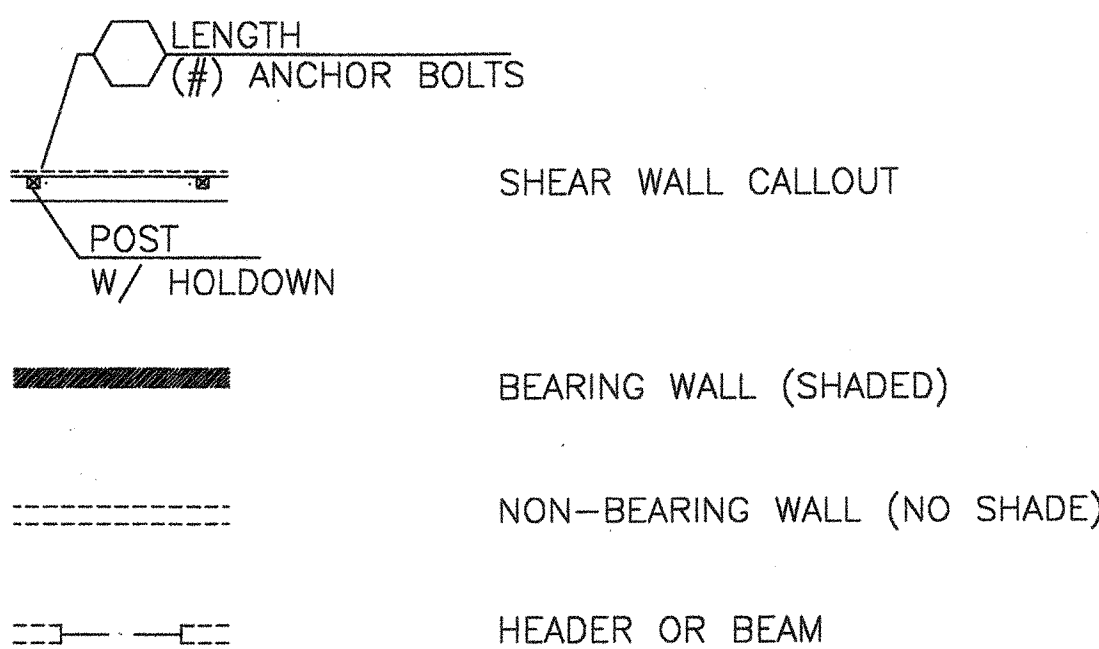


ABBREVIATIONS

● AT	ANCHOR BOLT	EXT	EXTERIOR	O/C	OVER
A.B.	ANCHOR BOLT	FIN	FINISHED	O/C	OPPOSITE
A.B.	ANCHOR BOLT	FND	FOUNDATION	±	PLUS OR MINUS
ALT.	ALTERNATE	F.O.C.	FACE OF CONCRETE	P.D.F.	POWER DRIVEN FASTENER
A.Y.C.	ALASKAN YELLOW CEDAR	F.O.M.	FACE OF MASONRY	PERF	PERFORATED
BD	BOARD	F.O.P.	FACE OF POST	PERF	PERFORATED
BLDG	BUILDING	F.S.	FACE OF STUD	PLATE	PLATE
BLK(O)	BLOCKING	F.N.	FACE NAIL	PLY	PLYWOOD
B.W.	BELLY	FRNG	FRAMING	PSF	POUNDS/SQUARE FOOT
BM	BEAM	FT	FOOT/FEET	PSI	POUNDS/SQUARE INCH
BOT	BOTTOM	FTG	FOOTING	P.S.L.	PARALLEL STRAND LUMBER
B.P.	BRACED PANEL	GA	GAGE OR GAGE	P.T.	PRESSURE TREATED
C	CENTERLINE	GALV	GALVANIZED	R	RADIUS
CA	CALIFORNIA	GLB	GLUE-LAMINATED BEAM	REF	REFERENCE
CNT	CANTILEVER	BYP	OUTSIDE	REF	REINFORCING
C.P.	CAST IN PLACE	HT	HEIGHT	REQ	REQUIRED
C.J.	CONTROL JOINT	HOR	HANGER	RET	RETAINING
C.L.	CLEAR	HORIZ	HORIZONTAL	R.O.	ROUGH OPENING
C.M.U.	CONCRETE MASONRY UNIT	H.S.	HIGH STRENGTH	RWD	REDWOOD
COL	COLUMN	HORIZ	HORIZONTAL	S.A.D.	SEE ARCHITECTURAL DRAWINGS
CONC	CONCRETE	INT	INTERIOR	SHTG	SHEATHING
CONC	CONCRETE	INT	INTERIOR	SM	SINGLE
CONT	CONTINUOUS	JOIST	JOIST	SM	SEE MECHANICAL DRAWINGS
COUNT	COUNTERSINK	JOIST	JOIST	SPRCS	SPECIFICATIONS
4	PENNY (NAIL SIZE)	K.P.	KING POST	SO	SQUARE
DET	DETAIL	L.B.	LAP BOLT	S.S.	SELECT STRUCTURAL
DBL	DOUBLE	L.B.	LAP BOLT	STD	STANDARD
D.F.	DOUGLAS FIR	L.L.S.	LIGHT GAUGE STEEL	STL	STEEL
DM	DIAMETER (ø)	LL	LIVE LOAD	SW	SHEARWALL
D.W.	DOWN	LDC	LOCATION	SYN	SYMMETRICAL
D.L.	DEAD LOAD	L.L.	LAMINATED VENEER LUMBER	T&B	TOP & BOTTOM
D.W.	DOWN	L.S.	LAMINATED VENEER LUMBER	T&B	TOP & BOTTOM
E	EXISTING	L.S.	LAMINATED VENEER LUMBER	T&B	TOP & BOTTOM
E.B.	EXPANSION BOLT	M	MAXIMUM	T&B	TOP & BOTTOM
E.L.	ELEVATION	M	MAXIMUM	T&B	TOP & BOTTOM
EMBED	EMBEDMENT	M	MAXIMUM	T&B	TOP & BOTTOM
E.N.	EDGE NAIL	M	MAXIMUM	T&B	TOP & BOTTOM
ENR	ENGINEER	M	MAXIMUM	T&B	TOP & BOTTOM
