikes	0.98	1.00	0.98	1.00	0.98	1.00	0.95	1.00	0.85	1.00	1.00	1.00	
Frt	1.00	0.99	1.00	0.99	1.00	0.99	1.00	0.96	1.00	0.85	0.92	0.92	
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.96	1.00	0.96	1.00	0.99	0.99	
Satd. Flow (prot)	1737	1828	1742	1849	1849	1692	1530	1609	1609	1609	1609	1609	
Flt Permitted	0.50	1.00	0.53	1.00	0.53	1.00	0.77	1.00	0.77	1.00	0.94	0.94	
Satd. Flow (perm)	920	1828	970	1849	1849	1357	1530	1529	1529	1529	1529	1529	
Peak-hour factor, PHF	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	
Adj. Flow (vph)	34	312	33	48	362	14	80	14	64	16	10	34	
RTOR Reduction (vph)	0	8	0	0	3	0	0	0	0	0	0	0	
Lane Group Flow (vph)	34	337	0	48	373	0	94	18	0	36	0	85	
Confl. Peds. (#/hr)	34	28	28	34	85	34	85	18	18	18	18	85	
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA	NA	
Permitted Phases	2	2	2	2	2	4	4	4	4	4	4	4	
Actuated Green, G (s)	23.3	23.3	23.3	23.3	23.3	11.7	11.7	11.7	11.7	11.7	11.7	11.7	
Effective Green, g (s)	23.3	23.3	23.3	23.3	23.3	11.7	11.7	11.7	11.7	11.7	11.7	11.7	
Actuated g/C Ratio	0.57	0.57	0.57	0.57	0.57	0.28	0.28	0.28	0.28	0.28	0.28	0.28	
Clearance Time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	520	1033	548	1045	1045	385	434	434	434	434	434	434	
v/s Ratio Prot	0.04	0.18	0.05	0.05	0.20	c0.20							
v/s Ratio Perm	0.07	0.33	0.09	0.36	0.36	0.24	0.04	0.04	0.04	0.04	0.04	0.04	
Uniform Delay, d1	4.0	4.8	4.1	4.9	4.9	11.3	10.7	10.7	10.7	10.7	10.7	10.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.1	0.2	0.1	0.2	0.2	0.1	0.1	0.1	0.1	0.1	0.1	0.1	
Delay (s)	4.1	5.0	4.2	5.1	5.1	11.5	10.7	10.8	10.8	10.8	10.8	10.8	
Level of Service	A	A	A	A	A	B	B	B	B	B	B	B	
Approach Delay (s)	4.9	4.9	4.9	5.0	5.0	11.2	11.2	10.8	10.8	10.8	10.8	10.8	
Approach LOS	A	A	A	A	A	B	B	B	B	B	B	B	
Intersection Summary													
HCM 2000 Control Delay	6.2											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.32												
Actuated Cycle Length (s)	41.2											Sum of lost time (s)	6.2
Intersection Capacity Utilization	62.0%											ICU Level of Service	B
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
3: Doyle Park Drive & Montgomery Drive

1/4/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB	
Traffic Volume (vph)	19	134	60	60	393	9	26	12	25	5	38	41	
Future Volume (vph)	19	134	60	60	393	9	26	12	25	5	38	41	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00	0.99	0.99	1.00	1.00	1.00	0.98	
Fllb. ped/bikes	0.99	1.00	0.99	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.93	
Frt	1.00	0.95	1.00	1.00	1.00	1.00	0.95	0.98	1.00	1.00	1.00	0.93	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	0.98	1.00	1.00	1.00	1.00	0.93	
Satd. Flow (prot)	1746	1760	1746	1756	1854	1700	1705	1705	1705	1705	1705	1705	
Flt Permitted	0.61	1.00	0.63	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.97	
Satd. Flow (perm)	1114	1760	1114	1157	1854	1735	1735	1735	1735	1735	1735	1666	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	21	146	65	65	427	10	28	13	27	5	41	45	
RTOR Reduction (vph)	0	35	0	0	2	0	0	23	0	0	38	0	
Lane Group Flow (vph)	21	176	0	65	435	0	0	45	0	0	53	0	
Confl. Peds. (#/hr)	35	16	16	16	35	25	17	17	17	17	17	25	
Turn Type	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4	
Prohibited Phases													
Actuated Green, G (s)	6.6	6.6	6.6	6.6	6.6	6.6	2.4	2.4	2.4	2.4	2.4	2.4	
Effective Green, g (s)	6.6	6.6	6.6	6.6	6.6	6.6	2.4	2.4	2.4	2.4	2.4	2.4	
Actuated g/C Ratio	0.43	0.43	0.43	0.43	0.43	0.43	0.16	0.16	0.16	0.16	0.16	0.16	
Clearance Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lane Grp Cap (vph)	477	754	477	495	794	270	270	270	270	259	259	259	
v/s Ratio Prot	0.02	0.10	0.10	0.06	0.06	0.06	0.03	0.03	0.03	c0.03	c0.03	c0.03	
v/s Ratio Perm	0.04	0.23	0.23	0.13	0.55	0.17	0.17	0.17	0.20	0.20	0.20	0.20	
Uniform Delay, d1	2.6	2.8	2.8	2.7	3.3	3.3	5.6	5.6	5.7	5.7	5.7	5.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.0	0.1	0.1	0.0	0.4	0.1	0.1	0.1	0.1	0.1	0.1	0.1	
Delay (s)	2.6	2.9	2.9	2.7	3.7	3.7	5.7	5.7	5.8	5.8	5.8	5.8	
Level of Service	A	A	A	A	A	A	A	A	A	A	A	A	
Approach Delay (s)	2.8	2.8	2.8	3.6	3.6	3.6	5.7	5.7	5.8	5.8	5.8	5.8	
Approach LOS	A	A	A	A	A	A	A	A	A	A	A	A	
Intersection Summary													
HCM 2000 Control Delay	3.8											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.46												
Actuated Cycle Length (s)	15.4											Sum of lost time (s)	6.4
Intersection Capacity Utilization	46.2%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

Traffic Study for the Santa Rosa Memorial Hospital  
AM Existing plus Project

Synchro 8 Report

HCM Signalized Intersection Capacity Analysis  
3: Doyle Park Drive & Montgomery Drive

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB	SB	
Traffic Volume (vph)	28	336	28	23	253	4	67	16	36	4	17	26	
Future Volume (vph)	28	336	28	23	253	4	67	16	36	4	17	26	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99	1.00	1.00	1.00	0.97	
Fllb. ped/bikes	0.98	1.00	0.99	1.00	1.00	1.00	0.98	0.98	1.00	1.00	1.00	0.92	
Frt	1.00	0.99	1.00	1.00	1.00	1.00	0.96	0.96	1.00	1.00	1.00	0.92	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	0.97	0.97	1.00	1.00	1.00	0.92	
Satd. Flow (prot)	1729	1836	1729	1752	1857	1686	1686	1686	1686	1686	1686	1662	
Flt Permitted	0.74	1.00	0.74	1.00	1.00	1.00	0.80	0.80	1.00	1.00	1.00	0.96	
Satd. Flow (perm)	1348	1836	1348	1366	1857	1381	1381	1381	1381	1381	1381	1607	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	30	365	30	25	275	4	73	17	39	4	18	28	
RTOR Reduction (vph)	0	7	0	0	0	1	0	0	29	0	0	21	
Lane Group Flow (vph)	30	388	0	25	278	0	0	100	0	0	29	0	
Confl. Peds. (#/hr)	47	26	26	26	47	64	64	64	38	38	38	64	
Turn Type	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4	
Prohibited Phases													
Actuated Green, G (s)	5.4	5.4	5.4	5.4	5.4	5.4	4.0	4.0	4.0	4.0	4.0	4.0	
Effective Green, g (s)	5.4	5.4	5.4	5.4	5.4	5.4	4.0	4.0	4.0	4.0	4.0	4.0	
Actuated g/C Ratio	0.34	0.34	0.34	0.34	0.34	0.34	0.25	0.25	0.25	0.25	0.25	0.25	
Clearance Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lane Grp Cap (vph)	460	627	460	466	634	349	349	349	349	349	349	349	
v/s Ratio Prot	0.02	0.21	0.21	0.02	0.02	0.02	c0.07	c0.07	c0.07	c0.07	c0.07	c0.07	
v/s Ratio Perm	0.07	0.62	0.62	0.05	0.44	0.29	0.29	0.29	0.29	0.29	0.29	0.29	
Uniform Delay, d1	3.5	4.3	4.3	3.5	4.0	4.0	4.8	4.8	4.8	4.8	4.8	4.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.0	1.3	1.3	0.0	0.2	0.2	0.2	0.2	0.2	0.2	0.2	0.2	
Delay (s)	3.5	5.6	5.6	3.5	4.2	4.2	4.9	4.9	4.9	4.9	4.9	4.9	
Level of Service	A	A	A	A	A	A	A	A	A	A	A	A	
Approach Delay (s)	5.5	5.5	5.5	4.1	4.1	4.1	4.9	4.9	4.9	4.9	4.9	4.9	
Approach LOS	A	A	A	A	A	A	A	A	A	A	A	A	
Intersection Summary													
HCM 2000 Control Delay	4.9											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.48												
Actuated Cycle Length (s)	15.8											Sum of lost time (s)	6.4
Intersection Capacity Utilization	44.3%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