EQ	EQUAL	M	MAXIMUM	T&B	TOP & BOTTOM
EQW	EQUAL	M	MAXIMUM	T&B	TOP & BOTTOM
E.W.	EACH WAY	M	MAXIMUM	T&B	TOP & BOTTOM
EXP	EXPANSION	M	MAXIMUM	T&B	TOP & BOTTOM

SYMBOL LEGEND



1 52 DETAIL NUMBER SHEET NUMBER

SECTION NUMBER SHEET NUMBER SECTION CUT

CONTINUOUS MEMBER

BLOCK MEMBER

SHEATHING

HANGERS

SHEATHING NAILING PER PLAN

REVISION AREA

REVISION NUMBER

GENERAL NOTES

- All work shall be in accordance with the 2019 CRC & CBC Edition and any applicable local ordinances except where other notes are more restrictive.
- Drawings are not to be scaled. Building dimensions shown on the structural drawings are for general reference only. See architectural drawings for all actual building dimensions. The contractor shall notify the architect/engineer of any discrepancies on the drawings requiring clarification or revisions prior to commencing work.
- Details not shown, nor detailed on drawings, nor called for in these notes shall be constructed to some size and character as for similar conditions which are shown, detailed or specified.
- At all times the contractor shall be solely and completely responsible for conditions at the job site, including safety of persons and property, also design, adequacy and safety of temporary shoring, bracing, form work, scaffolding, erection methods, etc. Any job site visit by the engineer is not intended to include review of the adequacy of the contractor's safety measures.
- The contractor shall warrant that all materials and workmanship are in compliance with the drawings and specifications. Any and all changes must have the architect's approval.

DESIGN CRITERIA

	2019	CRC & CBC
1 Vertical loads:		
Roof	Dead Load: 12 PSF	Live Load: 20 PSF
Floor	Dead Load: 11 PSF	Live Load: 40 PSF
2 Soil / Foundation:		
Soil Profile Type:	S	
Allowable Soil Bearing Pressure:	1500	PSF
3 Lateral Loads:		
Wind Loads:	80 MPH wind, Exposure C	110 MPH ULT.

FOUNDATION NOTES

- All site work, drainage systems, grading, and foundation excavations shall be done in accordance with Chapter 18 of the 2019 CRC and Chapter 3 & 4 of the 2019 CRC.
- Foundation design is based upon the minimum footing dimensions set forth in 2019 CRC Table R-403.1(1). The design assumes class 3 soil with a bearing pressure of 1500 p.s.f. and constant expansion index of less than 20 unless otherwise noted in the structural calculations.
- Bottom and top of footings shall be level and stepped where slope exceeds 1:10. Provide 7" minimum horizontal confinement from bottom of footing to face of slope.
- All foundations shall bear on firm, undisturbed, native soils or engineered fill at or exceeding the depths shown on the drawings. All footing excavations shall be backfilled with concrete, or shall be reported to the engineer or architect and backfilled as the engineer or architect directs. All loose soils shall be removed from excavations prior to placement of concrete.
- Garage slab on grade shall be 4 inches thick with #3 @ 18" o/c as way at center line of slab over 1" of sand over 6 mil. vapor barrier over 1" sand over 4" minimum of clean, free-draining gravel or crushed rock over prepared subgrade.
- Porch slab on grade shall be 4 inches thick with #3 @ 18" o/c as way at center line over compacted backfill or crushed gravel placed early during construction so as to achieve maximum settlement prior to pouring of slab.
- All slabs shall be completely separated from foundation stem walls with felt or insulate and slabs shall have 1/8" x 1" deep control joints at 12 feet on center maximum in each direction unless otherwise noted.
- Provide 20"-0" #4 AWG bare copper wire in bottom 2" of footing/grade beam with grounding electrode below electrical panel where occurs.
- Anchor bolt size and spacing shall be per plan. Bolts shall have 7" minimum embedment into concrete. Bolts shall be located 6" minimum and 12" maximum from end of all plates. Anchor bolts and inserts shall be rigidly held in place prior to placing concrete.
- Shear walls and interior braced wall panels are designated with See WALL FRAMING NOTES on this sheet for general requirements.
- Sheath all exterior walls and cripple walls per of "4/S1" unless otherwise noted (or shown as a shear wall on plan). All exterior wall panels four (4) feet or greater in length without openings shall be considered braced wall panels meeting the requirements of 2019 CRC & CBC.
- Shear wall tie down anchors shall be "Holdown" or "Tension Tie" anchors as manufactured by Simpson Strong-Tie Company, Inc. and referenced per the current catalog specifications (see H04, HT22). See detail "5/S1" for installation requirements. Where tie down anchors are specified on the foundation plan, then the tie down anchor shall be located just above the first floor framing. If cripple walls occur below the first floor framing, then extend the tie down anchor from the foundation up through the cripple wall to the tie down anchor, as shown in detail "5/S1".

REINFORCING STEEL

- All reinforcing steel shall conform with ASTM A-615, grade 40 for #5 bar and smaller and grade 60 for #6 bar and larger. Reinforcing steel shall be kept clean and free of rust.
- Welded wire mesh shall conform with ASTM A-185 and shall be lapped 12" minimum and be placed at center depth of slab.
- All reinforcing bars shall be as long as is practical and all bends shall be cold bent. Securely tie all reinforcing bars at each end or as near thereto as possible and at a maximum of 48" on center prior to placement of concrete. At corners and intersections, bars shall return a minimum of 24". All reinforcing bar splices shall be lapped per detail "11/S1".

CONCRETE NOTES

- All concrete shall be normal weight (150 pcf) hard rock concrete and shall develop a minimum ultimate compressive strength of 2500 psi at 28 days and be 3 sack mix unless otherwise noted. Design is based on compressive strength of 2500 psi. Concrete quality mixing and placing shall conform with USC Section 1905.
- Minimum concrete cover requirements for reinforcing steel shall be: 3" when cast against and exposed to earth 2" when formed and exposed to earth or weather 3/4" when not exposed to weather or in contact with ground (slabs & walls)
- 20 % of concrete mix to be made up of fly ash

MANUFACTURED WOOD PRODUCTS

- Glue-Lam beams shall conform to ANSI/APC 190.1 and ASTM D3737. Glue-Lams shall be combination 24F-V4 for simple span conditions and 24F-V8 for continuous and cantilever conditions unless otherwise noted. Beams shall have standard camber (1600±) unless otherwise noted. Glue-Lam beams shall meet the following design properties: E = 1,800,000 psi; Fb = 2400 psi; Fv = 185 psi. GLB's shall be industrial appearance u.o.n.
- All Glue-Lam beam inspection certificates shall be submitted to the field inspector prior to completion of frame inspection.