Traffic Study for the Santa Rosa Memorial Hospital  
PM Existing plus Project

Synchro 8 Report

HCM Signalized Intersection Capacity Analysis  
 4: Brookwood Avenue & Sonoma Avenue

11/4/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	65	393	28	146	306	168	16	309	357	146	197	55
Future Volume (vph)	65	393	28	146	306	168	16	309	357	146	197	55
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.6	3.0	3.0	3.6	3.0	3.2	3.0	3.6	3.0	3.6	3.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	1.00	0.98
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	0.99	1.00	1.00	0.95	1.00	1.00	0.92	1.00	1.00	1.00	0.85
Frt	1.00	0.99	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	0.85
Satd. Flow (prot)	1766	3490	1765	3319	1770	1672	1770	1672	1770	1863	1544	1544
Flt Permitted	0.33	1.00	0.21	1.00	0.21	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	607	3490	385	3319	1770	1672	1770	1672	1770	1863	1544	1544
Peak-hour factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Adj. Flow (vph)	72	437	31	162	340	187	18	343	397	162	219	61
RTOR Reduction (vph)	0	5	0	0	61	0	0	30	0	0	0	24
Lane Group Flow (vph)	72	463	0	162	466	0	18	710	0	162	219	37
Confl. Peds. (#/hr)	15	15	15	15	15	15	15	15	15	15	15	15
Turn Type	pm+pl	NA	NA	pm+pl	NA	NA	Prot	NA	Prot	NA	pm+ov	NA
Permitted Phases	5	2		1	6		3	8		7	4	5
Prohibited Phases	2			6							4	4
Actuated Green, G (s)	24.0	17.8		31.8	22.6		2.9	42.1		12.8	51.6	57.8
Effective Green, g (s)	24.0	17.8		31.8	22.6		2.9	42.1		12.8	51.6	57.8
Actuated g/C Ratio	0.25	0.18		0.33	0.23		0.03	0.44		0.13	0.53	0.60
Clearance Time (s)	3.0	3.6		3.0	3.6		3.0	3.2		3.0	3.6	3.0
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lane Grp Cap (vph)	225	643		284	777		53	729		234	996	924
v/s Ratio Prot	0.02	c0.13		c0.06	0.14		0.01	c0.42		c0.09	0.12	0.00
v/s Ratio Perm	0.06			0.12							0.02	0.02
v/c Ratio	0.32	0.72		0.57	0.60		0.34	0.97		0.69	0.22	0.04
Uniform Delay, d1	28.5	37.0		24.8	32.9		45.9	26.7		40.0	11.8	7.9
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.3	3.4		1.7	0.8		1.4	26.8		7.0	0.1	0.0
Delay (s)	28.8	40.4		26.5	33.8		47.3	53.5		46.9	11.9	8.0
Level of Service	C	D		C	C		D	D		D	B	A
Approach Delay (s)		38.8			32.0			53.3			24.2	
Approach LOS		D			C			D			C	

Intersection Summary	
HCM 2000 Control Delay	38.8
HCM 2000 Volume to Capacity ratio	0.83
Actuated Cycle Length (s)	96.5
Intersection Capacity Utilization	84.4%
Analysis Period (min)	15
c Critical Lane Group	

Traffic Study for the Santa Rosa Memorial Hospital  
 AM Existing plus Project

Synchro 8 Report

HCM Signalized Intersection Capacity Analysis  
 4: Brookwood Avenue & Sonoma Avenue

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	←	←	←	←	←	←	←	←	←	←	←	←
Traffic Volume (vph)	93	406	50	290	423	154	16	264	176	122	361	102
Future Volume (vph)	93	406	50	290	423	154	16	264	176	122	361	102
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.6	3.0	3.0	3.6	3.0	3.2	3.0	3.6	3.0	3.6	3.0
Lane Util. Factor	1.00	0.95	1.00	1.00	0.99	1.00	1.00	0.98	1.00	1.00	1.00	0.98
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	0.98	1.00	1.00	0.96	1.00	1.00	0.94	1.00	1.00	1.00	0.85
Frt	1.00	0.98	1.00	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	0.85
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	1.00	1.00	1.00	0.85
Satd. Flow (prot)	1767	3459	1766	3374	1770	1718	1770	1718	1770	1863	1549	1549
Flt Permitted	0.29	1.00	0.21	1.00	0.21	1.00	0.95	1.00	0.95	1.00	1.00	1.00
Satd. Flow (perm)	541	3459	387	3374	1770	1718	1770	1718	1770	1863	1549	1549
Peak-hour factor, PHF	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89	0.89
Adj. Flow (vph)	104	456	56	326	475	173	18	285	198	137	406	115
RTOR Reduction (vph)	0	8	0	0	29	0	0	21	0	0	0	52
Lane Group Flow (vph)	104	504	0	326	619	0	18	462	0	137	406	63
Confl. Peds. (#/hr)	15	15	15	15	15	15	15	15	15	15	15	15
Turn Type	pm+pl	NA	NA	pm+pl	NA	NA	Prot	NA	Prot	NA	pm+ov	NA
Permitted Phases	5	2		1	6		3	8		7	4	5
Prohibited Phases	2			6							4	4
Actuated Green, G (s)	27.6	18.7		36.9	25.0		2.7	31.4		11.9	40.2	49.1
Effective Green, g (s)	27.6	18.7		36.9	25.0		2.7	31.4		11.9	40.2	49.1
Actuated g/C Ratio	0.31	0.21		0.41	0.28		0.03	0.35		0.13	0.45	0.55
Clearance Time (s)	3.0	3.6		3.0	3.6		3.0	3.2		3.0	3.6	3.0
Vehicle Extension (s)	2.0	2.0		2.0	2.0		2.0	2.0		2.0	2.0	2.0
Lane Grp Cap (vph)	287	718		391	937		53	599		234	832	845
v/s Ratio Prot	0.04	0.15		c0.14	0.18		0.01	c0.27		c0.08	0.22	0.01
v/s Ratio Perm	0.08			c0.20							0.03	0.03
v/c Ratio	0.36	0.70		0.83	0.66		0.34	0.77		0.59	0.49	0.07
Uniform Delay, d1	23.2	33.1		20.5	28.7		42.8	26.1		36.7	17.6	9.7
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	0.3	2.6		13.6	1.4		1.4	6.1		2.4	0.5	0.0
Delay (s)	23.5	35.6		34.1	30.1		44.2	32.2		39.1	18.1	9.7
Level of Service	C	D		C	C		D	C		D	B	A
Approach Delay (s)		33.6			31.4			32.6			21.0	
Approach LOS		C			C			C			C	

Intersection Summary	
HCM 2000 Control Delay	29.6
HCM 2000 Volume to Capacity ratio	0.80
Actuated Cycle Length (s)	90.0
Intersection Capacity Utilization	74.3%
Analysis Period (min)	15
c Critical Lane Group	

Traffic Study for the Santa Rosa Memorial Hospital  
 PM Existing plus Project

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
5: Sonoma Avenue & Sotoyome Street

1/4/2017



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	W	W	T	T	W	W
Traffic Volume (veh/h)	182	656	546	84	26	90
Future Volume (Veh/h)	182	656	546	84	26	90
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.93	0.93	0.83	0.93	0.93	0.93
Hourly flow rate (vph)	196	705	587	90	28	97
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		TWLT	LT	TWLT		
Median storage (veh)		2	2			
Upstream signal (ft)		595				
pX platoon unblocked					0.86	
VC conflicting volume	677				1729	632
VC1 stage 1 conf vol					632	
VC2 stage 2 conf vol					1097	
VCu unblocked vol	677				1768	632
IC single (s)	4.1				6.4	6.2
IC 2 stage (s)	2.2				5.4	3.3
p0 queue free %	79				87	80
CM capacity (veh/h)	915				212	480
Direction_Lane #	EB 1	EB 2	WB 1	WB 1	SB 1	SB 1
Volumes Total	196	705	677	125		
Volume Left	196	0	0	28		
Volume Right	0	0	90	97		
cSH	915	1700	1700	374		
Volumes to Capacity	0.21	0.41	0.40	0.33		
Queue Length 95th (ft)	20	0	0	36		
Control Delay (s)	10.0	0.0	0.0	19.4		
Lane LOS	B			C		
Approach Delay (s)	2.2		0.0	19.4		
Approach LOS			C			
Intersection Summary						
Average Delay	2.6					
Intersection Capacity Utilization	60.9%					
Analysis Period (min)	15					
	ICU Level of Service					
	B					