CARPENTRY NOTES

- Lumber shall be grade marked in accordance with "standard grading and drawing rules" of the West Coast Lumber Inspection Bureau (WCLIB).
- All lumber framing members shall be Douglas Fir (D.F.) of the following grades unless specifically noted otherwise on the plans:
4x Beams & Posts No. 2
6x Beams & Posts No. 1
Studs > 10 feet No. 2
Studs < 10 feet STD or better
Plates & Bracing No. 1
- All framing lumber shall have a maximum moisture content of 19%. Green lumber shall be braced at midspan, until dried, to prevent sagging.
- Lumber in contact with concrete shall be pressure treated Douglas Fir (P.T.D.F.) conforming to AWPB LP-2 standards. Lumber embedded in concrete or earth shall be pressure treated Douglas Fir (P.T.D.F.) conforming to AWPB LP-22 standards. Use Chromated Copper Arsenate (CCA) treatment compounds or similar chemicals that are non-corrosive to zinc-coated steel.
- Structural plywood sheathing shall be CD grade, exposure 1, conforming to PS-1-95 of the American Plywood Association unless otherwise noted. Supported edges of plywood shall butt along the center of framing members.
- Nails used in exterior applications or in pressure preservative treated wood shall be hot-dipped zinc coated galvanized box nails, unless otherwise noted. Where nailing causes wood to split, replace member and predrill holes.
All nailing not specifically called out on plans shall be per table 2019 CRC & CBC (also applies to girt nailing). Nail types shall be as indicated below:
a. Roof and Floor Sheathing Common Nails
b. Shearwall Sheathing Common or Hot Dipped Galvanized Box Nails (not treated) box nails are not acceptable
Common, Box or Coated Sinker Nails u.o.n.
Nails shall be as specified by manufacturer u.o.n.
c. Framing
d. Metal Connectors

Nail Size	Length	Common	Wire Diameter (Inches)
6d	2"	.113	.099
8d	2 1/2"	.131	.113
10d	3"	.148	.128
12d	3 1/4"	.148	.128
16d	3 1/2"	.162	.135
20d	4"	.182	.148

- All metal framing connectors, clips, joist hangers, straps, etc. shall be manufactured by Simpson Strong-Tie Company. Fill all nail or bolt holes in connectors per manufacturer's specifications and per note 5 above. Install and tighten connectors just prior to covering where possible.
- Bolts shall be standard machine bolts (M.B.) conforming to ASTM 307 with slits as shown on the plans and details. Use malleable iron or plate washers under head and nut where bearing is against wood. Bolt holes in wood shall be 1/16" larger than the bolt diameter. Holes for bolts shall not be located in or near lumber with knots or checks. Bolts and washers exposed to weather shall be galvanized.
- Structural members shall not be notched, cut or otherwise altered for ducts, pipes, etc. u.o.n. or approved by the engineer. See detail "2/S1".
- Place all lumber joists and beams with the crown up.
- To minimize lumber shrinkage effects, install and tighten straps and tie down anchors (Holdowns) just prior to covering, whenever possible.
- Logs require preboring as follows:

SHANK DIAMETER	LEAD	HOLE (1)	HOLE (2)
1 1/4"	3/32"	1/4"	1/4"
1 1/2"	5/16"	1/2"	1/2"
3/8"	3/8"	3/8"	3/8"

(1) Lead hole depth equals log length.
Shank hole depth equals shank length.
Do not drive log screws with hammer.
Soak or lubricate threads to ease installation.

WALL FRAMING NOTES

- See sheet S1 for standard construction details.
- Shear walls and interior braced wall panels are designated with See SHEAR WALL / BRACED WALL SCHEDULE "4/S1" for specific and general requirements.
- Sheath all exterior walls and cripple walls per of "4/S1" unless otherwise noted (or shown as a shear wall on plan). All exterior wall panels four (4) feet or greater in length without openings shall be considered braced wall panels meeting the requirements of 2019 CRC & CBC unless otherwise noted.
- Stud sizes shall be as specified on plan and where not noted shall be per the table shown in detail "12/S1" unless otherwise noted.
- All studs shall be framed full height (continuous places) between supporting floors, ceilings, and roofs unless otherwise noted. End joints in wall double top plates shall be offset a minimum of 48 inches. See detail "11/S1" for typical lap splice connection.
- Foundation cripple walls shall be framed of studs not smaller in size than the studs in the level above. Cripple walls greater than four feet high shall be 2x6 @ 16" o/c minimum when supporting two floor levels.
- Cripple wall stud lengths less than 8 inches shall be framed of solid double 2x lumber.
- Header sizes shall be as specified on plan and where not noted shall be per the schedule shown in detail "15/S1" unless otherwise noted. Interior bearing walls below framing are shown shaded. All exterior walls are bearing walls unless otherwise noted.
- Posts indicated on framing plans are below unless otherwise noted. Post sizes are indicated on the highest level framing plan on which they occur. Provide post at each level below uppermost post to foundation or beam/header support.
- Posts indicated but not called out are a minimum of (2) 2x studs. When supporting 4x members provide 1/2" plywood filler between studs. See detail "3/S1" for headers framed into walls. Provide multiple studs under all multiple joists and roof girder trusses to match bearing width.
- At floor levels, solid block in the joist space under all posts to the full width of the post.
- Notches and holes cut in studs shall be per detail "2/S1" unless otherwise noted. Increase stud size as necessary to accommodate holes. Do not notch studs for "let in" bracing.
- Wall framing at chimneys and fireplaces shall be full height. Chimneys shall be 24 inches minimum in each direction. Where splicing of studs is required, studs shall be double. The splice length shall be a minimum of one-third the length of the individual piece nailed with (2) 16d @ 8" o/c. Top plates interrupted by full height framing shall be strapped with a continuous CS16 strap. Extend the strap 30 inches beyond each end of the full height framing and nail with (2) 8d @ 4" o/c to the top plates and 2x blocking unless otherwise noted. Chimneys and fireplaces resulting splicing shall be sheathed with plywood or panel siding.
- At wall heights greater than 10'-0", provide 2x blocking at 10'-0" o/c maximum. At vertical board siding provide 2x blocking at 24" o/c maximum unless otherwise noted.
- Provide a minimum of 3 studs at each corner.

FLOOR FRAMING NOTES

- See sheet S1 for standard construction details.
- Shear walls and interior braced wall panels are designated with See WALL FRAMING NOTES on this sheet for general requirements.
- Floor shall be sheathed with APA rated sheathing, 48/24, exposure 1, T & G, 3/4" minimum thickness. Install sheath with face grain perpendicular to supports. Stagger sheets and glue and nail with 8d @ 8" o/c edges and 12" o/c field typical unless otherwise noted.
- When braced panels run perpendicular to joist add blocking under wall and double joist at ends.
- When braced panels run parallel to joist add double joist under wall.
- All floor openings shall be between joists unless otherwise noted.
- Provide additional joist under all parallel partitions of length greater than one-half the joist span or walls greater than four (4) feet in length unless otherwise noted.

FRAMING HANGERS

I-Joist hangers shall be as follows unless otherwise noted on plans:

Single LPI 16" CTR 350	MIT3516-2
Double LPI 16" CTR 350	WP3516-2 *
Sloped or skewed members	LSU Hanger

Shawn lumber hangers shall be as follows unless otherwise noted on plans:

Single 2x6	US28 Hanger
Double 2x6	US28-2 Hanger
Single 2x10	US210 Hanger
Double 2x10	US210-2 Hanger
Single 2x12	US212 Hanger
Double 2x12	US212-2 Hanger

* Requires web stiffeners.

ROOF FRAMING NOTES (Stick framing)

- See WALL FRAMING NOTES on this sheet for shear wall and braced panel general requirements.
- Roof shall be sheathed with APA rated sheathing, 24/0, exposure 1, 15/32" minimum thickness. Install sheath with face grain perpendicular to supports. Stagger sheets and nail with 8d @ 6" o/c edges and 12" o/c field typical unless otherwise noted.
- Building has been designed for a maximum roofing dead load of 4 psf.
- All roof openings shall be between rafters unless otherwise noted on the drawings.
- Interior bearing walls are shown shaded. All exterior walls are bearing walls unless otherwise noted. Brace purlins, rafters, hips, valleys, and ridge boards to bearing walls or beams only.
- California framing shall be 2x6 rafters and 2x6 ridges and hips unless otherwise noted. Brace California framing as typical framing below at 48" o/c maximum. Typical roof framing and plywood sheathing shall be continuous below California framed areas. Use 2x plate under all California framed rafters.
- Rafters shall be nailed to adjacent ceiling joists to form a continuous tie between exterior walls when such joists are parallel to the rafters. Where not parallel, rafters shall be tied to 1x4 (nominal size) minimum cross tie. Rafter ties shall be spaced not more than four (4) feet on center. Nail cross ties or ceiling joists per "5/S1" unless otherwise noted.
- Ceiling joists shall be sized and spaced per "16/S1" unless otherwise noted on plans.