Traffic Study for the Santa Rosa Memorial Hospital  
AM Existing plus Project

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
5: Sonoma Avenue & Sotoyome Street

12/14/2016



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	W	W	T	T	W	W
Traffic Volume (veh/h)	82	627	608	51	75	181
Future Volume (Veh/h)	82	627	608	51	75	181
Sign Control	Free	Free	Free	Free	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	0.89	0.89	0.89	0.89	0.89	0.89
Hourly flow rate (vph)	92	704	683	57	84	203
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		TWLT	LT	TWLT		
Median storage (veh)		2	2			
Upstream signal (ft)		595				
pX platoon unblocked					0.83	
VC conflicting volume	740				1600	712
VC1 stage 1 conf vol					712	
VC2 stage 2 conf vol					888	
VCu unblocked vol	740				1620	712
IC single (s)	4.1				6.4	6.2
IC 2 stage (s)	2.2				5.4	3.3
p0 queue free %	89				70	53
CM capacity (veh/h)	867				283	433
Direction_Lane #	EB 1	EB 2	WB 1	WB 1	SB 1	SB 1
Volumes Total	92	704	740	287		
Volume Left	92	0	0	84		
Volume Right	0	0	57	203		
cSH	867	1700	1700	375		
Volumes to Capacity	0.11	0.41	0.44	0.77		
Queue Length 95th (ft)	9	0	0	156		
Control Delay (s)	9.6	0.0	0.0	40.0		
Lane LOS	A			E		
Approach Delay (s)	1.1		0.0	40.0		
Approach LOS			E			
Intersection Summary						
Average Delay	6.8					
Intersection Capacity Utilization	64.9%					
Analysis Period (min)	15					
	ICU Level of Service					
	C					

Traffic Study for the Santa Rosa Memorial Hospital  
PM Existing plus Project

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
6: Doyle Park Drive & Sonoma Avenue

11/4/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (veh/h)	91	428	66	36	556	21	51	2	35	11	1	48
Future Volume (Veh/h)	91	428	66	36	556	21	51	2	35	11	1	48
Sign Control	Free											
Grade	0%											
Peak Hour Factor	0.91											
Hourly flow rate (vph)	100	470	73	40	611	23	56	2	38	12	1	53
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)	1											
Median type	TWLTL											
Median storage (veh)	2											
Upstream signal (ft)												
pX platoon unblocked												
VC, conflicting volume	634			543			1451	1420	506	1392	1446	622
VC1, stage 1 conf vol				706			706	706		702	702	
VC2, stage 2 conf vol				744			744	714		690	743	
VCU, unblocked vol	634			543			1451	1420	506	1392	1446	622
IC, single (s)	4.1	0.91	4.1	4.1	0.91	4.1	7.1	6.5	6.2	7.1	6.5	6.2
IC, 2 stage (s)	2.2	2.2	2.2	2.2	2.2	2.2	6.1	5.5	6.1	6.1	5.5	5.5
p0 queue free %	89	96	96	74	99	93	74	99	93	95	100	89
CM capacity (veh/h)	949			1026			216	264	566	264	278	486
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1						
Volumes Total	100	543	40	634	96	66						
Volume Left	100	0	40	0	56	12						
Volume Right	0	73	0	23	38	53						
cSH	949	1700	1026	1700	360	418						
Volumes to Capacity	0.11	0.32	0.04	0.37	0.27	0.16						
Queue Length 95th (ft)	9	0	3	0	26	14						
Control Delay (s)	9.2	0.0	8.7	0.0	21.3	15.2						
Lane LOS	A	A	A	A	C	C						
Approach Delay (s)	1.4		0.5		21.3	15.2						
Approach LOS	C						C					
Intersection Summary												
Average Delay	2.9											
Intersection Capacity Utilization	55.9%											
ICU Level of Service	B											
Analysis Period (min)	15											

Traffic Study for the Santa Rosa Memorial Hospital  
AM Existing plus Project

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
6: Doyle Park Drive & Sonoma Avenue

12/14/2016

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (veh/h)	24	682	36	15	421	12	28	3	20	15	5	127
Future Volume (Veh/h)	24	682	36	15	421	12	28	3	20	15	5	127
Sign Control	Free											
Grade	0%											
Peak Hour Factor	0.92											
Hourly flow rate (vph)	26	741	39	16	458	13	30	3	22	16	5	138
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)	1											
Median type	TWLTL											
Median storage (veh)	2											
Upstream signal (ft)												
pX platoon unblocked												
VC, conflicting volume	471			780			1443	1316	760	1302	1328	464
VC1, stage 1 conf vol				812			812	812		496	496	
VC2, stage 2 conf vol				630			630	503		806	832	
VCU, unblocked vol	471			780			1443	1316	760	1302	1328	464
IC, single (s)	4.1	0.92	4.1	4.1	0.92	4.1	7.1	6.5	6.2	7.1	6.5	6.2
IC, 2 stage (s)	2.2	2.2	2.2	2.2	2.2	2.2	6.1	5.5	6.1	6.1	5.5	5.5
p0 queue free %	98	98	98	88	99	95	88	99	95	95	98	77
CM capacity (veh/h)	1091			837			249	327	406	293	317	598
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1						
Volumes Total	26	780	16	471	55	159						
Volume Left	26	0	16	0	30	16						
Volume Right	0	39	0	13	22	138						
cSH	1091	1700	837	1700	427	528						
Volumes to Capacity	0.02	0.46	0.02	0.28	0.13	0.30						
Queue Length 95th (ft)	2	0	1	0	11	31						
Control Delay (s)	8.4	0.0	9.4	0.0	18.4	14.7						
Lane LOS	A	A	A	A	C	B						
Approach Delay (s)	0.3		0.3		18.4	14.7						
Approach LOS	C						C					
Intersection Summary												
Average Delay	2.5											
Intersection Capacity Utilization	60.3%											
ICU Level of Service	B											
Analysis Period (min)	15											

Traffic Study for the Santa Rosa Memorial Hospital  
PM Existing plus Project

Synchro 8 Report

HCM Signalized Intersection Capacity Analysis  
 1: Brookwood Avenue & 3rd Street

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	137	157	17	6	97	189	27	591	8	104	390
Future Volume (vph)	137	157	17	6	97	189	27	591	8	104	390
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.6	3.0	3.6	3.0	3.2	3.2
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.95	1.00
Frbp. ped/bikes	1.00	1.00	1.00	1.00	0.98	1.00	1.00	1.00	1.00	0.98	1.00
Fllb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.99	1.00	1.00	0.90	1.00	1.00	1.00	1.00	0.96	1.00
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	1831	1770	1640	1770	3530	1770	3330	1770	3330	3330
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	1831	1770	1640	1770	3530	1770	3330	1770	3330	3330
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	137	157	17	6	97	189	27	591	8	104	390
RTOR Reduction (vph)	0	4	0	0	87	0	0	1	0	0	46
Lane Group Flow (vph)	137	170	0	6	199	0	27	598	0	104	504
Confl. Peds. (#/hr)	20	20	20	20	20	20	20	20	20	20	20
Turn Type	Prot	NA	NA	Prot	NA	NA	Prot	NA	Prot	NA	NA
Protected Phases	5	2		1	6	3	8		7	4	
Permitted Phases				6							
Actuated Green, G (s)	7.6	23.1		1.1	16.6	2.6	18.3		7.2	23.3	
Effective Green, g (s)	7.6	23.1		1.1	16.6	2.6	18.3		7.2	23.3	
Actuated g/C Ratio	0.12	0.37		0.02	0.27	0.04	0.29		0.12	0.37	
Clearance Time (s)	3.0	3.0		3.0	3.0	3.0	3.6		3.0	3.2	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	215	678		31	436	73	1036		204	1245	
v/s Ratio Prot	c0.08	0.09		0.00	c0.12	0.02	c0.17		c0.06	0.15	
v/s Ratio Perm											
v/c Ratio	0.64	0.25		0.19	0.46	0.37	0.58		0.51	0.40	
Uniform Delay, d1	26.0	13.6		30.2	19.1	29.1	18.7		25.9	14.4	
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	6.1	0.2		3.0	0.8	3.1	0.8		2.0	0.2	
Delay (s)	32.1	13.8		33.2	19.8	32.2	19.5		27.9	14.6	
Level of Service	C	B		C	B	C	B		C	B	
Approach Delay (s)	21.9		C	20.1		20.0		C	16.7		B
Approach LOS	C		C	C		C		C	B		B
Intersection Summary											
HCM 2000 Control Delay	19.2										
HCM 2000 Volume to Capacity ratio	0.54										
Actuated Cycle Length (s)	62.3										
Intersection Capacity Utilization	62.8%										
Analysis Period (min)	15										
c Critical Lane Group											

HCM Signalized Intersection Capacity Analysis  
 1: Brookwood Avenue & 3rd Street