TRUSS NOTES

- NOTE: PROVIDE TRUSS CALCULATIONS AND LAYOUT TO BUILDING OFFICIAL PRIOR TO INSTALLATION OF TRUSSES. TRUSS DOCUMENTATION SHALL INCLUDE:
- SLOPE OR DEPTH, SPAN AND SPACING
 - LOCATION OF ALL JOINTS
 - REQUIRED BEARING WIDTHS
 - DESIGN LOADS FOR TOP & BOTTOM CHORD LIVE & DEAD LOADS AND CONCENTRATED LOADS AND THEIR POINTS OF APPLICATION
 - ADJUSTMENTS TO LUMBER AND JOINT CONNECTOR DESIGN VALUES FOR CONDITIONS OF USE
 - EACH REACTION FORCE AND DIRECTION
 - JOINT CONNECTOR TYPE AND DESCRIPTION (SUCH AS SIZE, THICKNESS OR GAGE) AND THE DIMENSIONAL LOCATION OF EACH JOINT CONNECTOR EXCEPT WHERE SYMMETRICALLY LOCATED RELATIVE TO THE JOINT INTERFACE
 - LUMBER SIZE, SPECIES, AND GRADE FOR EACH MEMBER
 - CONNECTION REQUIREMENTS FOR:
A. TRUSS TO GIRDER TRUSS
B. TRUSS PLY TO PLY
C. FIELD SPLICES
 - CALCULATED DEFLECTION RATIO AND/OR MAXIMUM DEFLECTION FOR LIVE AND TOTAL LOAD
 - MAXIMUM AXIAL COMPRESSION FORCES IN THE TRUSS MEMBERS TO ENABLE THE BUILDING DESIGNER TO DESIGN THE SIZE CONNECTIONS AND ANCHORAGE OF THE PERMANENT CONTINUOUS LATERAL BRACING. FORCES SHALL BE SHOWN ON THE TRUSS DESIGN DRAWING OR SUPPLEMENTAL DOCUMENTS
 - REQUIRED PERMANENT TRUSS MEMBER BRACING LOCATION
 - REFER TO ARCHITECTURAL DRAWINGS FOR REQUIRED TRUSS SPANS AND REQUIRED CEILING AND ROOF LINES
 - MPD ROOF TRUSS DESIGN LOADS:
A. TOP CHORD: 2 PSF DEAD LOAD FOR COMPOSITION ROOFING AND 15 PSF DEAD FOR TILE ROOFS, LIVE LOAD OF 20 PSF
B. BOTTOM CHORD: 5 PSF DEAD LOAD, 10 PSF LIVE LOAD
 - THE PORTIONS, WEIGHTS, & METHODS OF ATTACHMENT OF ALL MECHANICAL UNITS, ELECTRICAL FIXTURES, PLUMBING, ETC. SHALL BE INCLUDED IN THE DESIGN OF THE TRUSSES BY THE TRUSS MANUFACTURER AND SHALL BE VERIFIED WITH THE DESIGNER. ADDITIONAL TRUSSES OR SPECIAL DESIGNED TRUSSES MAY BE REQUIRED.
 - SUBIMPOSED LOADS FROM ARCHITECTURAL FINISHES OR OTHER SECONDARY FRAMING (SUCH AS CALIFORNIA FRAMING, FURRED CEILING, SOFFITS, ETC.) SHALL BE INCLUDED IN THE DESIGN OF SUPPORTING TRUSSES.
 - TRUSS MANUFACTURER SHALL INCLUDE DEFLECTION CALCULATIONS WITH THE SHOP DRAWING SUBMITTALS
 - TRUSS MANUFACTURER IS RESPONSIBLE FOR DESIGN AND DETAILING OF ALL TRUSSES TO TRUSS AND BEAM TO BEAM CONNECTIONS.
 - MANUFACTURED TRUSS DOCUMENTS SHALL BE SUBMITTED AND APPROVED BY THE BUILDING OFFICIAL (AND ENGINEER OR ARCHITECT) PRIOR TO THE FABRICATION AND ERECTION OF THE TRUSSES.

REVISIONS

NO.	DESCRIPTION	DATE

FILE : 0010051

STUDIO 202
ARCHITECTURAL DESIGN, REVISIONS, COMPLIANCE, SITE PLANNING
BRENT RUSSELL ARCHITECT
202 WESTERN AVENUE, SUITE 200
SAN ANTONIO, TEXAS 78205
PHONE: FAX 787-769-0653

PROPOSED SHED REPLACEMENT AT
729 WHEELER ST., SANTA ROSA, CA
APN 009-252-022



Date :
Scale : NOTED
Drawn :
Job : 00100
Sheet

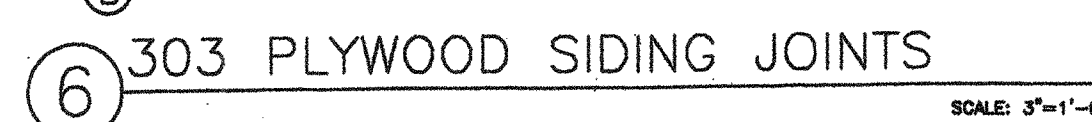
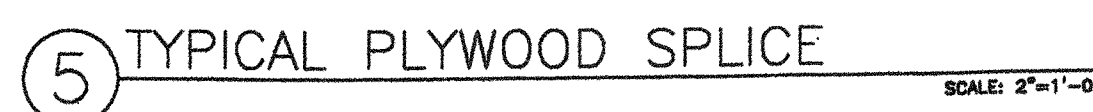
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SEE SHEARWALL/BRACED WALL NOTES 1-10

4 SHEARWALL/BRACED WALL SCHEDULE

- SHAPED/BRACED WALL SCHEDULE NOTES
1. SHEATHING SHALL BE APA RATED, EXPOSURE 1 OR EXTERIOR GRADE.
 2. SHEAR WALL NAILS SHALL BE COMMONS OR HOT-DIPPED GALVANIZED BOX (INCLUDING INTERIOR WALLS) SEE CARPENTRY NOTES 8 AND 7 ON SHEET SN.
 3. BLOCK AND NAIL ALL SHEATHING EDGES.
 4. FIELD NAILING TO BE 12" O.C. U.O.N.
 5. EXTERIOR WALLS NOT REFERENCED TO SHEAR WALL SCHEDULE ARE TO BE BRACED PER
 6. WALL FRAMING TO BE 16" O/C MAXIMUM.
 7. EDGE NAIL WALL PLY TO STUDS OR POST W/ HOLDINGS.
 8. SEE DETAIL 12/S1 FOR TYPICAL SHEAR WALL CONSTRUCTION.
 9. ALL SHEATHING SHALL BE SPICED AT CENTERLINE OF FRAMING OR BLOCKING (SEE 9/S1, 6/S1 & 7/S1).
 10. OPENINGS IN SHEAR WALLS ARE NOT ALLOWED UNLESS SPECIFICALLY DETAILLED.
- ADDITIONAL NOTES: (N/A UNLESS NOTED IN SCHEDULE)
- A. FRAMING AT ADJOINING PANEL EDGES SHALL BE 3 INCH MINIMUM OR WIDER AND NAILS SHALL BE STAGGERED. SEE 10/S1.
 - B. EDGE NAIL PANEL JOINTS THRU THICK PORTION OF PANEL PER 6/S1.
 - C. NAILING APPLICABLE TO ALL STUDS, TOP AND BOTTOM PLATES. USE 8D UNLS @ 5" x 5" O.P.D. OR. HORIZONTAL JOINTS WITH W/ALL AT A HEART LESS THAN 1/2 TIMES THE JOINT MAY BE UNLOCKED.
 - D. FRAMING AT ADJOINING PANEL EDGES SHALL BE 4 INCH MINIMUM OR WIDER AND NAILS SHALL BE STAGGERED. SEE DETAIL "10/S1".
 - E. MUDDSHILL SHALL BE 3 INCH NOMINAL OR WIDER AND NAILS INTO MUDDSHILL SHALL BE STAGGERED.



⑧ HOLDOWN SCHEDULE



⑨ NAILING SCHEDULE U.O.N.



⑮ HEADER SCHEDULE U.O.N.

(16) CEILING JOIST SCHEDULE U.O.N.

17 TYPICAL STUD SCHEDULE U.O.N.