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	1	1	1	1	1	1	1	1	1	1	1
Traffic Volume (vph)	148	181	20	16	176	203	27	434	14	119	518
Future Volume (vph)	148	181	20	16	176	203	27	434	14	119	518
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0	3.0	3.0	3.0	3.6	3.0	3.6	3.0	3.2	3.2
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	0.95	1.00
Frbp. ped/bikes	1.00	1.00	1.00	1.00	0.98	1.00	1.00	1.00	1.00	0.99	1.00
Fllb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.99	1.00	1.00	0.92	1.00	1.00	1.00	1.00	0.98	1.00
Flt Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	1830	1770	1684	1770	3517	1770	3441	1770	3441	3441
Flt Permitted	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	1830	1770	1684	1770	3517	1770	3441	1770	3441	3441
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	148	181	20	16	176	203	27	434	14	119	518
RTOR Reduction (vph)	0	3	0	0	46	0	0	2	0	0	14
Lane Group Flow (vph)	148	198	0	16	333	0	27	446	0	119	583
Confl. Peds. (#/hr)	20	20	20	20	20	20	20	20	20	20	20
Turn Type	Prot	NA	NA	Prot	NA	NA	Prot	NA	Prot	NA	NA
Protected Phases	5	2		1	6	3	8		7	4	
Permitted Phases				6							
Actuated Green, G (s)	11.3	31.2		1.2	21.1	2.7	16.1		8.2	22.0	
Effective Green, g (s)	11.3	31.2		1.2	21.1	2.7	16.1		8.2	22.0	
Actuated g/C Ratio	0.16	0.45		0.02	0.30	0.04	0.23		0.12	0.32	
Clearance Time (s)	3.0	3.0		3.0	3.0	3.0	3.6		3.0	3.2	
Vehicle Extension (s)	3.0	3.0		3.0	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	288	823		30	512	68	817		209	1092	
v/s Ratio Prot	c0.08	0.11		0.01	c0.20	0.02	0.13		c0.07	c0.17	
v/s Ratio Perm											
v/c Ratio	0.51	0.24		0.53	0.65	0.40	0.55		0.57	0.53	
Uniform Delay, d1	26.5	11.7		33.8	20.9	32.5	23.4		28.9	19.4	
Progression Factor	1.00	1.00		1.00	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	1.5	0.2		17.0	3.0	3.8	0.7		3.5	0.5	
Delay (s)	28.0	11.9		50.8	23.9	36.3	24.1		32.4	19.9	
Level of Service	C	B		D	C	D	C		C	B	
Approach Delay (s)	18.7		B	24.9		24.8		C	22.0		C
Approach LOS	B		B	C		C		C	B		C
Intersection Summary											
HCM 2000 Control Delay	22.7										
HCM 2000 Volume to Capacity ratio	0.61										
Actuated Cycle Length (s)	69.3										
Intersection Capacity Utilization	68.1%										
Analysis Period (min)	15										
c Critical Lane Group											

HCM Signalized Intersection Capacity Analysis  
 2. Sotoyome Street & Montgomery Drive

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Traffic Volume (vph)	22	242	62	91	464	11	33	11	90	4	12
Future Volume (vph)	22	242	62	91	464	11	33	11	90	4	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.96	0.95
Fllb. ped/bikes	0.99	1.00	0.99	1.00	1.00	0.99	1.00	0.93	1.00	1.00	1.00
Frt	1.00	0.97	1.00	1.00	1.00	1.00	1.00	0.85	1.00	0.85	0.94
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.96	1.00	0.96	1.00	0.96	0.99
Satd. Flow (prot)	1748	1792	1748	1854	1854	1669	1513	1643	1643	1643	1643
Flt Permitted	0.44	1.00	0.57	1.00	1.00	0.84	1.00	0.84	1.00	0.98	0.98
Satd. Flow (perm)	805	1792	1043	1854	1854	1460	1513	1617	1617	1617	1617
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	22	242	62	91	464	11	33	11	90	4	12
RTOR Reduction (vph)	0	17	0	0	2	0	0	0	68	0	10
Lane Group Flow (vph)	22	287	0	91	473	0	44	22	0	19	0
Confl. Peds. (#/hr)	24	18	18	24	109	24	109	28	28	28	109
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	2	2	2	4	4	4	4	4	4
Prohibited Phases											
Actuated Green, G (s)	28.8	28.8	28.8	28.8	28.8	11.5	11.5	11.5	11.5	11.5	11.5
Effective Green, g (s)	28.8	28.8	28.8	28.8	28.8	11.5	11.5	11.5	11.5	11.5	11.5
Actuated g/C Ratio	0.62	0.62	0.62	0.62	0.62	0.25	0.25	0.25	0.25	0.25	0.25
Clearance Time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	2.0	2.0	2.0	2.0	2.0	2.0
Lane Grp Cap (vph)	498	1109	645	1148	1148	361	374	399	399	399	399
v/s Ratio Prot	0.03	0.16	0.09	0.09	0.26	c0.26					
v/s Ratio Perm	0.04	0.26	0.14	0.41	0.41	c0.03	0.01	0.01	0.01	0.01	0.01
w/C Ratio	3.5	4.0	3.7	4.5	4.5	13.6	13.4	13.3	13.3	13.3	13.3
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Progression Factor	0.0	0.1	0.1	0.2	0.2	0.1	0.0	0.0	0.0	0.0	0.0
Incremental Delay, d2	3.5	4.1	3.8	4.8	4.8	13.6	13.4	13.3	13.3	13.3	13.3
Delay (s)	A	A	A	A	A	B	B	B	B	B	B
Level of Service	A	A	A	A	A	B	B	B	B	B	B
Approach Delay (s)	4.1	4.6	4.6	4.6	4.6	13.5	13.5	13.3	13.3	13.3	13.3
Approach LOS	A	A	A	A	A	B	B	B	B	B	B
Intersection Summary											
HCM 2000 Control Delay	5.8										
HCM 2000 Level of Service	A										
HCM 2000 Volume to Capacity ratio	0.33										
Actuated Cycle Length (s)	46.5										
Sum of lost time (s)	6.2										
Intersection Capacity Utilization	65.1%										
ICU Level of Service	C										
Analysis Period (min)	15										
c Critical Lane Group											

HCM Signalized Intersection Capacity Analysis  
 2. Sotoyome Street & Montgomery Drive

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBR
Lane Configurations	EB	EB	EB	WB	WB	WB	NB	NB	NB	SB	SB
Traffic Volume (vph)	31	298	30	70	497	13	77	13	71	31	15
Future Volume (vph)	31	298	30	70	497	13	77	13	71	31	15
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.2	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.96	1.00	0.96	0.97
Fllb. ped/bikes	0.98	1.00	0.98	1.00	1.00	0.94	1.00	0.94	1.00	0.99	0.99
Frt	1.00	0.99	1.00	1.00	1.00	1.00	1.00	0.85	1.00	0.85	0.96
Flt Protected	0.95	1.00	0.95	1.00	1.00	0.96	1.00	0.96	1.00	0.97	0.97
Satd. Flow (prot)	1741	1829	1737	1853	1853	1679	1527	1687	1687	1687	1687
Flt Permitted	0.41	1.00	0.55	1.00	1.00	0.76	1.00	0.76	1.00	0.86	0.86
Satd. Flow (perm)	756	1829	1002	1853	1853	1339	1527	1484	1484	1484	1484
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Adj. Flow (vph)	31	298	30	70	497	13	77	13	71	31	15
RTOR Reduction (vph)	0	7	0	0	2	0	0	0	53	0	11
Lane Group Flow (vph)	31	321	0	70	508	0	90	18	0	44	0
Confl. Peds. (#/hr)	34	28	28	34	85	34	85	18	18	18	85
Turn Type	Perm	NA	NA	Perm	NA	Perm	NA	Perm	NA	Perm	NA
Permitted Phases	2	2	2	2	2	4	4	4	4	4	4
Prohibited Phases											
Actuated Green, G (s)	28.5	28.5	28.5	28.5	28.5	11.5	11.5	11.5	11.5	11.5	11.5
Effective Green, g (s)	28.5	28.5	28.5	28.5	28.5	11.5	11.5	11.5	11.5	11.5	11.5
Actuated g/C Ratio	0.62	0.62	0.62	0.62	0.62	0.25	0.25	0.25	0.25	0.25	0.25
Clearance Time (s)	3.2	3.2	3.2	3.2	3.2	3.0	3.0	3.0	3.0	3.0	3.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	2.0	2.0	2.0	2.0	2.0	2.0
Lane Grp Cap (vph)	466	1128	618	1143	1143	333	380	369	369	369	369
v/s Ratio Prot	0.04	0.18	0.07	0.07	0.27	c0.27					
v/s Ratio Perm	0.07	0.29	0.11	0.44	0.44	c0.07	0.01	0.01	0.01	0.01	0.03
w/C Ratio	4.1	4.1	3.6	4.7	4.7	14.0	13.2	13.4	13.4	13.4	13.4
Uniform Delay, d1	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Progression Factor	0.1	0.1	0.1	0.3	0.3	0.2	0.0	0.0	0.0	0.1	0.1
Incremental Delay, d2	3.6	4.3	3.7	5.0	5.0	14.1	13.2	13.5	13.5	13.5	13.5
Delay (s)	A	A	A	A	A	B	B	B	B	B	B
Level of Service	A	A	A	A	A	B	B	B	B	B	B
Approach Delay (s)	4.2	4.8	4.8	4.8	4.8	13.7	13.7	13.5	13.5	13.5	13.5
Approach LOS	A	A	A	A	A	B	B	B	B	B	B
Intersection Summary											
HCM 2000 Control Delay	6.3										
HCM 2000 Level of Service	A										
HCM 2000 Volume to Capacity ratio	0.39										
Actuated Cycle Length (s)	46.2										
Sum of lost time (s)	6.2										
Intersection Capacity Utilization	66.4%										
ICU Level of Service	C										
Analysis Period (min)	15										
c Critical Lane Group											

HCM Signalized Intersection Capacity Analysis  
3: Doyle Park Drive & Montgomery Drive

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	
Traffic Volume (vph)	19	151	64	65	575	9	43	12	32	5	38	41	
Future Volume (vph)	19	151	64	65	575	9	43	12	32	5	38	41	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	1.00	0.99	1.00	1.00	1.00	1.00	0.99	0.99	1.00	1.00	0.98	1.00	
Flpb. ped/bikes	0.98	1.00	0.99	1.00	1.00	1.00	0.99	0.99	1.00	1.00	0.98	1.00	
Frt	1.00	0.96	1.00	1.00	1.00	1.00	0.95	0.95	1.00	0.93	0.93	1.00	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	0.98	0.98	1.00	1.00	0.93	1.00	
Satd. Flow (prot)	1743	1761	1749	1857	1692	1701	1692	1692	1701	1701	1692	1701	
Flt Permitted	0.39	1.00	0.62	1.00	1.00	0.80	0.80	0.80	0.97	0.97	0.97	0.97	
Satd. Flow (perm)	716	1761	1148	1857	1388	1660	1388	1388	1660	1660	1660	1660	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	19	151	64	65	575	9	43	12	32	5	38	41	
RTOR Reduction (vph)	0	25	0	0	1	0	0	27	0	0	34	0	
Lane Group Flow (vph)	19	190	0	65	583	0	0	60	0	0	50	0	
Confl. Peds. (#/hr)	35	16	16	16	25	35	25	17	17	17	17	25	
Turn Type	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4	
Prohibited Phases													
Actuated Green, G (s)	13.1	13.1	13.1	13.1	13.1	13.1	3.8	3.8	3.8	3.8	3.8	3.8	
Effective Green, g (s)	13.1	13.1	13.1	13.1	13.1	13.1	3.8	3.8	3.8	3.8	3.8	3.8	
Actuated g/C Ratio	0.56	0.56	0.56	0.56	0.56	0.56	0.16	0.16	0.16	0.16	0.16	0.16	
Clearance Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lane Grp Cap (vph)	402	990	0.11	645	1044	226	270	270	270	270	270	270	
v/s Ratio Prot	0.03	0.11	0.11	0.06	0.31	c0.31	0.03	0.03	0.03	0.03	0.03	0.03	
v/s Ratio Perm	0.05	0.19	0.19	0.10	0.56	0.27	0.04	0.04	0.04	0.04	0.04	0.04	
Uniform Delay, d1	2.3	2.5	2.5	2.4	3.3	8.5	8.4	8.4	8.4	8.4	8.4	8.4	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.0	0.0	0.0	0.0	0.4	0.2	0.1	0.1	0.1	0.1	0.1	0.1	
Delay (s)	2.3	2.5	2.5	2.4	3.6	8.8	8.5	8.5	8.5	8.5	8.5	8.5	
Level of Service	A	A	A	A	A	A	A	A	A	A	A	A	
Approach Delay (s)	2.5	2.5	2.5	3.5	3.5	8.8	8.5	8.5	8.5	8.5	8.5	8.5	
Approach LOS	A	A	A	A	A	A	A	A	A	A	A	A	
Intersection Summary													
HCM 2000 Control Delay	4.1											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.49												
Actuated Cycle Length (s)	23.3											Sum of lost time (s)	6.4
Intersection Capacity Utilization	56.5%											ICU Level of Service	B
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
3: Doyle Park Drive & Montgomery Drive

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	
Traffic Volume (vph)	28	359	29	31	431	4	88	16	39	4	17	26	
Future Volume (vph)	28	359	29	31	431	4	88	16	39	4	17	26	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	0.99	0.99	1.00	1.00	0.97	1.00	
Flpb. ped/bikes	0.98	1.00	0.99	1.00	1.00	1.00	0.98	0.98	1.00	1.00	0.93	1.00	
Frt	1.00	0.99	1.00	1.00	1.00	1.00	0.96	0.96	1.00	0.93	0.93	1.00	
Flt Protected	0.95	1.00	0.95	1.00	1.00	1.00	0.97	0.97	1.00	1.00	0.93	1.00	
Satd. Flow (prot)	1729	1836	1748	1859	1678	1656	1656	1656	1678	1678	1656	1656	
Flt Permitted	0.51	1.00	0.53	1.00	1.00	0.78	0.78	0.78	0.96	0.96	0.93	1.00	
Satd. Flow (perm)	920	1836	979	1859	1357	1597	1357	1357	1597	1597	1597	1597	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	28	359	29	31	431	4	88	16	39	4	17	26	
RTOR Reduction (vph)	0	6	0	0	1	0	0	31	0	0	20	0	
Lane Group Flow (vph)	28	382	0	31	434	0	112	0	0	0	27	0	
Confl. Peds. (#/hr)	47	26	26	26	47	64	38	38	47	38	47	64	
Turn Type	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	Perm	NA	NA	
Permitted Phases	2	2	2	6	6	6	8	8	8	4	4	4	
Prohibited Phases													
Actuated Green, G (s)	9.0	9.0	9.0	9.0	9.0	9.0	4.2	4.2	4.2	4.2	4.2	4.2	
Effective Green, g (s)	9.0	9.0	9.0	9.0	9.0	9.0	4.2	4.2	4.2	4.2	4.2	4.2	
Actuated g/C Ratio	0.46	0.46	0.46	0.46	0.46	0.46	0.21	0.21	0.21	0.21	0.21	0.21	
Clearance Time (s)	3.3	3.3	3.3	3.3	3.3	3.3	3.1	3.1	3.1	3.1	3.1	3.1	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
Lane Grp Cap (vph)	422	843	0.21	449	853	290	342	342	342	342	342	342	
v/s Ratio Prot	0.03	0.21	0.21	0.03	0.23	c0.23	0.03	0.03	0.03	0.03	0.03	0.03	
v/s Ratio Perm	0.07	0.45	0.45	0.07	0.51	0.39	0.08	0.08	0.08	0.08	0.08	0.08	
Uniform Delay, d1	3.0	3.6	3.6	3.0	3.7	6.6	6.2	6.2	6.2	6.2	6.2	6.2	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.0	0.1	0.1	0.0	0.2	0.3	0.0	0.0	0.0	0.0	0.0	0.0	
Delay (s)	3.0	3.8	3.8	3.0	3.9	6.9	6.2	6.2	6.2	6.2	6.2	6.2	
Level of Service	A	A	A	A	A	A	A	A	A	A	A	A	
Approach Delay (s)	3.7	3.7	3.7	3.9	3.9	6.9	6.2	6.2	6.2	6.2	6.2	6.2	
Approach LOS	A	A	A	A	A	A	A	A	A	A	A	A	
Intersection Summary													
HCM 2000 Control Delay	4.3											HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio	0.47												
Actuated Cycle Length (s)	19.6											Sum of lost time (s)	6.4
Intersection Capacity Utilization	48.1%											ICU Level of Service	A
Analysis Period (min)	15												
c Critical Lane Group													

HCM Signalized Intersection Capacity Analysis  
4: Brookwood Avenue & Sonoma Avenue

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	
Traffic Volume (vph)	78	405	28	146	332	194	17	348	357	276	369	114	
Future Volume (vph)	78	405	28	146	332	194	17	348	357	276	369	114	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.0	3.6	3.0	3.0	3.6	3.0	3.0	3.2	3.0	3.0	3.6	3.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	1.00	1.00	0.98	1.00	1.00	1.00	0.98	
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	1.00	0.99	1.00	0.94	1.00	0.94	1.00	0.92	1.00	1.00	1.00	0.85	
Frb. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	
Satd. Flow (prot)	1767	3491	1764	3310	1770	1682	1770	1682	1770	1863	1545	1545	
Frb. Permitted	0.24	1.00	0.23	1.00	0.23	1.00	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	446	3491	422	3310	1770	1682	1770	1682	1770	1863	1545	1545	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	78	405	28	146	332	194	17	348	357	276	369	114	
RTOR Reduction (vph)	0	4	0	0	72	0	0	27	0	0	0	42	
Lane Group Flow (vph)	78	429	0	146	454	0	17	678	0	276	369	72	
Confl. Peds. (#/hr)	15	15	15	15	15	15	15	15	15	15	15	15	
Turn Type	pm+pl	NA	NA	pm+pl	NA	NA	Prot	NA	Prot	NA	pm+ov	NA	
Permitted Phases	5	2		1	6		3	8		7	4	5	
Prohibited Phases	2			6								4	
Actuated Green, G (s)	24.7	16.7	29.5	19.1	24.7	19.1	3.0	41.9	15.1	53.6	61.6	61.6	
Effective Green, g (s)	24.7	16.7	29.5	19.1	24.7	19.1	3.0	41.9	15.1	53.6	61.6	61.6	
Actuated g/C Ratio	0.25	0.17	0.30	0.20	0.30	0.20	0.03	0.43	0.16	0.55	0.64	0.64	
Clearance Time (s)	3.0	3.6	3.0	3.6	3.0	3.6	3.0	3.2	3.0	3.6	3.0	3.0	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	3.0	2.0	3.0	2.0	2.0	
Lane Grp Cap (vph)	222	601	272	652	54	727	54	727	275	1030	982	982	
v/s Ratio Prot	0.03	0.12	0.06	0.14	0.01	0.14	0.01	0.40	0.16	0.20	0.20	0.04	
v/s Ratio Perm	0.06	0.11	0.06	0.11	0.06	0.11	0.06	0.11	0.06	0.11	0.06	0.04	
v/c Ratio	0.35	0.71	0.54	0.70	0.31	0.31	0.31	0.93	1.00	0.36	0.07	0.07	
Uniform Delay, d1	28.5	37.8	26.2	36.2	45.9	26.2	45.9	26.2	40.9	12.1	6.7	6.7	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.4	3.3	1.0	2.6	1.2	18.8	1.2	18.8	55.2	0.2	0.0	0.0	
Delay (s)	28.9	41.2	27.2	38.8	47.2	45.0	47.2	45.0	96.1	12.3	6.8	6.8	
Level of Service	C	D	C	D	D	D	D	D	F	B	B	A	
Approach Delay (s)	39.3			36.3			45.1			41.9			
Approach LOS	D			D			D			D		D	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	40.9											HCM 2000 Level of Service	D
HCM 2000 Volume to Capacity ratio	0.87												
Actuated Cycle Length (s)	96.9											Sum of lost time (s)	13.2
Intersection Capacity Utilization	94.6%											ICU Level of Service	F
Analysis Period (min)	15												
c Critical Lane Group													

Traffic Study for the Santa Rosa Memorial Hospital  
AM Future + Project

Synchro 8 Report

HCM Signalized Intersection Capacity Analysis  
4: Brookwood Avenue & Sonoma Avenue

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	
Traffic Volume (vph)	104	421	50	290	445	166	18	296	189	199	567	175	
Future Volume (vph)	104	421	50	290	445	166	18	296	189	199	567	175	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	3.0	3.6	3.0	3.0	3.6	3.0	3.0	3.2	3.0	3.0	3.6	3.0	
Lane Util. Factor	1.00	0.95	1.00	0.95	1.00	1.00	1.00	0.98	1.00	1.00	1.00	0.98	
Frb. ped/bikes	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Frb. ped/bikes	1.00	0.98	1.00	0.96	1.00	0.96	1.00	0.94	1.00	1.00	1.00	0.85	
Frb. Protected	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	0.95	
Satd. Flow (prot)	1767	3461	1765	3371	1770	1724	1770	1724	1770	1863	1548	1548	
Frb. Permitted	0.27	1.00	0.22	1.00	0.22	1.00	0.95	1.00	0.95	1.00	1.00	1.00	
Satd. Flow (perm)	505	3461	416	3371	1770	1724	1770	1724	1770	1863	1548	1548	
Peak-hour factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Adj. Flow (vph)	104	421	50	290	445	166	18	296	189	199	567	175	
RTOR Reduction (vph)	0	8	0	0	31	0	0	19	0	0	0	74	
Lane Group Flow (vph)	104	463	0	290	580	0	18	466	0	199	567	101	
Confl. Peds. (#/hr)	15	15	15	15	15	15	15	15	15	15	15	15	
Turn Type	pm+pl	NA	NA	pm+pl	NA	NA	Prot	NA	Prot	NA	pm+ov	NA	
Permitted Phases	5	2		1	6		3	8		7	4	5	
Prohibited Phases	2			6								4	
Actuated Green, G (s)	27.3	18.2	35.4	23.3	27.3	23.3	2.7	32.5	15.3	44.7	53.8	53.8	
Effective Green, g (s)	27.3	18.2	35.4	23.3	27.3	23.3	2.7	32.5	15.3	44.7	53.8	53.8	
Actuated g/C Ratio	0.29	0.20	0.38	0.25	0.38	0.25	0.03	0.35	0.16	0.48	0.58	0.58	
Clearance Time (s)	3.0	3.6	3.0	3.6	3.0	3.6	3.0	3.2	3.0	3.6	3.0	3.0	
Vehicle Extension (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	3.0	2.0	3.0	2.0	2.0	
Lane Grp Cap (vph)	271	677	364	844	60.12	0.17	0.01	60.27	60.11	291	895	895	
v/s Ratio Prot	0.04	0.13	0.07	0.18	0.01	0.17	0.01	0.27	0.11	0.30	0.01	0.01	
v/s Ratio Perm	0.07	0.11	0.07	0.18	0.01	0.17	0.01	0.27	0.11	0.30	0.01	0.01	
v/c Ratio	0.38	0.68	0.80	0.69	0.35	0.77	0.35	0.77	0.68	0.63	0.11	0.11	
Uniform Delay, d1	24.9	34.7	22.4	31.6	44.3	27.0	44.3	27.0	36.6	18.0	8.8	8.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	0.3	2.3	10.8	1.9	1.5	6.2	1.5	6.2	5.2	1.5	0.0	0.0	
Delay (s)	25.2	37.0	33.1	33.4	45.8	33.1	45.8	33.1	41.8	19.5	8.9	8.9	
Level of Service	C	D	C	C	D	D	D	D	D	B	B	A	
Approach Delay (s)	34.9			33.3			33.6			22.2			
Approach LOS	C			C			C			C		C	
<b>Intersection Summary</b>													
HCM 2000 Control Delay	30.1											HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.80												
Actuated Cycle Length (s)	93.0											Sum of lost time (s)	13.2
Intersection Capacity Utilization	81.8%											ICU Level of Service	D
Analysis Period (min)	15												
c Critical Lane Group													

Traffic Study for the Santa Rosa Memorial Hospital  
PM Future + Project

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
 5: Sonoma Avenue & Sotoyome Street

09/07/2017



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	192	660	647	88	26	115
Traffic Volume (veh/h)	192	660	647	88	26	115
Future Volume (Veh/h)	192	660	647	88	26	115
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	192	660	647	88	26	115
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		TWLT/L	TWLT/L			
Median storage (veh)		2	2			
Upstream signal (ft)		595				
pX platoon unblocked					0.88	
VC conflicting volume	735				1735	691
VC1 stage 1 conf vol					691	
VC2 stage 2 conf vol					1044	
VCu unblocked vol	735				1768	691
IC single (s)	4.1				6.4	6.2
IC 2 stage (s)	2.2				5.4	
p0 queue free %	78				88	74
ICM capacity (veh/h)	870				222	445
Direction Lane #	EB 1	EB 2	WB 1	WB 1	SB 1	SB 1
Volume Total	192	660	735	141		
Volume Left	192	0	0	26		
Volume Right	0	0	88	115		
cSH	870	1700	1700	375		
Volumes to Capacity	0.22	0.39	0.43	0.38		
Queue Length 95th (ft)	21	0	0	43		
Control Delay (s)	10.3	0.0	0.0	20.3		
Lane LOS	B			C		
Approach Delay (s)	2.3		0.0	20.3		
Approach LOS				C		
Intersection Summary						
Average Delay			2.8			
Intersection Capacity Utilization			Err%		ICU Level of Service	
Analysis Period (min)			15		H	

Traffic Study for the Santa Rosa Memorial Hospital  
 AM Future + Project

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
 5: Sonoma Avenue & Sotoyome Street

09/07/2017



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations	91	631	709	56	75	206
Traffic Volume (veh/h)	91	631	709	56	75	206
Future Volume (Veh/h)	91	631	709	56	75	206
Sign Control	Free	Free	Free	Stop	Stop	Stop
Grade	0%	0%	0%	0%	0%	0%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	91	631	709	56	75	206
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		TWLT/L	TWLT/L			
Median storage (veh)		2	2			
Upstream signal (ft)		595				
pX platoon unblocked					0.85	
VC conflicting volume	765				1550	737
VC1 stage 1 conf vol					737	
VC2 stage 2 conf vol					813	
VCu unblocked vol	765				1559	737
IC single (s)	4.1				6.4	6.2
IC 2 stage (s)	2.2				5.4	
p0 queue free %	89				75	51
ICM capacity (veh/h)	848				301	418
Direction Lane #	EB 1	EB 2	WB 1	WB 1	SB 1	SB 1
Volume Total	91	631	765	281		
Volume Left	91	0	0	75		
Volume Right	0	0	56	206		
cSH	848	1700	1700	379		
Volumes to Capacity	0.11	0.37	0.45	0.74		
Queue Length 95th (ft)	9	0	0	146		
Control Delay (s)	9.8	0.0	0.0	37.3		
Lane LOS	A			E		
Approach Delay (s)	1.2		0.0	37.3		
Approach LOS				E		
Intersection Summary						
Average Delay			6.4			
Intersection Capacity Utilization			Err%		ICU Level of Service	
Analysis Period (min)			15		H	

Traffic Study for the Santa Rosa Memorial Hospital  
 PM Future + Project

Synchro 8 Report

HCM Unsignalized Intersection Capacity Analysis  
6: Doyle Park Drive & Sonoma Avenue

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	104	428	66	49	639	32	61	3	36	11	1	57
Future Volume (Veh/h)	104	428	66	49	639	32	61	3	36	11	1	57
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	104	428	66	49	639	32	61	3	36	11	1	57
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)									1			
Median type												
Median storage (veh)												
Upstream signal (ft)												
VC, platoon unblocked												
VC, conflicting volume	671	494		494		1464	1438	461	1408	1455	1455	655
VC1, stage 1 conf vol				669		669	669		753	753	753	
VC2, stage 2 conf vol				794		794	769		656	702	702	
VCU, unblocked vol	671	494		494		1464	1438	461	1408	1455	1455	655
IC, single (s)	4.1	4.1		4.1		7.1	6.5	6.2	7.1	6.5	6.5	6.2
IC, 2 stage (s)												
IC, 2 stage free %	2.2	2.2		2.2		3.5	4.0	3.3	3.5	4.0	3.3	3.3
p0 queue free %	89	95		95		69	99	94	96	100	88	88
CM capacity (veh/h)	919	1070		1070		195	249	600	260	274	274	466
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 1	SB 1	SB 1				
Volume Total	104	494	49	671	100	69						
Volume Left	104	0	49	0	61	11						
Volume Right	0	66	0	32	36	57						
cSH	919	1700	1070	1700	284	410						
Volumes to Capacity	0.11	0.29	0.05	0.39	0.35	0.17						
Queue Length 95th (ft)	10	0	4	0	38	15						
Control Delay (s)	9.4	0.0	8.5	0.0	24.4	15.5						
Lane LOS	A	A	A	C	C	C						
Approach Delay (s)	1.6	0.6		24.4	15.5							
Approach LOS				C	C							
Intersection Summary												
Average Delay	3.3											
Intersection Capacity Utilization	62.2%											
ICU Level of Service	B											
Analysis Period (min)	15											

HCM Unsignalized Intersection Capacity Analysis  
6: Doyle Park Drive & Sonoma Avenue

09/07/2017

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Traffic Volume (veh/h)	41	764	41	15	421	14	28	5	24	21	7	152
Future Volume (Veh/h)	41	764	41	15	421	14	28	5	24	21	7	152
Sign Control	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free	Free
Grade	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
Peak Hour Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Hourly flow rate (vph)	41	764	41	15	421	14	28	5	24	21	7	152
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)									1			
Median type												
Median storage (veh)												
Upstream signal (ft)												
VC, platoon unblocked												
VC, conflicting volume	435	805		805		1473	1332	784	1318	1345	1345	428
VC1, stage 1 conf vol				866		866	866		458	458	458	
VC2, stage 2 conf vol				606		606	465		860	887	887	
VCU, unblocked vol	435	805		805		1473	1332	784	1318	1345	1345	428
IC, single (s)	4.1	4.1		4.1		7.1	6.5	6.2	7.1	6.5	6.5	6.2
IC, 2 stage (s)												
IC, 2 stage free %	2.2	2.2		2.2		3.5	4.0	3.3	3.5	4.0	3.3	3.3
p0 queue free %	96	98		98		88	98	94	92	98	98	76
CM capacity (veh/h)	1125	819		819		235	312	393	273	302	302	627
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	NB 1	SB 1	SB 1				
Volume Total	41	805	15	435	57	180						
Volume Left	41	0	15	0	28	21						
Volume Right	0	41	0	14	24	152						
cSH	1125	1700	819	1700	427	525						
Volumes to Capacity	0.04	0.47	0.02	0.26	0.13	0.34						
Queue Length 95th (ft)	3	0	1	0	11	38						
Control Delay (s)	8.3	0.0	9.5	0.0	18.8	15.4						
Lane LOS	A	A	A	C	C	C						
Approach Delay (s)	0.4	0.3		18.8	15.4							
Approach LOS				C	C							
Intersection Summary												
Average Delay	2.8											
Intersection Capacity Utilization	66.9%											
ICU Level of Service	C											
Analysis Period (min)	15											

# Appendix C

---

## Pedestrian Crossing Warrant





# TCRP Report 112 - NCHRP Report 562 - Pedestrian Crossing Treatment Worksheet

## Worksheet 1: Peak-Hour, 35 MPH or Less

### Analyst and Site Information

Analyst: W-Trans	Major Street: Montgomery Drive
Analysis Date: 12-Jun-17	Minor Street or Location: Mid-Block Crossing at Memorial Hospital
Data Collection Date: 11/3/2015	Peak Hour: PM

**Step 1:** Select worksheet (speed reflects posted or statutory speed limit or 85th percentile speed on the major street):

a) Worksheet 1 - 35 mph or less  
 b) Worksheet 2- exceeds 35 mph, communities with less than 10,000, or where major transit stop exists

**Step 2:** Does the crossing meet minimum pedestrian volumes to be considered for a TCD type of treatment?

**2a** Peak-hour pedestrian volume (ped/h), vp **2a** 61  
 o If 2a ≥ 20 ped/h, then go to Step 3.  
 o If 2a < 20 ped/h, then consider median refuge islands, curb extensions, traffic calming, etc. as feasible.

**Step 3:** Does the crossing meet the pedestrian volume warrant for a traffic signal?

**3a** Major road volume, total of both approaches during peak hour (veh/h), V maj-s **3a** 679  
**3b** o Minimum signal warrant volume for peak hour (use 3a for Vmaj-s), SC **3b** 437.33  
 • SC = 0.00021 Vmaj-s<sup>2</sup> - 0.74072 Vmaj-s + 734.125/0.75 OR  
 • [(0.00021 3a<sup>2</sup> - 0.74072 3a + 734.125)/0.75]

**3c** o If 3b < 133, then enter 133. If 3b ≥ 133, then enter 3b. **3c** 437.3263067  
**3d** o If 15th percentile crossing speed of pedestrians is less than 3.5 ft/s (1.1 m/s), then reduce 3c by up to 50 percent; otherwise enter 3c. **3d** 437.3263067  
 o If 2a ≥ 3d, then the warrant has been met and a traffic signal should be considered if not within 300 ft of another traffic signal. Otherwise, the warrant has not been met. Go to Step 4.

**Step 4:** Estimate pedestrian delay.

**4a** Pedestrian crossing distance, curb to curb (ft), L **4a** 56  
**4b** Pedestrian walking speed (ft.s), Sp **4b** 2.5  
**4c** Pedestrian start-up time and end clearance time (s), ts **4c** 4  
**4d** o Critical gap required for crossing pedestrian (s), tc = (L/Sp) + ts OR [(4a/4b) + 4c] **4d** 26.40

**4e** Major road volume, total of both approaches or approach being crossed if median refuge island is present during peak hour (veh.h), Vmaj-d **4e** 679  
**4f** o Major road flow rate (veh/s), v = Vmaj-d/3600 OR [4e/3600] **4f** 0.19

**4g** o Average pedestrian delay (s/person), dp = (e<sup>-v tc</sup> - v tc - 1) / v OR [(e<sup>4f x 4d</sup> - 4f x 4d - 1) / 4f] **4g** 739.08  
**4h** o Total pedestrian delay (h), Dp = (dp x Vp) / 3600 OR [(4g x 2a) / 3600] **4h** 12.52

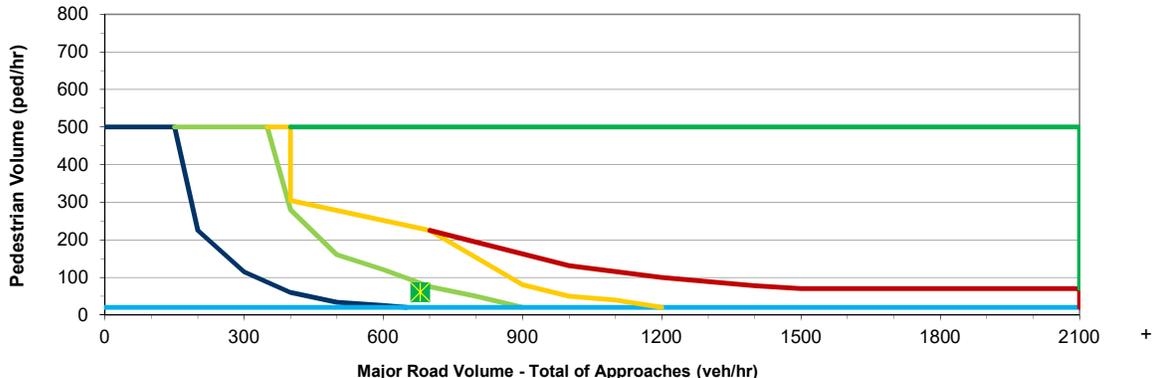
(this is estimated delay for all pedestrians crossing the major roadway without a crossing treatment - assumes 05 compliance). This calculated value can be replaced with the actual total pedestrian delay measured at the site.

**Step 5:** Select treatment based upon total pedestrian delay and expected motorist compliance.

**5a** Expected motorist compliance at pedestrian crossings in region, Comp = high or low **5a** HIGH

Total Pedestrian Delay Dp (4h) and Comp (5a)	Treatment Category (see Descriptions of Sample Treatments for examples)
Dp ≥ 21.3h (Comp = high or low) OR 5.3h ≤ Dp < 21.3 h and Comp = low	DO NOT USE RED
1.3h ≤ Dp < 21.3h and Comp = high or low) OR 5.3 ≤ Dp < 21.3 h and Comp = high	USE ACTIVE OR ENHANCED
Dp < 1.3 h (Comp = high or low)	DO NOT USE CROSSWALK

**Roadway Configuration:** 50' Wide, >35 mph, Vped = 3.5 ft/s



LEGEND
<span style="background-color: #90ee90; border: 1px solid black; display: inline-block; width: 15px; height: 10px;"></span> Study Intersection
<span style="background-color: #90ee90; border: 1px solid black; display: inline-block; width: 15px; height: 10px;"></span> Signal
<span style="background-color: #90ee90; border: 1px solid black; display: inline-block; width: 15px; height: 10px;"></span> Enhanced-High Visibility/Active when Present
<span style="background-color: #ff0000; border: 1px solid black; display: inline-block; width: 15px; height: 10px;"></span> Red
<span style="background-color: #ff0000; border: 1px solid black; display: inline-block; width: 15px; height: 10px;"></span> Enhanced-High Visibility/Active when Present (if high compliance expected) OR Red (if low compliance expected)
<span style="background-color: #0000ff; border: 1px solid black; display: inline-block; width: 15px; height: 10px;"></span> Striped Crosswalk
<span style="background-color: #0000ff; border: 1px solid black; display: inline-block; width: 15px; height: 10px;"></span> No Treatment

DESCRIPTIONS OF TREATMENT TYPE		
RED	ENHANCED-HIGH VISIBILITY/ACTIVE WHEN PRESENT	
	Active When Present	Enhanced/High Visibility
<ul style="list-style-type: none"> <li>• Midblock Signal</li> <li>• Half Signal</li> <li>• HAWK</li> </ul>	<ul style="list-style-type: none"> <li>• In Roadway Warning Lights</li> <li>• Passive/Pushbutton Flashing Beacons</li> <li>• Pedestrian Crossing Flags</li> <li>• Rapid Rectangular Flashing Beacons</li> </ul>	<ul style="list-style-type: none"> <li>• In-Street Crossing Signs</li> <li>• High Visibility Signs/Markings</li> <li>• Pedestrian Refuge Islands</li> <li>• Raised Crosswalks</li> <li>• Curb Extensions</li> <li>• Advanced Signage</li> <li>• Advanced Stop/Yield Lines</li> <li>• Constant Flashing Yellow Beacons</li> </ul>



# Appendix D

---

## Traffic Signal Warrant





# Warrant 3: Peak-Hour Volumes and Delay

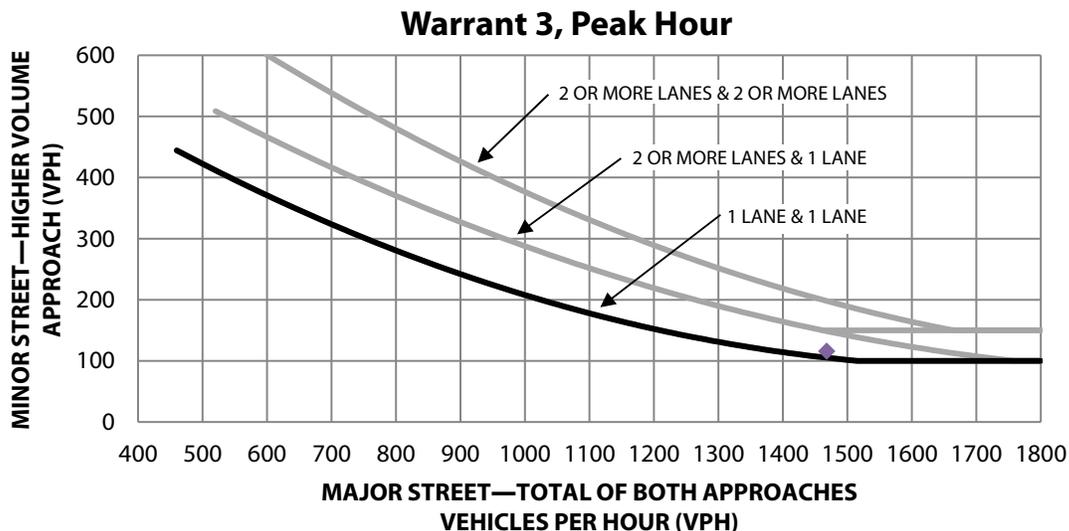
City of Santa Rosa  
Sonoma Ave & Sotoyome St

SRO376

	<b>Major Street</b>	<b>Minor Street</b>
<b>Street Name</b>	Sonoma Ave	Sotoyome St
<b>Direction</b>	E-W	N-S
<b>Number of Lanes</b>	1	1
<b>Approach Speed</b>	35	25
<b>Date of Count:</b>	Tuesday, November 03, 2015	
<b>Population less than 10,000?</b>	No	

**Scenario:** AM Existing plus Project

<b>Warrant 3 Met?: Met when either Condition A or B is met</b>	<b>Yes</b>
Condition A: Met when conditions A1, A2, and A3 are met	Not Met
Condition A1	Not Met
<p>The total delay experienced by traffic on one minor street approach (one direction only) controlled by a STOP sign equals or exceeds four vehicle-hours for a one lane approach, or five vehicle-hours for a two-lane approach</p> <p style="text-align: right;">Minor Approach Delay: 0.63 vehicle-hours</p>	
Condition A2	Met
<p>The volume on the same minor street approach (one direction only) equals or exceeds 100 vph for one moving lane of traffic of 150 vph for two moving lanes</p> <p style="text-align: right;">Minor Approach Volume: 116 vph</p>	
Condition A3	Met
<p>The total entering volume serviced during the hour equals or exceeds 800 vph for intersections with four or more approaches or 650 vph for intersections with three approaches</p> <p style="text-align: right;">Total Entering Volume: 1584 vph</p>	
Condition B	Met
The plotted point falls above the curve	



# Warrant 3: Peak-Hour Volumes and Delay

City of Santa Rosa  
Sonoma Ave & Sotoyome St

SRO376

	<b>Major Street</b>	<b>Minor Street</b>
<b>Street Name</b>	Sonoma Ave	Sotoyome St
<b>Direction</b>	E-W	N-S
<b>Number of Lanes</b>	1	1
<b>Approach Speed</b>	35	25
<b>Date of Count:</b>	Tuesday, November 03, 2015	
<b>Population less than 10,000?</b>	No	

**Scenario:** PM Existing plus Project

<b>Warrant 3 Met?: Met when either Condition A or B is met</b>	<b>Yes</b>
Condition A: Met when conditions A1, A2, and A3 are met	Not Met
Condition A1	Not Met
The total delay experienced by traffic on one minor street approach (one direction only) controlled by a STOP sign equals or exceeds four vehicle-hours for a one lane approach, or five vehicle-hours for a two-lane approach	
Minor Approach Delay: 2.84 vehicle-hours	
Condition A2	Met
The volume on the same minor street approach (one direction only) equals or exceeds 100 vph for one moving lane of traffic of 150 vph for two moving lanes	
Minor Approach Volume: 256 vph	
Condition A3	Met
The total entering volume serviced during the hour equals or exceeds 800 vph for intersections with four or more approaches or 650 vph for intersections with three approaches	
Total Entering Volume: 1624 vph	
Condition B	Met
The plotted point falls above the curve	

